KINGDOM OF THAILAND
MINISTRY OF TRANSPORT AND COMMUNICATIONS
DEPARTMENT OF HIGHWAYS

## STANDARD DRAWINGS

FOR

HIGHWAY CONSTRUCTION

1994

# MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

## STANDARD DRAWINGS

FOR

### HIGHWAY CONSTRUCTION

### JOINTLY PREPARED BY

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1994

### PART I

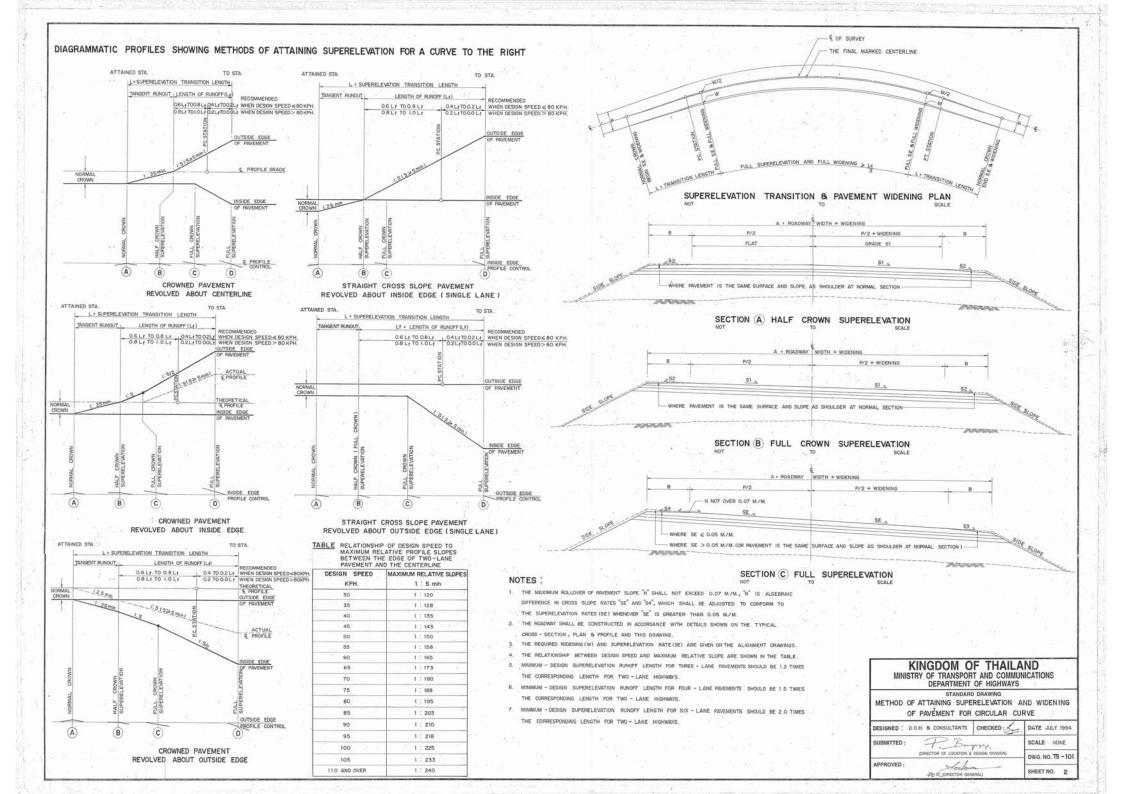
### LIST OF STANDARD DRAWINGS FOR ROADWORK

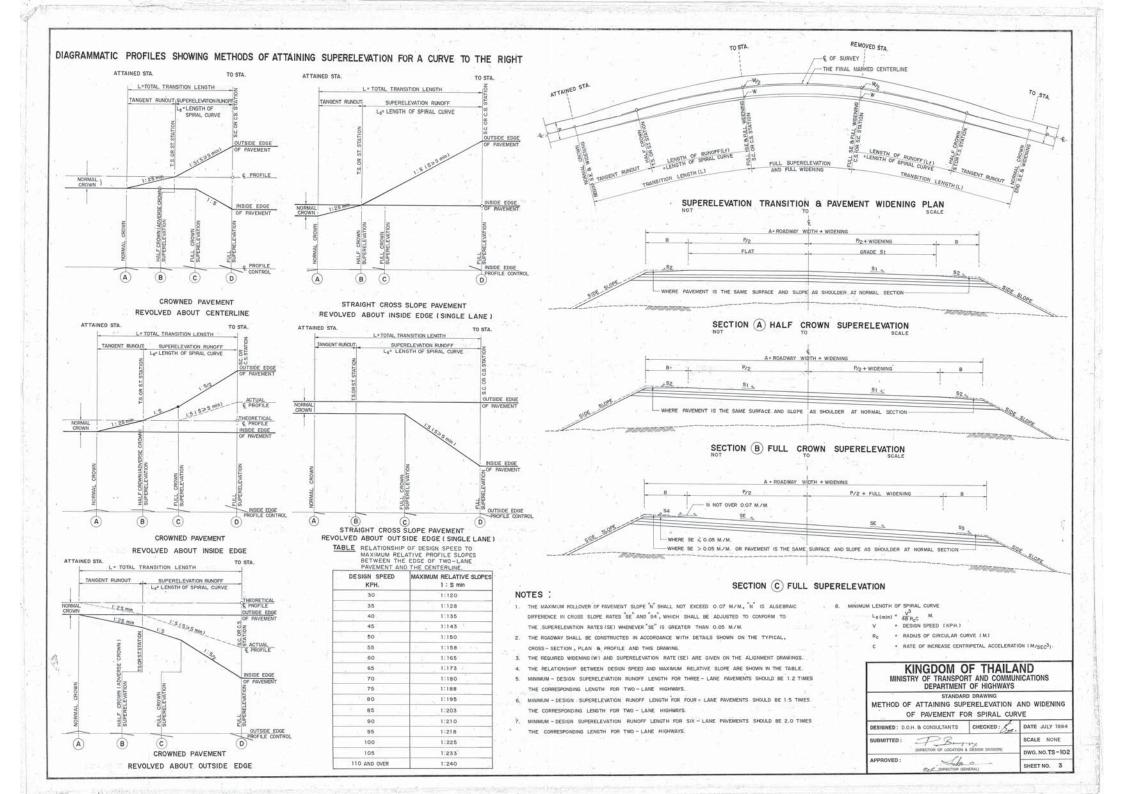
SHEET NO.	TITLE	DRAWING NO.			SHEET N	NO. TITLE	DRAWING N	NO.
				4				
E	ABBREVIATIONS AND SYMBOLS	AS - 001			57	RIP RAP PROTECTION FOR EMBANKMENT SLOPE	SP - 10	05
2	METHOD OF ATTAINING SUPERELEVATION AND WIDENING OF PAVEMENT FOR CIRCULAR CURVE	TS - 101			58	SPECIAL PROVISION OF GABIONS	SP - 10	06
	METHOD OF AT TAINING SUPERELEVATION AND WIDENING OF PAVENETY FOR CIRCULAR CORVE	TS - 102			59	SPECIFICATION OF GABIONS	SP - 10	
3,					60	SPECIFICATION OF GABIONS SPECIFICATION FOR GABIONS CONSTRUCTION	SP - 10	
	METHOD OF ATTAINING SUPERELEVATION AND WIDENING OF PAVEMENT FOR COMPOUND AND REVERSE CURVE BRIDGE APPROACH TRANSITION	TS - 103			61	FERRO - CEMENT BACK SLOPE PROTECTION	SP - 2	
		TS - 201			-	CONCRETE SQUARE GRID SLOPE PROTECTION	SP - 2	202
	TYPICAL CROSS—SECTION AT VILLAGE SECTION—I TYPICAL CROSS—SECTION AT VILLAGE SECTION—II	TS - 301			62	GRASSING IN CONCRETE SQUARE GRID AND GRID BEAM	SP - 2	
./		TS - 302			64	VETIVER GRASSING FOR SLOPE PROTECTION	SP - 20	
9	TYPICAL ROADWAY CROSS - SECTION FOR R.O.W. WIDTH 20.00 M. TYPICAL ROADWAY CROSS - SECTION FOR R.O.W. WIDTH 30.00 M.	TS - 303 TS - 304			65	HYDROSEEDING FOR SLOPE PROTECTION	SP - 2	
10	TYPICAL ROADWAY CROSS - SECTION FOR R.O. W. WIDTH 40,00 M.	TS - 305			66	CONCRETE GRID BEAM BACK SLOPE PROTECTION	SP - 3	
10	23 CM. CONCRETE PAVEMENT	TS - 401			67	RENO MATTRESS SLOPE PROTECTION	SP - 4	401
12	25 CM, CONCRETE PAVEMENT	TS - 402			68	R.C. PIPE CULVERT	DS - 10	
13					69		DS - 10	
14	BRIDGE APPROACH SLAB AND POROUS BACKFILL MATERIALS CROSS-SECTION FOR DEEPCUT AND HIGH FILL	TS - 403 TS - 501			70	R.C. PIPE CULVERT INSTALLATION HEADWALL FOR R.C. PIPE CULVERT	DS - 10	
15/								
16	PAVEMENT TRANSITION DETAILS	TS - 601			71	R.C. HEADWALL FOR SINGLE R.C.P. CULVERT	DS - 10	55.67
17	TYPICAL SURFACE OVERLAY SECTION	TS - 602			72	R.C. HEADWALL FOR MULTIPLE R.C.P. CULVERT	DS - 10	
18 /	TYPICAL CLIMBING LANE SECTION	TS - 701			73	R.C. HEADWALL FOR SKEW R.C.P. CULVERT	DS - 10	
19 /	MINOR SIGN & SIGN POST DETAILS	RS - 101			74	SIDE DITCH LINING & DROP INLET CROSS DRAIN	DS - 2	
20/	TRAFFIC CONTROL DEVICES FOR HIGHWAY UNDER CONSTRUCTION - I	RS - 102			- 75	CONCRETE DITCH AT HILLSIDE	DS - 2	
21/	TRAFFIC CONTROL DEVICES FOR HIGHWAY UNDER CONSTRUCTION -I	RS - 103			76	R.C. DRAIN OUTLET FOR R.C.P. CULVERT	DS - 2	203
1000	TRAFFIC CONTROL DEVICES FOR HIGHWAY UNDER CONSTRUCTION -III	RS - 104			77	CATCH BASIN AT INLET FOR R.C.P. CULVERT	DS - 3	301
22/	TRAFFIC CONTROL DEVICES FOR HIGHWAY UNDER CONSTRUCTION - IX	RS - 105			78	ASPHALT CURB & DRAIN CHUTE FOR EMBANKMENT PROTECTION	DS - 3	502
23	OVERHEAD SIGN BOARD DETAILS	RS - 106			79	MANHOLE DETAILS -1	DS - 4	401
24	STEEL FRAME FOR MOUNTING OVERHEAD SIGN TYPE I SPAN 20.00 M. MAX.	RS - 107	14.73		. 80	MANHOLE DETAILS - II	DS - 4	402
25	STEEL FRAME FOR MOUNTING OVERHEAD SIGN TYPE I SPAN 17.00 - 28.00 M.	RS - 108			81	CATCH BASIN	DS - 4	403
26	STEEL POLE FOR OVERHANGING TRAFFIC SIGN FOR SIGN PLATES NOT MORE THAN 52,800 SQ.CM.	RS - 109	1.1		82	MEDIAN DROP INLET TYPE I	DS - 4	404
27	STEEL POLE FOR OVERHANGING TRAFFIC SIGN FOR SIGN PLATES NOT MORE THAN 108,000 SQ.CMI	RS - 110			83	MEDIAN DROP INLET TYPE II	DS - 4	405
28	STEEL POLE FOR OVERHANGING TRAFFIC SIGN FOR SIGN PLATES NOT MORE THAN 108,000 SQ.CM-II	RS - III			84	R.C. DITCH DETAILS - I	DS - 4	406
29	STEEL POLE FOR TWO LEGS OVERHANGING TRAFFIC SIGN FOR SIGN PLATES NOT MORE THAN 2x52,800 SQ.CMI	RS - 112			85	R.C. DITCH DETAILS - II	DS - 4	407
30	STEEL POLE FOR TWO LEGS OVERHANGING TRAFFIC SIGN FOR SIGN PLATES NOT MORE THAN 2x52,800 SQ.CMII	RS - 113	and the second	- 4	86	DETAIL OF INLET AND MANHOLE TYPE "E" & BOX CULVERT (OPEN TYPE)	ps - 5	501
31	INSTALLATION OF OVERHEAD SIGN AND FRAME TYPE II & TRAFFIC SIGN AND FRAME ON BARRIERS	RS - 114			87	DETAIL OF INLET AND MANHOLE TYPE "F" & BOX CULVERT (CLOSE TYPE)	DS - 5	502
32	STEEL FRAME FOR MOUNTING OVERHEAD SIGNS AT BRIDGE DECK	RS - 115			88	DROP INLET & U - DITCH FOR BRIDGE DRAINAGE	ps - 5	
33	PERMANENT TIMBER BARRICADE FOR T-INTERSECTION	RS - 201			89	CLEARING AND GRUBBING	MD - 10	101
34 /	W-BEAM GUARDRAIL	RS - 301			90	CONNECTION ROAD DETAILS	MD - 2	
35	W-BEAM GUARDRAIL INSTALLATION AND W-BEAM GUARDRAIL APPROACH TYPE I	RS - 302			91	SIDE ROAD & PRIVATE DRIVE DETAILS	MD - 2	
36 /	W-BEAM GUARDRAIL INSTALLATION AND W-BEAM GUARDRAIL APPROACH TYPE II	RS - 303			92	BUS STOP LAYOUT	MD - 3	W. S. C.
37 /	GUIDE POST INSTALLATION	RS - 401			93	BUS STOP SHELTER TYPE A	MD - 3	
38	CONCRETE BARRIER TYPE I	RS - 501			94	BUS STOP SHELTER TYPE B	MD - 3	1100
39	CONCRETE BARRIER TYPE II	RS - 502			95	BUS STOP SHELTER TYPE C-I	MD - 3	
40	CONCRETE BARRIER TYPE I FOR DEEP CUT AND HIGH FILL SECTION	RS - 503			96	BUS STOP SHELTER TYPE C-II	MD - 3	
41	PRE - CAST CONCRETE BARRIER TYPE I	RS - 504			97	BUS STOP SHELTER TYPE D-I	MD - 3	
42	PRE - CAST CONCRETE BARRIER TYPE II	RS - 505			98	BUS STOP SHELTER TYPE D-I	MD - 3	
43	CONCRETE BARRIER AT BRIDGE APPROACH	RS - 506	120		99	BUS STOP SHELTER TYPE E-I	MD - 3	
44 -	IMPLEMENT OF CABLE GUARDRAIL	RS - 507			100	BUS STOP SHELTER TYPE E-II	MD - 3	
45	CONCRETE CURB & CURB AND GUTTER DETAILS	RS - 508			101	BUS STOP SHELTER TYPE F -I	MD - 3	
46/	ROAD STUD	RS - 601			102	BUS STOP SHELTER TYPE F -II	MD - 3	
47	MARKING DETAILS - I	RS - 602			103	RIGHT - OF - WAY MONUMENT & BENCH MARK	MD - 4	
48 /	MARKING DETAILS-II	RS - 603			104	KILOMETER STONE		
49	RETAINING WALL — I	RS - 701			105	PLANTING SPACE ON SIDEWALK	MD - 4	
50	RETAINING WALL —II	RS - 702	10.7		106	PLANTING AND GRASSING IN MEDIANS	MD - 50	
51	ROAD SIGNS AT MEDIAN OPENING & EXIT AND ENTRANCE	RS - 801	100		107	MISCELLANEOUS LIGHTING DETAILS	MD = 60	752
52	ROAD SIGNS AT INTERSECTION	RS - 802			108	FENCING DETAILS	MD - 60	350
53/	SODDING DETAILS	SP - 101			109	CONCRETE PAVING BLOCK	200	
54	CONCRETE SLOPE PROTECTION	SP - 102	3.00				MD - 80	
55	SHOTCRETE BACK SLOPE AND SIDE SLOPE PROTECTION	SP - 102			111	CONCRETE PAVEMENT REPAIRING METHOD OF TRANSPLANTING TREE	MD - 80	
56	SACKED - CONCRETE SLOPE PROTECTION	SP - 104			112	MEDIAN OPENING	MD - 90	
					113	SPECIAL U - TURN DETAILS	MD - 90	70.0
		10				100		

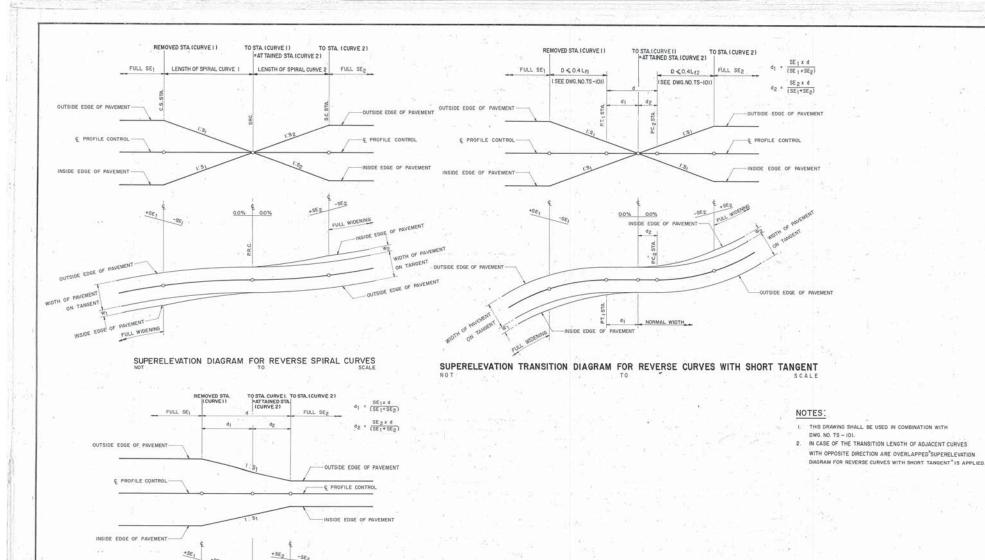
			ABBREVIATIONS		
Α	AREA	1.0.	INSIDE DIAMETER	900	COMPANY CAMPANY DAY AND AND
AASHTO	AMERICAN ASSOCIATION OF STATE HIGHWAY	IN.		R.C.B.	REINFORCED CONCRETE BOX CULVERT
	AND TRANSPORTATION OFFICIALS		INCH	R.C.P.	REINFORCED CONCRETE PIPE CULVERT
4.0		INV.	INVERT	RD.	ROAD
A.C.	ASPHALTIC CONCRETE	JT.	JOINT	R.E.	RESIDENT ENGINEER
ADT.	AVERAGE DAILY TRAFFIC	KG	KILOGRAM	REF.	REFERENCE
AGG.	AGGREGATE	KM.	KILOMETER	RENF.	REINFORCEMENT
AH.	AHEAD	KPH.	KILOMETER PER HOUR	REQ D	REQUIRED
AISC. ALT.	AMERICAN INSTITUTE CONSTRUCTION OF STEEL	KSC.	KILOGRAM PER SQUARE CENTIMETER	R.I.D.	ROYAL IRRIGATION DEPARTMENT
APPROX.	ALTERNATIVE	L.	LENGTH OF HORIZONTAL CURVE OR LENGTH	RP	REFERENCE POINT
ASTM.	APPROXIMATE	LEV.	LEVEL	RT;	RIGHT
ivg.	AMERICAN SOCIETY FOR TESTING AND MATERIALS  AVERAGE	LG.	LIGHT GAGE STEEL	R/W(OR R.O	W) RIGHT-OF-WAY
BIT.	BITUMINOUS	L.M.	LINEAR METER	S.B.S.T.	SINGLE BITUMINOUS SURFACE TREATMENT
ik.	BACK	LPS.	LOW PRESSURE SOOIUM LAMP	s.c.	SPIRAL - CIRCLE
IM.	BENCH MARK	Ls.	SUPERELEVATION TRANSITION LENGTH	SE.	SUPERELEVATION
OTT.	BOTTOM	L.S.	LUMP SUM	SEC. (OR Sec	
RDG.	BRIDGE	LT.	LEFT	SECT	SECTION
RG.	BEARING	LVC.	LENGTH OF VERTICAL CURVE	SP.	SPAN
т,	BACK TANGENT	M.	and made server	SPG.	SPACING
B.R.	CALIFORNIA BEARING RATIO	M. <sup>2</sup>	METER SQUARE METER	, S.R.	SIDE ROAD
/C(OR CTO C)	CENTER TO CENTER	M <sup>3</sup>	CUBIC METER	S.R.T.	STATE RAILWAYS OF THAILAND
	CLEARANCE	MAG. AZ.	MAGNETIC AZIMUTH	S.T.	SPIRAL - TANGENT
M.	CENTIMETER	MAX.	MAXIMUM	STA.	STATION
M <sup>2</sup>	SQUARE CENTIMETER	MEA.	METROPOLITAN - ELECTRICITY AUTHORITY	STD.	STANDARD
M.R	CORRUGATED METAL PIPE	M.H.	MANHOLE	STR.	STRAIGHT
DL.	COLUMN	MIN.	MINIMUM	SYMM.	SYMMETRY OR SYMMETRICAL
ONC.	CONCRETE	MISC.	MISCELLANEOUS	5/8	SOUTH BOUND
INSTR.	CONSTRUCTION	MM.	MILLIMETER	T. THK	TANGENT LENGTH , TON THICKNESS
P	CONCRETE PIPE	MM. <sup>2</sup>	SQUARE MILLIMETER	TIS.	THAT INDUSTRIAL STANDARD
s.	CIRCLE - SPIRAL	M.O.	MIDDLE ORDINATE	T.S.	TANGENT - SPIRAL
w	CARRIAGEWAY	MONT,	MONUMENT	TYP	TYPICAL
I.M.	CUBIC METER	M.S.L.	MEAN SEA LEVEL	VOL.	VOLUME
	DEGREE OF CURVE	N.	NORTH	V.C.	LENGTH OF VERTICAL CURVE
	DEFORMED BAR	N/B	NORTH BOUND	V.	VELOCITY
	DOUBLE BITUMINOUS SURFACE TREATMENT	NC.	NORMAL CROWN	w.	WIDENING
	DEGREE	NO.	NUMBER	W/	WITH
	DIAMETER DEPARTMENT OF HIGHWAYS	0.D.	OUTSIDE DIAMETER	W/B	WEST BOUND
	DRAWING	PB.M.	PERMAMENT BENCH MARK	W/0	WITHOUT
	EXTERNAL DISTANCE OF SIMPLE CURVE OR EAST	P.C.C.	POINT OF CURVE OR PRESTRESSED CONCRETE	WT.	WEIGHT
	EACH	P.C.C.	POINT OF COMPOUND CURVE	X SECT.	CROSS SECTION
(OR ELEV.)	ELEVATION	P.G.	PROVINCIAL-ELECTRICITY AUTHORITY PROFILE GRADE	E	CENTERLINE
	ENGINEER	P.G. LINE	PROFILE GRADE LINE	E	PROPERTY LINE
R.	EDGE OF PAVEMENT	P.I.	POINT OF HORIZONTAL INTERSECTION	£	SPUR LINE OR SURVEY LINE
(OR = )	EQUATION OR EQUAL	PL.	PLATE	%	PERCENT
	EQUIVALENT	PO.C.	POINT ON CURVE	8	AND
ST.	EXISTING	PO.S.T	POINT ON SUBTANGENT	@	SPACING.
	EXPANSION	RO.T.	POINT ON TANGENT		DIAMETER
	EAST BOUND	PR.C.	POINT OF REVERSE CURVE	Δ	DEFLECTION ANGLE
	FORWARD TANGENT	PROJ.	PROJECT		THE THE MANUE
	OOTING	P.T.	POINT OF TANGENT		
	RAM	PV.C.	POINT OF VERTICAL CURVE		
	ROUND LEVEL	PV.I.	POINT OF VERTICAL INTERSECTION		
	UB AND RED NAIL	PV.R.C.	POINT OF VERTICAL REVERSE CURVE		
	EADWALL	PVT.	POINT OF VERTICAL TANGENT		
	ORIZONTAL	R.	RADIUS		
	HIGH PRESSURE SODIUM LAMP	RB.	ROUND BAR		
100	IGH WATER LEVEL	R.C.	REINFORCED CONCRETE		
Y. F	IGHWAY	RC.	REMOVED ADVERSE CROWN		

			SYMBOLS		
CONST. &	_ CENTER LINE	<b>*</b>	TRAFFIC SIGNAL	9	PERMANENT BENCH MARK (P.B.M.)
SURVEY &	TRANSIT LINE	0	FIRE HYDRANT	4	POINT OF HORIZONTAL INTERSECTION
EXIST. R/W	_ EXISTING R/W	ļ	EXISTING PIPE CULVERT (WITH OR WITHOUT HEADWALL)		
PROPOSED R/W	PROPOSED R/W			Н	TEMPORARY SHELTER, HUT
е	_ PROPERTY LINE	>======================================	EXISTING BOX CULVERT		
_	_ EDGE OF NEW PAVEMENT	)00000	NEW BOX CULVERT	1-S-W	BUILDING, ONE - STOREY WOODEN FRA
	_ EDGE OF EXISTING PAVEMENT	000	EXISTING HIGHWAY GUARDRAIL		
	EXISTING SHOULDER LINE		NEW HIGHWAY GUARDRAIL	2-S-W	BUILDING, TWO - STOREY WOODEN FRAM
	. NEW SHOULDER LINE -	Ĭ	RAIL ROAD CROSSING SIGN	1-S-C	
	EXISTING CURB	0 *	RAIL ROAD CROSSING SIGNAL LIGHT		BUILDING, ONE - STOREY CONCRETE
	NEW CURB		RAIL ROAD CROSSING GATE	2-s-c	The second secon
	. GROUND PROFILE	00	GASOLINE STATION		BUILDING, TWO - STOREY CONCRETE
	. EXISTING ROAD PROFILE	350	INDEX CONTOUR	1- S- W (10)	COMMERCIAL BUILDING, ONE - STORE
	BACKFILLING EXISTING CHANNELS		INTERMEDIATE CONTOUR		WOODEN FRAME, 10 UNITS
	EXISTING INLETS	-* <u>\</u>	WOODEN OR BARBED WIRE FENCE	2-S-W(10)	COMMERCIAL BUILDING, TWO - STOREY
	EXISTING PIPE		BRIDGE, ROAD OVER STREAM		WOODEN FRAME, 10 UNITS
======	EXISTING DITCH	$\Rightarrow$	RAIL ROAD SINGLE TRUCK	1-S-C (10)	COMMERCIAL BUILDING , ONE - STOREY
	PARALLEL DITCH		KHLONG OR RIVER		CONCRETE STRUCTURE, 10 UNITS
	PIPES	-	DITCH , WATERWAY	2-s-c (10)	COMMERCIAL BUILDING, TWO - STOREY CONCRETE STRUCTURE, 10 UNITS
	INLET	~-	FLOW DIRECTION		CONCRETE STRUCTURE, 10 UNITS
wo	WATER & WATER VALVE	======================================	SWAMP AREA	1 - S-W	SCHOOL BUILDING , ONE - STOREY WOODEN FRAME
o	TELEPHONE & MANHOLE	MM	BILLBOARD , SINGLE FACE		HOUSEN FRANKE
E0	ELECTRICITY	MM	BILLBOARD, DOUBLE FACE	2 - S - W	SCHOOL BULLDING, TWO - STOREY WOODEN FRAME
PTPT	POWER TRANSMISSION LINE WITH STEEL TOWER	φ	POWER POST OR TELEPHONE POLE		
•	WOODEN ELECTRIC POLE		TRAFFIC SIGN	1 - S - C	SCHOOL BUILDING , ONE - STOREY CONCRETE STRUCTURE
	CONCRETE ELECTRIC POLE		GUIDE POST		era annetifetati i ettetti tili 1770 c
0	INDIVIDUAL TREE	<u>-</u> ∇	HIGH WATER LEVEL	2-S-C	SCHOOL BUILDING, TWO - STOREY CONCRETE STRUCTURE
0	HEDGE		KILOMETER MARKER (EXISTING, PROPOSED)		
	BUS STOP SHELTER	ο .	R/W MARKER (EXISTING, PROPOSED)		
	TELEPHONE JUNCTION		RAILWAY		
	PUBLIC TELEPHONE BOX	-11111	SLOPE		
	MAIL BOX	•	BENCH MARK		

# KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS STANDARD DRAWING ABBREVIATION AND SYMBOLS DESIGNED: D.O.H. & CONSULTANTS CHECKED: DATE JULY 1994 SUBMITTED: DESIGNED: DOWN DATE OF LOCATION A DEBIGN DIVISION DIVISIONE DI PROPRIME DI







OUTSIDE EDGE OF PAVEMENT

SUPERELEVATION TRANSITION DIAGRAM FOR COMPOUND CURVES

FULL WIDENING

FULL WIDENING

WIDTH OF PRIEMEN

INSIDE EDGE OF PAVEMENT

### KINGDOM OF THAILAND

MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

METHOD OF ATTAINING SUPERELEVATION AND WIDENING OF PAVEMENT FOR COMPOUND AND REVERSE CURVES

SUBMITTED: SALE NONE

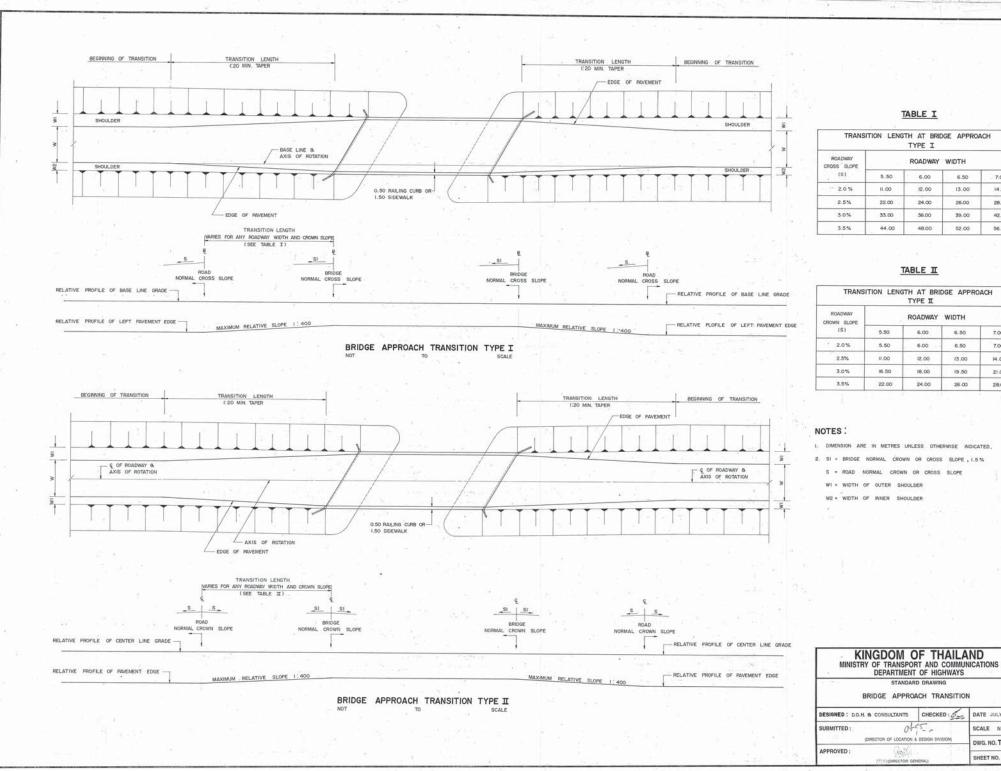
APPROVED: FREE PROTOGRAPH OF STANDING

APPROVED: SALE NONE

SALE NONE

DWG. NO.TS - 103

SHEET NO. 4



7.00

14.00

28.00

42.00

56.00

7.00

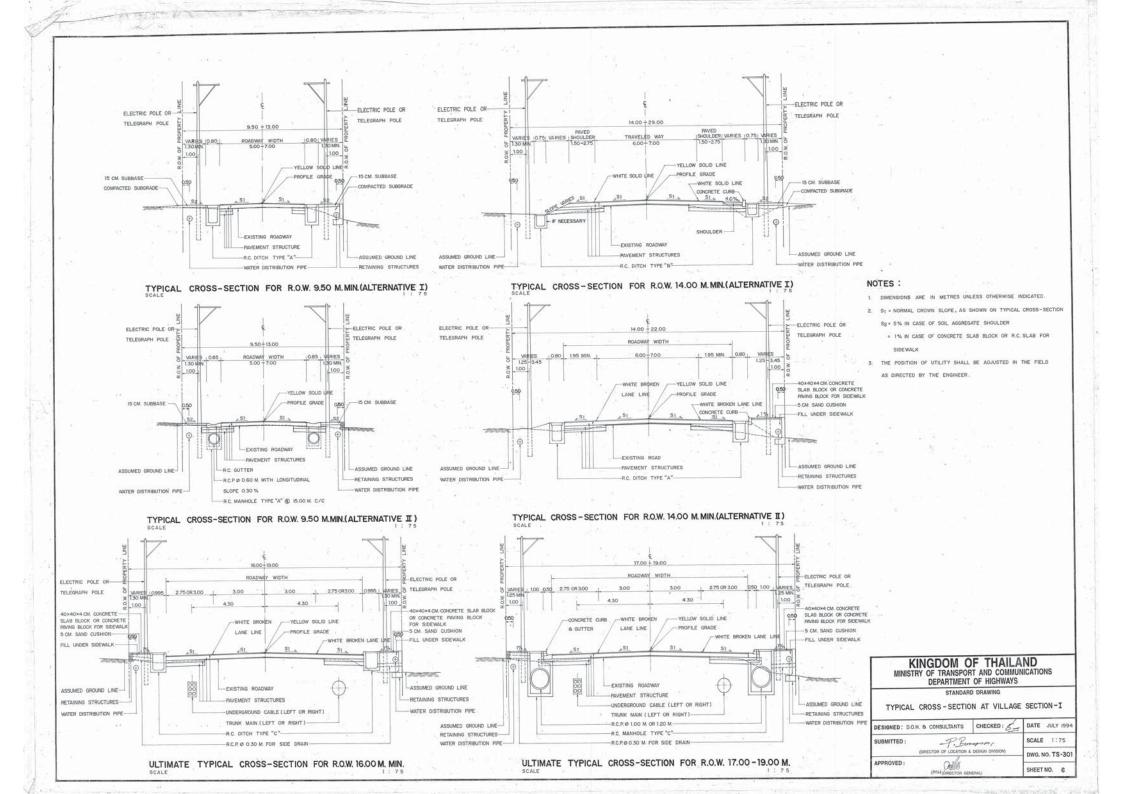
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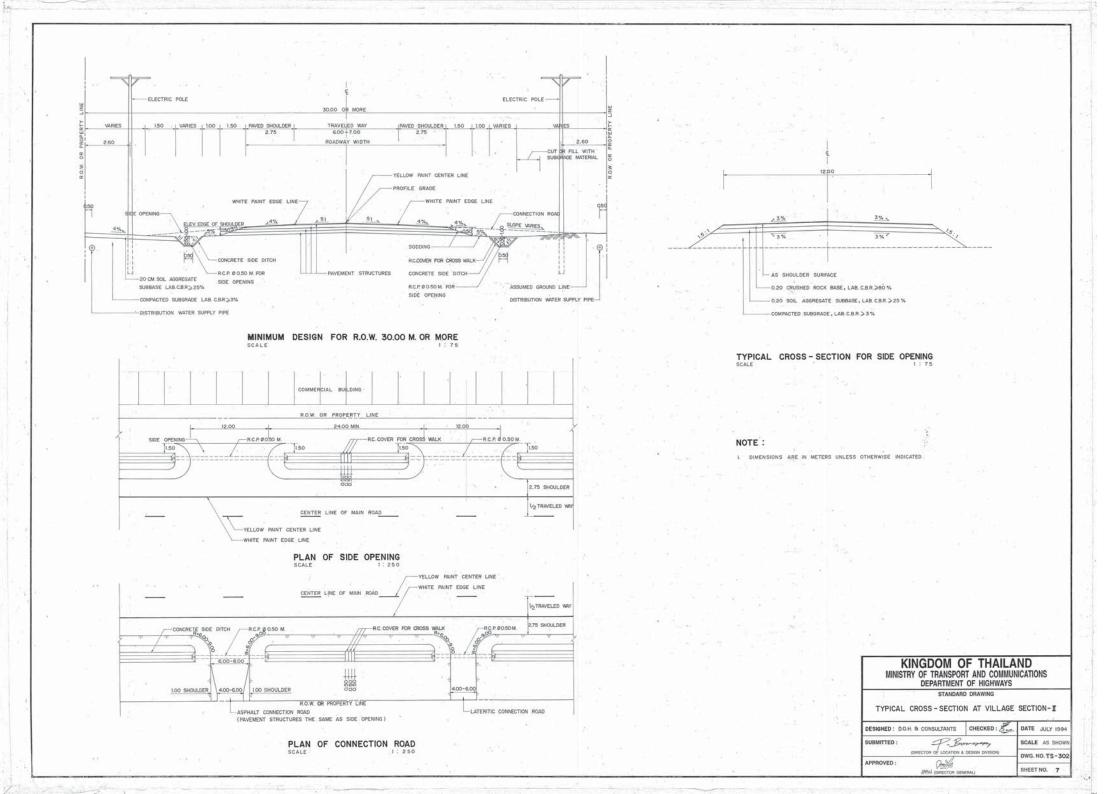
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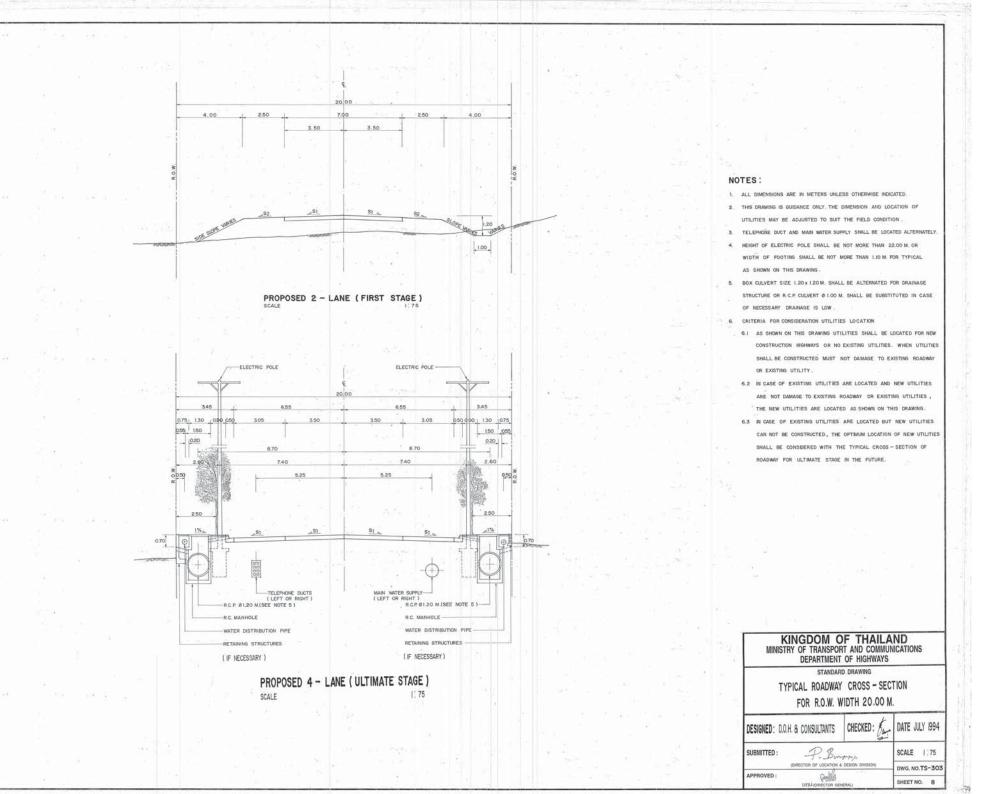
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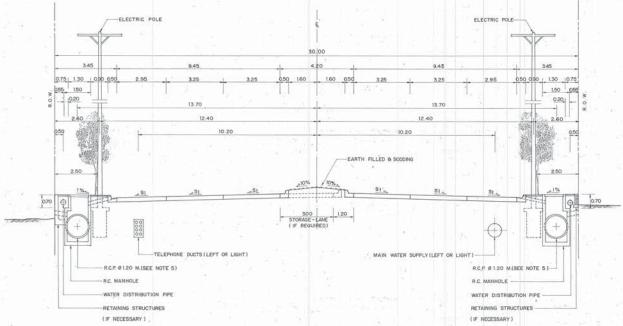
DATE JULY 1994 SCALE NONE DWG. NO. TS-201







### 3.90 2.50 6.50 6.50 050 1.60 1,60 0.50 EARTH FILLED & SODDING 3.00 1.20 STORAGE - LANE (IF REQUIRED) 1.00 PROPOSED 4-LANE DIVIDED (FIRST STAGE) -ELECTRIC POLE ELECTRIC POLE-



PROPOSED 4-LANE DIVIDED (ULTIMATE STAGE)

### NOTES:

- 1. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- 2. THIS DRAWING IS GUIDANCE ONLY. THE DIMENSION AND LOCATION OF UTILITIES MAY BE ADJUSTED TO SUIT THE FIELD CONDITION .
- 3. TELEPHONE DUCT AND MAIN WATER SUPPLY SHALL BE LOCATED ALTERNATELY.
- HEIGHT OF ELECTRIC POLE SHALL BE NOT MORE THAN 22.00 M. OR WIDTH OF FOOTING SHALL BE NOT MORE THAN I.IO M. FOR TYPICAL AS SHOWN ON THIS DRAWING.
- 5. BOX CULVERT SIZE 1.20 x 1.20 M. SHALL BE ALTERNATED FOR DRAINAGE STRUCTURE OR R.C.P. CULVERT Ø LOO M. SHALL BE SUBSTITUTED IN CASE OF NECESSARY DRAINAGE IS LOW
- 6. CRITERIA FOR CONSIDERATION UTILITIES LOCATION
  - 6.1 AS SHOWN ON THIS DRAWING UTILITIES SHALL BE LOCATED FOR NEW CONSTRUCTION HIGHWAYS OR NO EXISTING UTILITIES. WHEN UTILITIES SHALL BE CONSTRUCTED MUST NOT DAMAGE TO EXISTING ROADWAY OR EXISTING UTILITY
  - 6.2 IN CASE OF EXISTING UTILITIES ARE LOCATED AND NEW UTILITIES ARE NOT DAMAGE TO EXISTING ROADWAY OR EXISTING UTILITIES. THE NEW UTILITIES ARE LOCATED AS SHOWN ON THIS DRAWING.
  - 6.3 IN CASE OF EXISTING UTILITIES ARE LOCATED BUT NEW UTILITIES CAN NOT BE CONSTRUCTED, THE OPTIMUM LOCATION OF NEW UTILITIES SHALL BE CONSIDERED WITH THE TYPICAL CROSS - SECTION OF ROADWAY FOR ULTIMATE STAGE IN THE FUTURE



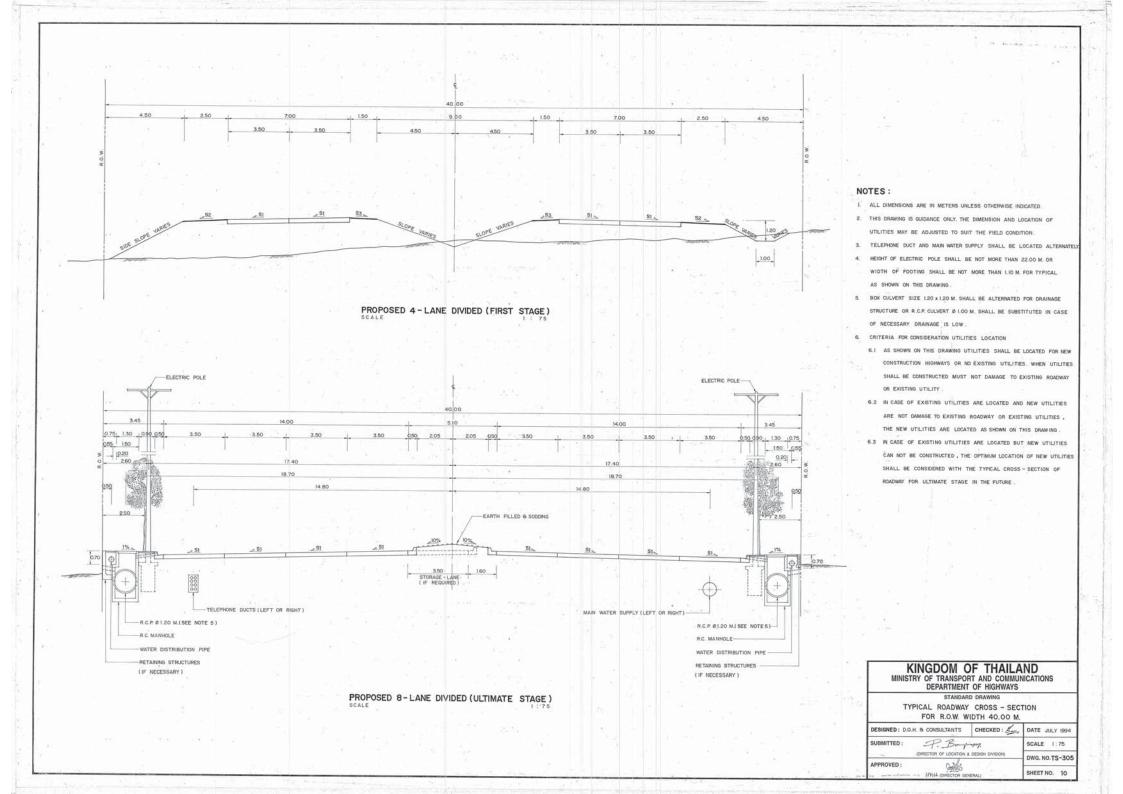
STANDARD DRAWING

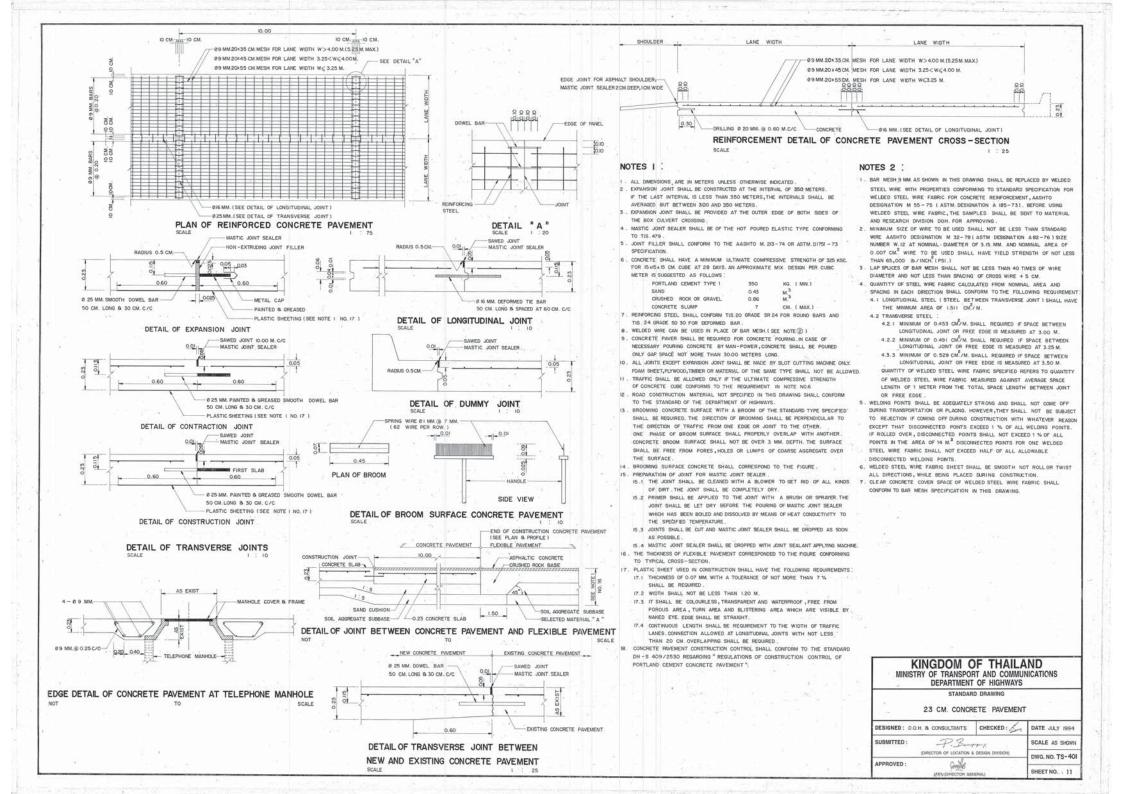
TYPICAL ROADWAY CROSS - SECTION FOR R.O.W. WIDTH 30.00 M.

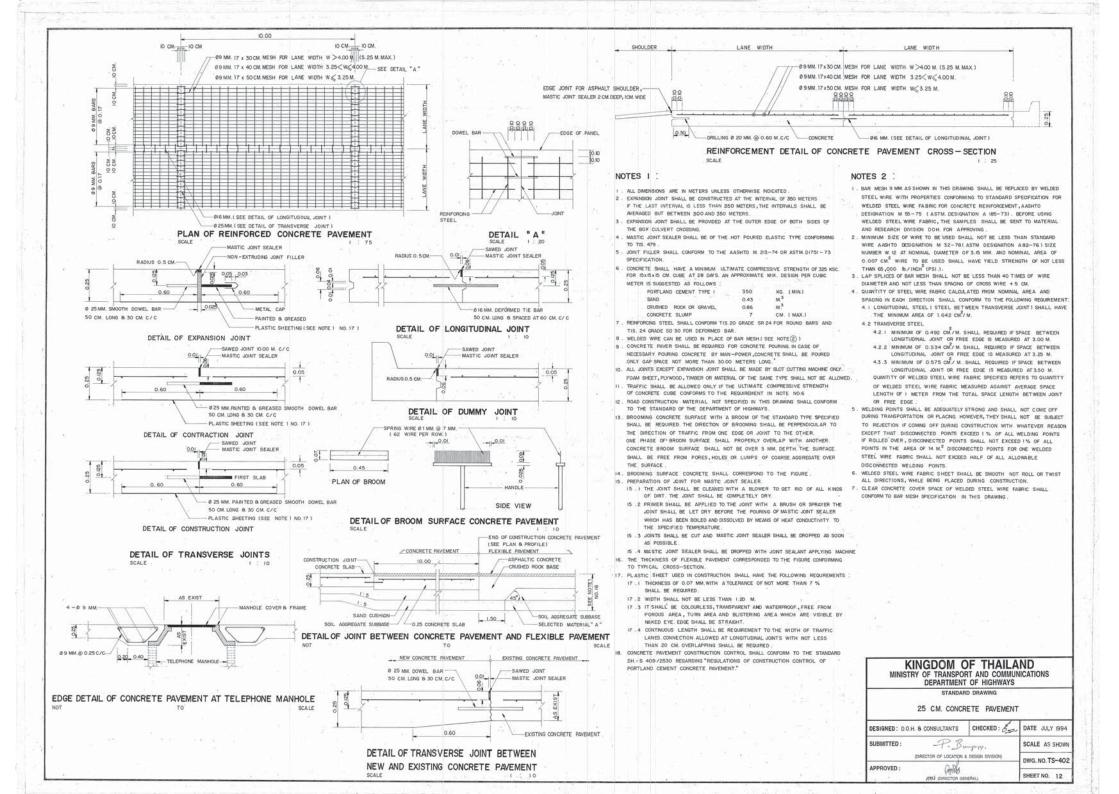
DESIGNED : D.O.H. & CONSULTANTS | CHECKED : 6 DATE JULY 1994 P. Burpay.

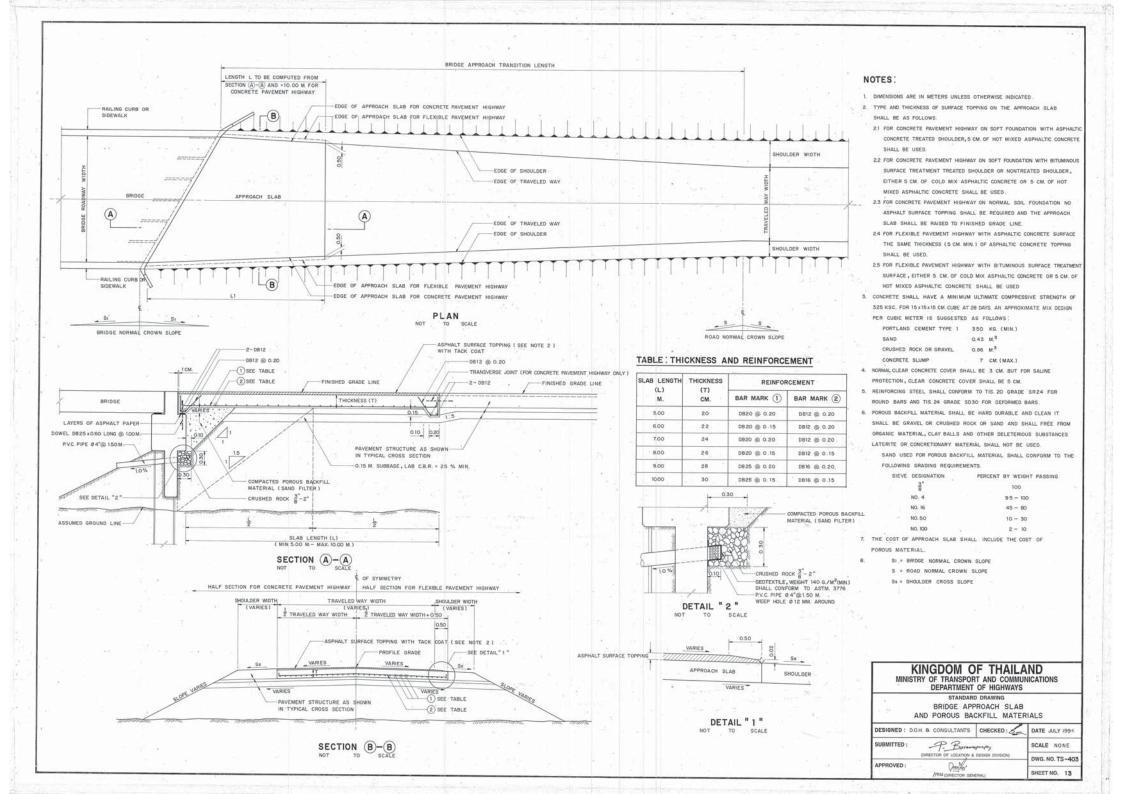
SCALE 1:75 DWG. NO.TS-304

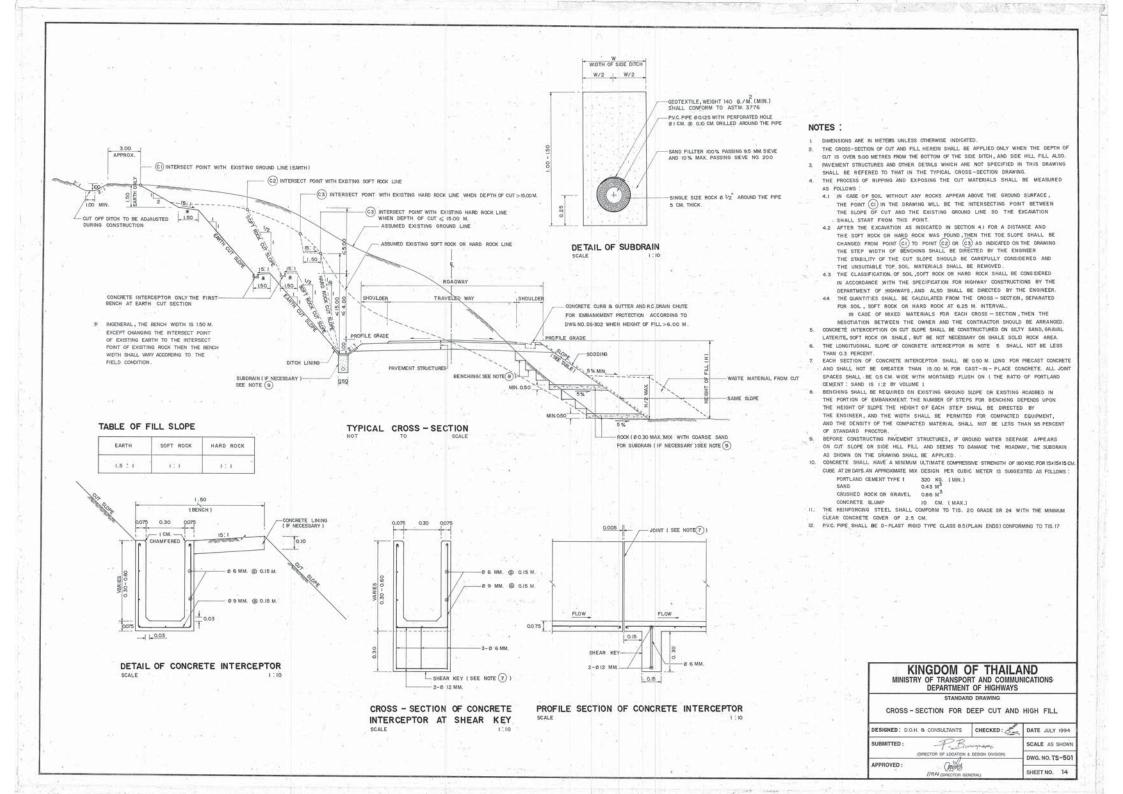
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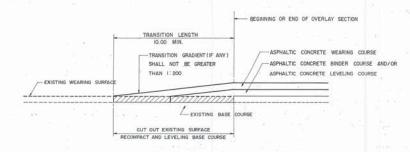




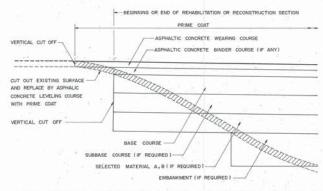




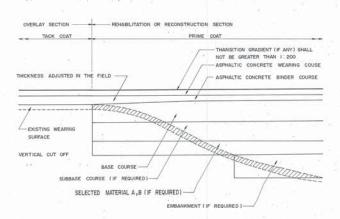




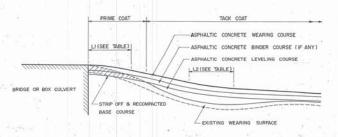
### TRANSITION OF OVERLAY SECTION TO EXISTING ROAD



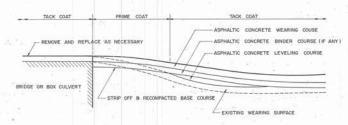
### TRANSITION OF REHABILITATION OR RECONSTRUCTION TO EXISTING ROAD



TRANSITION OF OVERLAY SECTION TO REHABILITATION OR RECONSTRUCTION SECTION TO SCALE



### CASE I : PAVE TO EXISTING STRUCTURE



CASE 2 : PAVE OVER EXISTING STRUCTURE

OVERLAY AT BRIDGE / BOX CULVERT APPROACH

TABLE .

ALGEBRAIC DIFFERRENT OF PROPOSED GRADE	LENGTH OF CREST CURVE, LI	LENGTH OF SAG CURVE, LE
0 - 2 %	10	20
2 - 4%	20	35
4 - 6%	30	50
6 - 8%	40	65

#### NOTES

- THE DETAILS AT LEFT ARE NOT TO SCALE AND ARE EXAGGERATED IN ORDER TO SHOW DETAIL THE ENGINEER WILL MAKE NECESSARY.
- PRIME COAT SHALL BE AFFLIED TO A COMPACTED BASE AFTER STREPPING OFF OF THE EXISTING SURFACE APPLICATION RATE SHALL BE KEPT TO A MINIMUM DEPENDING ON THE POROSITY AND TEXTURE OF THE EXPOSED BASE COURSE.
- 3. TACK COAT SHALL BE APPLIED TO EACH LAYER OF ASPHALTIC CONCRETE
- 4. WHERE POSSIBLE, ASPHALTIC LEVELING COURSE SHALL BE
  PLACED IN LAYERS BY AN ASPHATIC FINISHER COMPACTED
  IN THE GENERAL SPECIFICATION.

KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

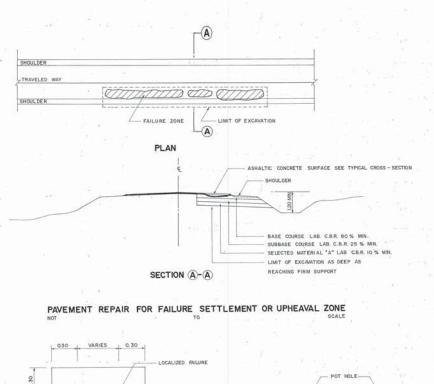
PAVEMENT TRANSITION DETAILS

DESIGNED: D.O.H. & CONSULTANTS CHECKED: DATE JULY 1994

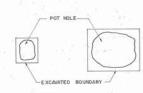
SUBMITTED: SCALE NONE

DIRECTOR OF LOCATION & DESIGN EMISSION

DWG. NO. TS-601



LIMIT OF EXCAVATION



PLAN

ASPHALTIC CONCRETE LEVELING OR BINDER COURSE (REPLACE)

BASE MATERIAL (REPLACE)

SUBBASE MATERIAL (REPLACE)

EXIST SUBBASE

EXIST SUBBASE

EXPOSED SURFACE OF POT HOLE (SEE NOTE 6.)

SECTION LOCALIZED FAILURE

PAVEMENT REPAIR FOR LOCALIZED FAILURE AND POT HOLE

LIMIT OF EXCAVATION AS DEEP AS REACHING FIRM SUPPORT

SUBGRADE FAILURE OR SOFT SPOT

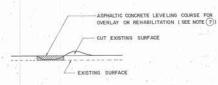
SELECTED MATERIAL "A" LAB C.BR. 10 % MIN.

ASPHALTIC CONCRETE LEVELING COURSE FOR OVERLAY OR REHABILITATION (SEE NOTE(7))

EXISTING SURFACE EXISTING SHOULDER

### LONGITUDINAL DEPRESSION

SHOVING



RAVELING

PAVEMENT PATCHING FOR DEPRESSION CORRUGATION, SHOVING AND RAVELING

#### REPAIR OF CRACKS

- ALLIGATOR CRACKS ARE NORMALLY CAUSED BY EXCESSIVE DEFLECTION OF THE SUBFACE OVER UNSTABLE LOWER COURSES OF THE PAWEMENT ALLIGATOR CRACKS SHOULD THEN BE TREATED AND REPAIRED AS LOCALIZED FAILURE
- REFLECTION CRACKS AND SHRINKAGE CRACKS SHALL SEALED BY EITHER TACK COAT OF A HIGHER APPLICATION RATES OR ASPHALTIC SLURRY SEAL AS DIRECTED BY THE ENGINEER.
- 3. SLIPPAGE CRACKS ARE CAUSED BY THE LACK OF A GOOD BOND BETWEEN THE SURFACE LAYER AND THE COURSE BENEATH REMOVE THE SLIPPING AREA, CLEAN THE SURFACE OF THE EXPOSED UNDERLYING LAYER AND APPLY A SUITABLE APPLICATION RATE OF TACK COAT AS DIRECTED BY THE ENDINEER.

#### NOTES :

L FOR CONTINUOUS FAILURE SECTIONS AND LOCALIZED FAILURE, FAILURE ZONES SHALL BE EXCAMIED TO THE UNDETERIORATED LAYER UNSUITABLE MATERIAL OR SOFT SPOT SHALL BE REMOVED TO, THE SATISFACTION OF THE ENGINEER.

EXISTING SURFACE

CORRUGATION

ASPHALTIC CONCRETE LEVELING COURSE FOR

OVERLAY OR REHABILITATION (SEE NOTE 7)

ASPHALTIC CONCRETE LEVELING COURSE FOR OVERLAY OR REHABILITATION ( SEE NOTE (7) )

- EXISTING SURFACE

- A SOFT SPOT IN AN EXISTING ROAD CAN BE VISUALLY DETERMINED BY THE MOVEMENT OF THE PAVEMENT UNDER PROOF ROLLING.
- THE RECONSTRUCTION OF PAVEMENT STRUCTURE SHALL BE DONE LAYER BY LAYER TO THE REQUIRED THICKNESS SUITABLE MATERIALS MAY BE REUSED. ALL MATERIALS SHALL BE IN ACCORDANCE WITH THE DRAWING AND THE SPECIFICATION.
- 4. BENCHING INTO SIDE SLOPES SHALL BE DONE WITH CARE IN ORDER TO PREVENT THE EXISTING PAVEMENT FROM COLLAPSING UNDER TRAFFIC CONDITIONS.
- 5. THE NEED FOR SIDE DITCHES FOR PAVEMENT SUBSURFACE DRAINAGE SHALL BE VERFIED BY THE ENGINEER AND EXCAMION SHALL COMMENCE AS EARLY IN THE CONTRACT AS POSSIBLE TO REDUCE THE EXCESS WATER CONTENT FROM THE ROAD STRUCTURE.
- 6. FOR POT MOLES, THE HOLE SHALL BE SQUARED, DRESSED AND DATCHED AS SHOWN USING GOOD FIELD PRACTICE AND IN ACCORDANCE WITH THE SPECIFICATIONS.
- FOR LONGITUDINAL DEPRESSION, CORRUGATION, SHOVING AND PITTING THE EXISTING SURFACE SHALL BE THOROGHLY, CLEANED AND LOOSE GRAVEL REMOVED HOMPS IN THE EXISTING SURFACE SHALL BE CUT TO THE ADJACENT SURFACE LEVEL PRIOR TO OVERLAY OR REHABILITATION.
- B. TACK COAT APPLIED TO PAVEMENT CRACKS OR PITTING AREA SHALL BE ADJUSTED HIGHER APPLICATION RATES IF DIRECTED BY THE ENGINEER.
- BLEEDING SURFACE SHALL BE REMOVED AS APPLICABLE OR SEALED WITH ASPHALTIC CONCRETE LEVELING OR BINDER COURSE THE ASPHALT CONTENT SHALL BE KEPT AT A SULTABLE RATE AS APPROVED BY PROJECT ENGINEER.
- IO. TRAFFIC SIGN, BARRICADES, LIGHTS, ETC. SHALL BE PROVIDED AT EACH UNCOMPLETED REPAIR SECTIONS FOR SAFTY MEASURES.

KINGDOM OF THAILAND
MINISTRY OF TRANSPORT AND COMMUNICATIONS
DEPARTMENT OF HIGHWAYS

TYPICAL SURFACE OVERLAY SECTION

DESIGNED: D.O.H. & CONSULTANTS CHECKED: DATE JULY 1994

SUBMITTED: SCALE NONE

DIRECTOR OF LOCATION & DESIGN DIVISION

APPROVED: SHEET NO. 16

### CRITICAL LENGTH OF GRADE (SEE NOTE 3) 60.00 M. (MINIMUM) 60.00 M. (MINIMUM.) OR TAPER 50: I(DESIRABLE L<sub>2</sub> (DESIRABLE ) PLAN THE CREST POIN DOWN GRADE PROFILE FOR VERTICAL CURVE TYPE I (SEE NOTE 3) FOR VERTICAL CURVE TYPE I (SEE NOTE 3) CLIMBING LANES FOR TWO-LANE HIGHWAYS SPEED REDUCTIO 2,000 2,500 LENGHT OF GRADE (FEET) 500 1,000 1,500 700 LENGHT OF GRADE (METER) 400 500 600 CRITICAL LENGTHS OF GRADE FOR DESIGN, ASSUMED TYPICAL HEAVY TRUCK OF 300 Ib/HP, ENTERING SPEED = 55 MPH

#### CLIMBING LANES FOR TWO - LANE HIGHWAYS

CONDITION AND CRITERIA

THE FOLLOWING THREE CONDITIONS AND CRITERIA , REFLECTING ECONOMIC CONSIDERATION , SHOULD BE SATISFIED TO JUSTIFY A CLIMBING LANE :

- 1.1 UPGRADE TRAFFIC FLOW RATE IN EXCESS OF 200 VEHICLES PER HOUR.
  1.2 UPGRADE TRUCK FLOW RATE IN EXCESS OF 20 VEHICLES PER HOUR.
- 1.3 ONE OF THE FOLLOWING CONDITIONS EXISTS :
  - A 10 MPH.(16 KPH.) OR GREATER SPEED REDUCTION IS EXPECTED
    FOR A TYPICAL HEAVY TRUCK.
  - LEVEL OF SERVICE E OR F EXISTS ON THE GRADE
  - A REDUCTION OF TWO OR MORE LEVELS OF SERVICE IS EXPERIENCED
    WHEN MOVING FROM THE APPROACH SEGMENT TO THE GRADE.
- THESE GENERAL GUIDES FOR THE CONSIDERATION OF CLIMBING LANES ON GRADES WOULD APPLY ONLY TO CLIMBING LANES ON TWO - LANE HIGHWAYS AND SHOULD NOT BE USED IN CONJUNCTION WITH CONSIDERATION OF CLIMBING LANES ON MULTILANE HIGHWAYS.
- THE CRITICAL LENGTH OF GRADE IN FIGURE IS DERIVED AS THE LENGTH OF TANDENT GRADE. WHERE A VERTICAL IS PART OF A CRITICAL LENGTH OF GRADE, AN APPROXIMATE EQUIVALENT TANDENT GRADE LENGTH MUST BE USED. WHERE THE CONDITION INVOLVES VERTICAL CURVE TYPE I AND THE ALGEBRAC DIFFERENCE IN GRADES IS NOT TOO GREAT, THE MEASUREMENT OF CRITICAL LENGTH OF GRADE MAY BE MADE BETWEEN THE POINT OF VERTICAL INTERSECTION (PVI.). WHERE VERTICAL CURVE OF TYPE I IS INVOLVED. PARTICULARLY WHERE THE ALGEBRAIC DIFFERENCE IN GRADE IS APPRECUABLE. ABOUT ONE "QUARTER OF THE VERTICAL CURVE LENGTH MAY BE CONSDERED AS PART OF THE GRADE UNDER CONSIDERATION."
- . THE BEGINNING POINT OF CLIMBING LANE IS THE POINT WHERE A IO MPH.

  (16 KPH.) DECREASE IN TRUCK SPEED BELOW THE OPERATING SPEED, AS
  "CRITICAL LENGTHS OF GRADE".
- 5. THE END POINT OF CLIMBING LANE IS THE POINT WHERE THE TRUCK CAN RETURN TO THE NORMAL LANE WITHOUT UNDUE HAZARD IN PARTICULAR, WHERE THE SIGHT DISTANCE BECOMES SUFFICIENT TO PERMIT PASSING WITH SAFETY WHEN THERE IS NO ONCOMING TRAFFIC OR PREFERBLY; AT LEAST 60.00 M. BEYOND THE CREST POINT.

#### CLIMBING LANES ON MULTILANE HIGHWAYS

- I, CONDITION AND CRITERIA
- I.I IF THE CRITICAL LENGTH OF GRADE, THE LENGTH OF A PARTICULAR UPGRADE WHICH REDUCES THE OPERATING SPEED OF TRUCKS 10 MPH. (16 KPH.) BELOW THE OPERATING SPEED OF THE REMAINING TRAFFIC. IS LESS THAN THE LENGTH OF GRADE BEING STUDIED A CLIMBING LANE IS WARRANTED.
- 1.2 IF THE FLOW RATE ON THE GRADE DETERMINED BY DIVIDING THE UPGRADE DOWN (THE DIRECTIONAL DISTRIBUTION OF TRAFFIC ON MULTILANE FACILITIES DURING THE DESIGN HOUR) BY THE PEAK HOUR FACTOR (PHF) EXCEEDS THAT OF THE NEXT POORER LEVEL-OF-SERVICE, A CLIMBING LANE IS WARRANTED.

GENERALLY, CLIMBING LANES SHOULD NOT BE CONSIDERED UNLESS THE UPGRADE DIRECTIONAL TRAFFIC VOLUME IS EQUAL TO OR GREATER THAN THE LEVEL- OF-SERVICE D SERVICE VOLUME FOR TWO OF THE LANES OF A FOUR-LANE HIGHWAY WITH THE NORMAL PERCENTAGE OF TRUCKS.

- 2. IN MOST CASES WHEN THE SERVICE VOLUME, INCLUDING TRUCKS, IS GREATER THAN 1,700 VPH, AND THE LENGTH OF THE GRADE AND THE PERCENTAGE OF TRUCKS ARE SUFFICIENT TO CONSIDER CLIMBING LANES, THE VOLUME IN TERMS OF EQUIVALENT PASSENGER CARS IS LIKELY TO APPROACH OR EVEN EXCEED THE CAPACITY. AN INCREASE IN THE NUMBER OF LANES THROUGHOUT THE HIGHWAY SECTION WOULD REPRESENT A BETTER INVESTMENT THAN THE PROVISION OF CLIMBING LANES.
- A CLIMBING LANE IS ALSO GENERALLY NOT WARRANTED ON FOUR-LANE HIGHWAYS FOR A DIRECTIONAL SERVICE VOLUME BELOW LOOD VPH. REGARDLESS OF THE PERCENTAGE OF TRUCKS.
- . WHEN THE ANALYSIS NOICATES THAT A CLIMBING LANE MAY IMPROVE OPERATIONS OR MAINTAIN THE LEVEL- OF-SERVICE ON THE SECTION WITH A GRADE AN ADDITIONAL CHECK MUST BE MADE TO DETERMINE IF THE NUMBER OF LANES REQUIRED ON THE GRADE ARE SUFFICIENT EVEN WITH A CLIMBING LANE.
- 5. THE PRINCIPLES FOR CROSS SLOPES, FOR LOCATING TERMINAL POINTS,
  AND FOR DESIGNING TERMINAL AREAS OR TAPERS FOR CLIMBING LANES
  ON TWO-LANE HIGHWAYS ARE EQUALLY APPLICABLE TO CLIMBING LANES
  ON MULTILANE FACILITIES. THE PRIME FACTOR BEING THAT OF DETERMINAD
  THE LOCATION OF THE UPHILL TERMINUS OF THE CLIMBING LANE AT THE
  POINT WHERE A SATISFACTORY SPEED IS ATTAINED BY TRUCKS, PREFERABLY
  ABOUT IO MPH, (16 KPH.) BELOW THE OPERATING SPEED OF THE HIGHWAY.
  PASSING SIGHT DISTANCE NEED NOT BE CONSIDERED.

KINGDOM OF THAILAND
MINISTRY OF TRANSPORT AND COMMUNICATIONS
DEPARTMENT OF HIGHWAYS
STANDARD DRAWING

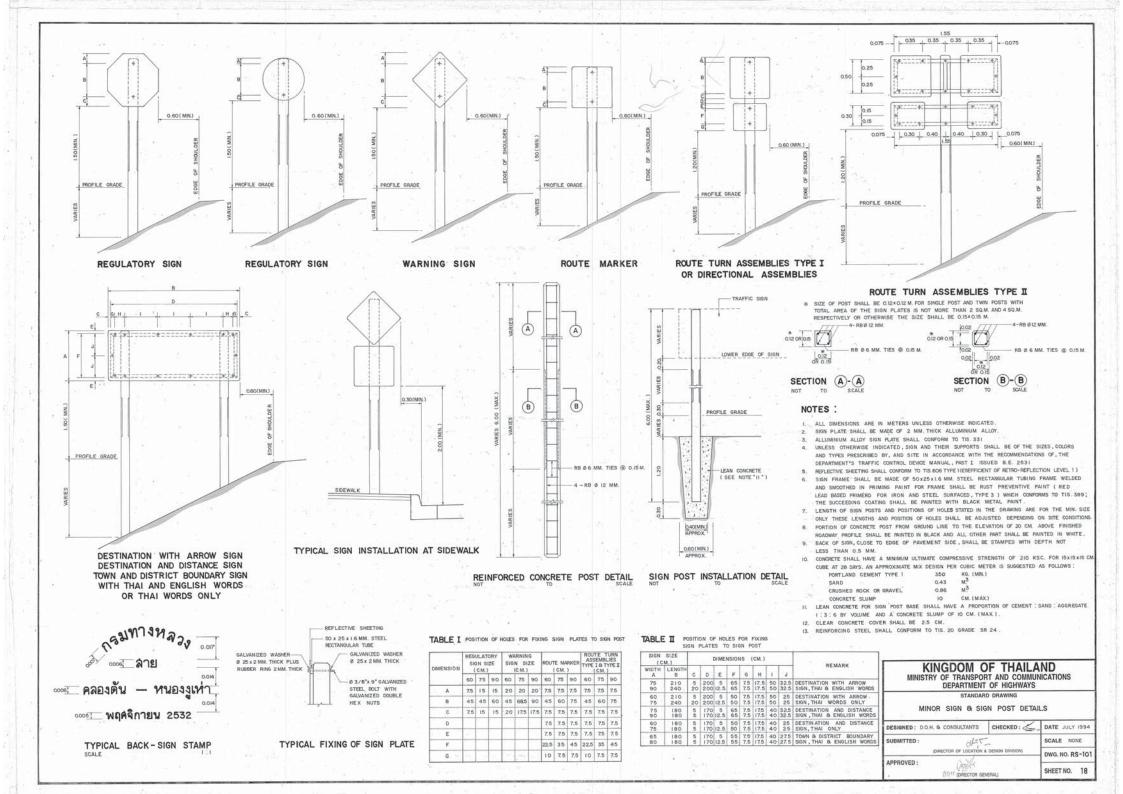
TYPICAL CLIMBING LANE SECTION

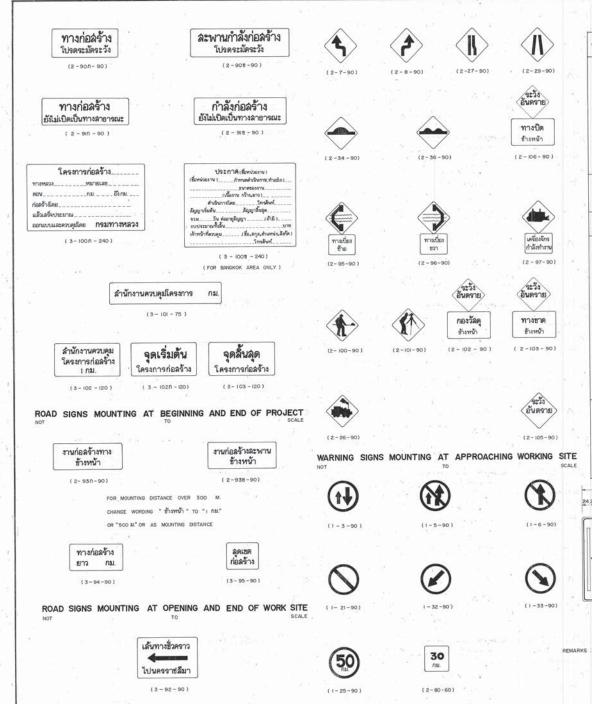
DESIGNED: D.O.H. & CONSULTANTS CHECKED: DATE JULY 1994

SUBMITTED: SCALE NONE

APPROVED: DWG.NO.TS-701

78/2 (DIRECTOR GENERAL)





TEMPORARY ROAD GUIDE SIGN

TO

SIGNS TO CONTROL SPEED

		SIZE	LETTER	R SIZE	The superstanting of the super	REFLETO-	COLOUR
TRAFFIC SIGN CONTROL	LED PATTERN	(CM.)	1 ST ROW	2ND ROW	- RECOMMENDED LOCATION	PROPERTIES	BOARD
11	2-900-90	90x240	20	15	FOR EXISTING ROAD CONSTRUCTION	REFLECT	RED - ORAM
	2 - 905 - 90	90×240	20	15	FOR EXISTING ROAD CONSTRUCTION	REFLECT	RED-ORAN
	2 - 9IN - 90	90 x 240	20	15 ,	FOR NEW ROAD CONSTRUCTION THAT ALLOWS SOME TRAFFIC		RED - ORAN
	2 - 918 - 90	90x 240	20	15	FOR NEW ROAD CONSTRUCTION THAT ALLOWS SOME TRAFFIC		WHITE
ROAD SIGNS MOUNTING	3 -100ft-240 3 -100ft-240		FOR IST-2ND RO		MOUNTING AT BEGINNING OF PROJECT MOUNTING AT BEGINNING OF PROJECT		WHITE
AT BEGINNING AND END OF PROJECT	3-1008-240	2401360	AND	M	MOUNTING AT BEGINNING OF PROJECT		
END OF PROJECT		- (	กรมทางหล	วร์ 15	The second secon		
		1	OTHER	20			
	3-101-75	75 x 360	20	- 1	MOUNTING WITH 3-1000 -240 SIGN BOARD		WHITE
	12 10	and the second	IST-2ND		MOUNTING ALONE		WHITE
			3RD RO			REFLECT	RED - ORA
	3-102n-120		25 25	15	MOUNTING AT BEGINNING OF PROJECT MOUNTING AT END OF PROJECT	REFLECT	RED - ORA
	3-103-120	120 x 240	25	15	MOUNTING AT END OF PROSECT		1000
ROAD SIGNS MOUNTING	2-930-90	90 x 240	20	20	MOUNTING AT 200 M. BEFORE APPROACHING	REFLECT	RED - ORA
AT OPENING WORK	5 - 934 - 80	90×240	20	20	MOUNTING AT 200 M. BEFORE APPROACHING	REFLECT	RED - ORA
SITE	3-94 -90		20	20	MOUNTING AT 200. M. BEFORE APPROACHING MOUNTING AT END OF WORK SITE	REFLECT	RED - ORA
	3-95 -90	75 x 120	15	15	MOUNTING AT END OF WORK SITE	3155315	2000
TEMPORARY ROAD	3-92 -90	90 x 135	10	10	MOUNTING WITH 3-90 OR 3-91		WHITE
GUIDE SIGNS					SIGN BOARD	-1-	
	N 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		200	1 30	MOUNTING AT 100~200 M. BEFORE A PPROACHING	REFLECT	RED - ORA
	2-7 -90	90 x 90	-	- E	MOUNTING AT 100-200 M. BEFORE APPROACHING  MOUNTING AT 100-200 M. BEFORE APPROACHING	REFLECT	RED- OR
	2-8-90	90 x 90	-	1 - 2	MOUNTING AT 100-200 M. BEFORE APPROACHING	REFLECT	RED - ORA
	2-29-90	90 x 90	1 2	-	MOUNTING AT 100-200 M. BEFORE APPROACHING	REFLECT	RED - OR
	2 - 34 - 90	90 x 90	-	-	MOUNTING AT 100-200 M. BEFORE APPROACHING	REFLECT	RED - ORA
	2 - 36 - 90		-		MOUNTING AT 100-200 M. BEFORE APPROACHING	REFLECT	RED - ORA
	2-106-90	90 x 150	20	. 15	MOUNTING AT 100-200 M. BEFORE APPROACHING	125 325	WHITE
WARNING SIGNS	2 - 95 - 90		17		MOUNTING AT 150 M. BEFORE APPROACHING	REFLECT	RED - ORA
MOUNTING AT .		BOARD	11				
APPROACHING	2-96-90	90x90	1 10	10	MOUNTING AT 150 M. BEFORE APPROACHING	REFLECT	RED - ORA
WORK SITE	2-96-90	BOARD	11 "		modified at least to the first terms	The state of the	2007-120-0
	2 -97 -90	60×80	1		MOUNTING AT 150 M. BEFORE APPROACHING	-	RED- ORA
	2-102-90	90xi50	20	15	MOUNTING NEAR AND SAME SIDE OF STOCKPILE MATERIAL		WHITE
	2-100-90	90× 90	_		MOUNTING AT 150 M. BEFORE APPROACHING	REFLECT	RED - ORA
	2-101 -90	90×90	2	1 -	MOUNTING AT 150 M, BEFORE APPROACHING	-	RED - ORA
	2-26-90	90×90	-	-	MOUNTING AT 100-200 M. BEFORE APPROACHING	REFLECT	YELLO
	2-103-90	90x150	20	15	FOR EMERGENCY CASE MOUNTING ON BARRICADE	REFLECT	WHITE
	2-105-90	90x90	_	-	FOR EMERGENCY CASE WHERE SUITABLE ROAD SIGNS ARE NOT AVAILABLE	REFLECT	RED- ORA
	F 20 828	90	1 10	1000	MOUNTING ON CONTROLLED POINT OR NEARER IN DISTANCE 3-5 M.	REFLECT	WHITE
	1-3-90	19075	15	1	MOUNTING ON CONTROLLED POINT OR NEARER IN DISTANCE 3 - 5 M.	REFLECT	WHITE
	1-5-90	90			MOUNTING ON CONTROLLED POINT OR NEARER IN DISTANCE 3-5 M.	REFLECT	WHITE
	100,000	0.00			MOUNTING ON CONTROLLED POINT OR NEARER IN DISTANCE 3-5 M.	REFLECT	BLUE
SIGNS TO CONTROL	1-21 -90	1000			MOUNTING ON CONTROLLED POINT OR NEARER IN DISTANCE 3-5 M.  MOUNTING ON CONTROLLED POINT OR NEARER IN DISTANCE 3-5 M.	REFLECT	WHITE
SPEED	1-32-90	22,000	. 5		MOUNTING ON CONTROLLED POINT OR NEARER IN DISTANCE 3-5 M.  MOUNTING ON CONTROLLED POINT OR NEARER IN DISTANCE 3-5 M.	REFLECT	WHITE
	1 - 33 - 90	90	-		MOUNTING BEFORE APPROACHING CONTROLLED SPEED ZONE	REFLECT	WHITE
	2-80-60		25	15	MOUNTING BEFORE APPROXIMENTS CONTINUED SEED BOTTO	REFLECT	RED - OR
	2-80-60	60 x 60	20	10	mostrate miche trans datatit.	100000	
	350			-	300	-	
			120	24.2	20   120   120   120	20	
120	61.6	+	120	64.6	R*3.6		R=3.6
2.4 1.8		1.1			2.4 1.8 FLUORESCENT LAMP 40 40W 40W	WALLS	20
1-1-40W	_		40W		30 130 st. 80		+
Q/	างก่อล	ร้าง			208	7	
,	1 1/11 15794	9 14		- 111	20 40W 25	40W	
40 W	ข้างหร	200		40W	200 160 25 40W	>	120 160
40 W	ALIAN.	R.I		37.11	2014		100
6 . 98	301.6		e .	ο-	25		
<b>โปรดใช้รถ</b>	ดวยคว	ามระ	มดระ	:34	208		
	- 0	100	40W	10	30 40W \ 40W		20

THE SIGNS WITH FLUORESCENT LAMP SHALL BE MOUNTED

2. ALL DIMENSIONS ARE IN CENTIMETERS UNLESS OTHERWISE INDICATED.

FOR ROAD WHICH ADT EXCEEDS 4,000 VEHICLE/DAY

KINGDOM OF THAILAND
MINISTRY OF TRANSPORT AND COMMUNICATIONS
DEPARTMENT OF HIGHWAYS

SCALE NONE

DWG. NO. RS-102

SHEET NO. 19

STANDARD DRAWING

TRAFFIC CONTROL DEVICES
FOR HIGHWAYS UNDER CONSTRUCTION-I

DESIGNED: D.O.H. & CONSULTANTS CHECKED: DATE JULY 1994

P. Barry

APPROVED:

DIRECTOR OF LOCATION & DESIGN DIVISION

















EQUIVALENT FOR ROAD WHICH ADT

EXCEEDS 8,000 VEHICLE / DAY

9 - YELLOW REFLECTORIZED CIRCLES TARGET FOR ROAD WHICH ADT EXCEEDS 4,000 VEHICLE / DAY







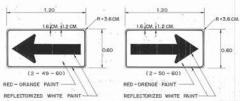
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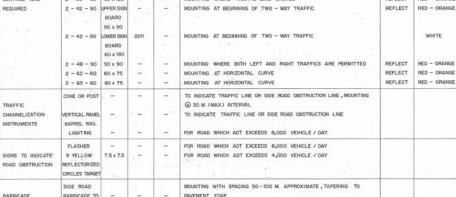
(2-42-90)



SCALE

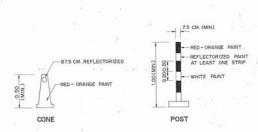
SIGNS TO INDICATE ROAD OBSTRUCTION





1. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.

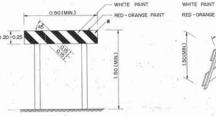
### ROAD SIGN MOUNTING AT TRAFFIC LINE CHANGE OR TRAFFIC CONTROL

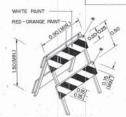


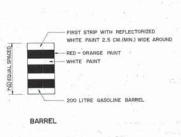
REFLECTORIZED WHITE

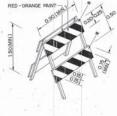
WHITE PAINT RED - ORANGE PAINT

VERTICAL PANEL

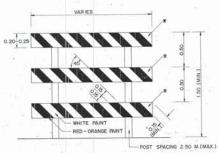








### BARRICADE TYPE I



PROVISION:

### THE AMOUNTS (IN TWO DIRECTIONS) AND SPACING OF WARNING SIGN BEFORE CONSTRUCTION AREA BESIDES OF THE NORMAL SKINS ARE SHOWN IN THE TABLE BELOW

2. REFLECTORIZED PROPERTIES MEANS : SIGN BOARD WHICH REFLECTING BY REFLECTIVE SHEETING

TRAFFIC CONTROL DEVICES FOR HIGHWAYS UNDER CONSTRUCTION SHALL BE IN ACCORDANCE WITH

THIS DRAWING AND THE MANUAL OF TRAFFIC CONTROL DEVICES PART 3 AS PUBLISHED BY THE DOH. 2. SIGN BOARD FOR REFLECTORIZED SHALL BE METAL OR NON - METAL WHICH TOLERANCE TO ALL CLIMATES ALONG TERM OF CONTRACT UNDER THE APPROVAL OF THE ENGINEER.

OR REFLECTIVE BEADS OR OTHERS UNDER THE APPROVAL OF THE ENGINEER. SIZE OF WARNING SIGN CODE 2-49-60 AND 2-50-60 MAY BE INCREMENTED AT SUITABLE SIZE ON HIGH SPEED AND HEAVY TRAFFIC ROAD UNDER THE APPROVAL OF THE ENGINEER.

CLASSIFICATION	WARNING SIGN SPACING BEFORE APPROACHING DIVERTED POINT(M.)			
OF HIGHWAYS	REDUCED TRAFFIC LANE OR DIVERTED TRAFFIC LANE	SIDE ROAD OBSTRUCTION		
EXPRESSWAY AND MAJOR RURAL HIGHWAY	500 - 1,000 (2-3)	300 - 500 (2)		
RURAL HIGHWAY	200 - 300 (2)	150 - 200 (1)		
URBAN HIGHWAY	70 - 100 (1)	_		

### TRAFFIC CHANNELIZATION INSTRUMENT TO BE USED

REFLECTORIZED WHITE

- 1. TO INDICATE SIDE ROAD HAZARD, VERTICAL PANEL OR BARREL SHALL BE INSTALLED BEFORE CONES, POSTS, VERTICAL PANELS, OR BARRELS FOLLOW ON THE LINE.
- 2. FOR ROAD WHICH ADT EXCEEDS 8,000 VEHICLE/DAY, RAIL LIGHTING SHALL BE REQUIRED ON TRAFFIC CHANNELIZATION INSTRUMENT POSITIONS.

- FOR BARRICADE TYPE I, I
- \* REFLECTIVE SHEETING OR REFLECTORIZED WHITE PAINT OR REFLECTORIZED CIRCLES TARGET OR
- FOR MOUNTING, THE PAINTED STRIPE SHALL BE INCLINED TO APPROACH TRAFFIC SIDE

BARRICADE TYPE II

- 4. FOR URBAN HIGHWAY WITH AMOUNT OF PEDESTRIANS, WALKWAY WHICH SAFETY SHALL BE PROVIDED FOR PEDESTRIANS.
- THE CONTRATOR SHALL BE PROVIDED THE TRAFFIC CONTROL DEVICES FOR HIGHWAYS UNDER CONSTRUCTION ALL ITEMS.
- 6. IN CASE OF THE FORMAT IS NOT THE SAME AS THE DRAWINGS, THE MOUNTING OF TRAFFIC CONTROL DEVICES SHALL BE DIRECTED BY THE ENGINEER.

FLASHER DEPENDED ON TRAFFIC VOLUME, MOUNTING AT THE LAST CORNER.

STANDARD DRAWING TRAFFIC CONTROL DEVICES FOR HIGHWAYS UNDER CONSTRUCTION - II

> CHECKED: 5-DESIGNED : D.O.H. DATE JULY 1994 SUBMITTED P. Burry. SCALE NONE IDIRECTOR OF LOCATION & DESIGN DIVISION DWG. NO. RS - 103 APPROVED: SHEET NO. 20

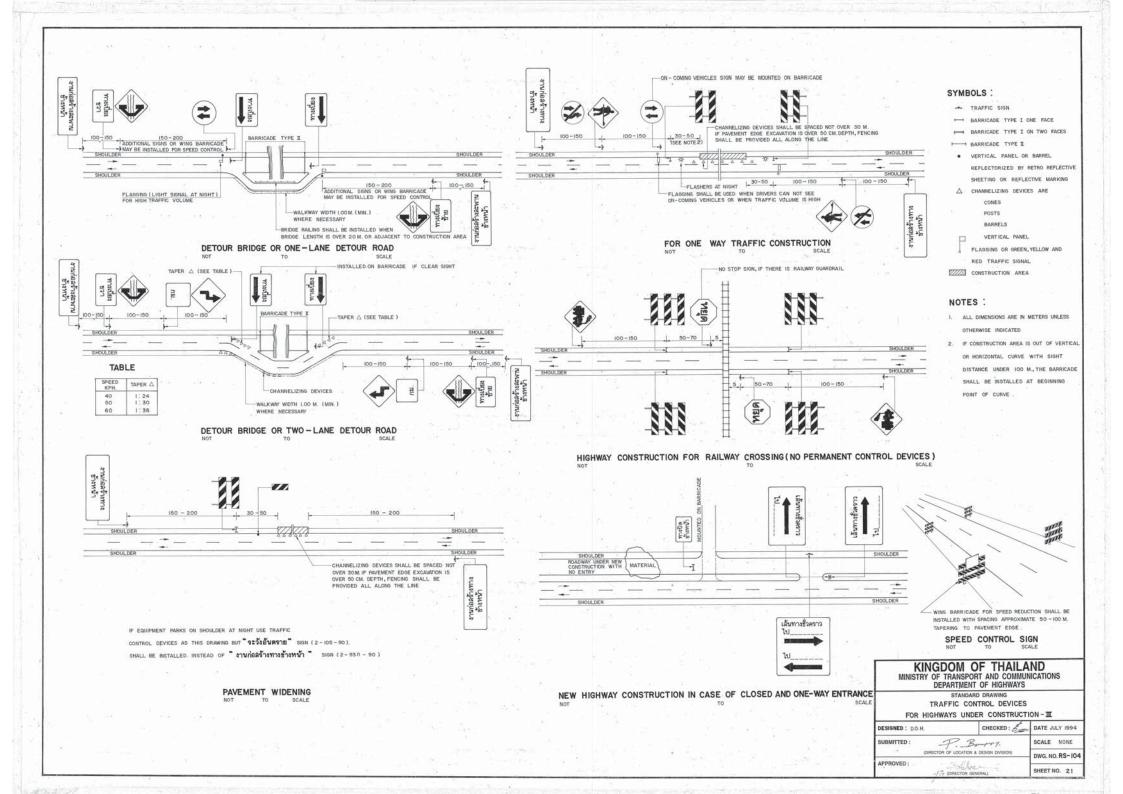
KINGDOM OF THAILAND

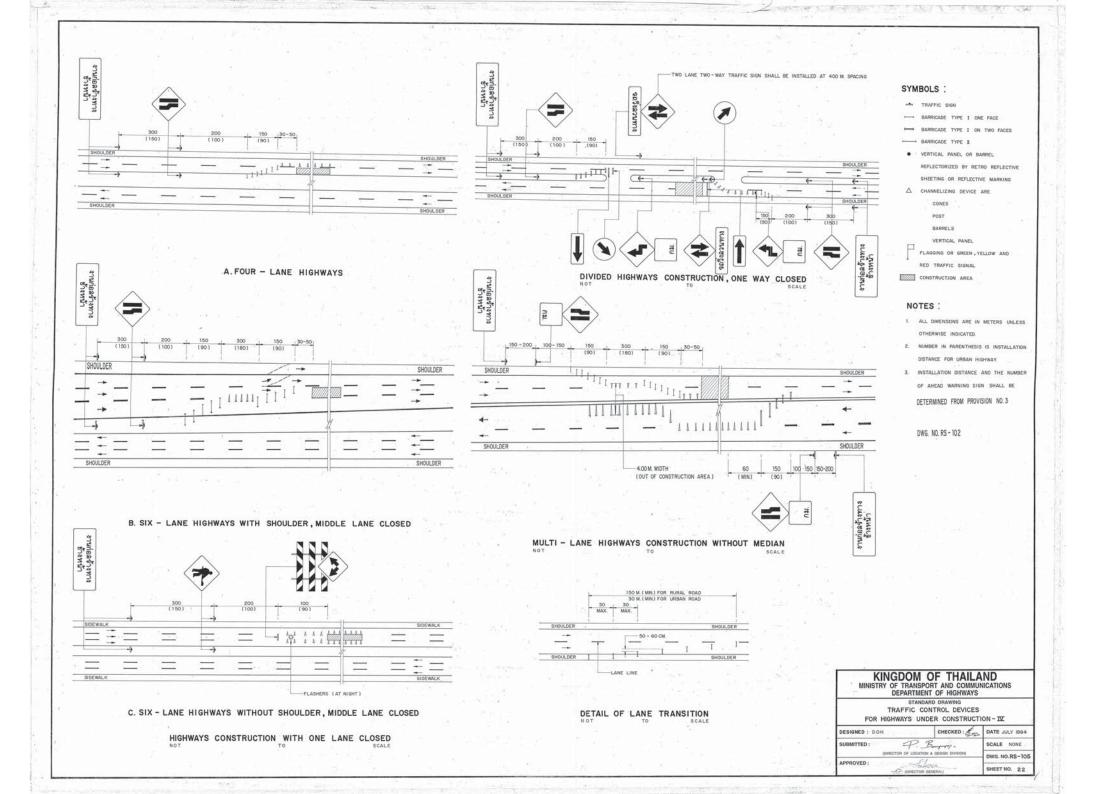
MINISTRY OF TRANSPORT AND COMMUNICATIONS

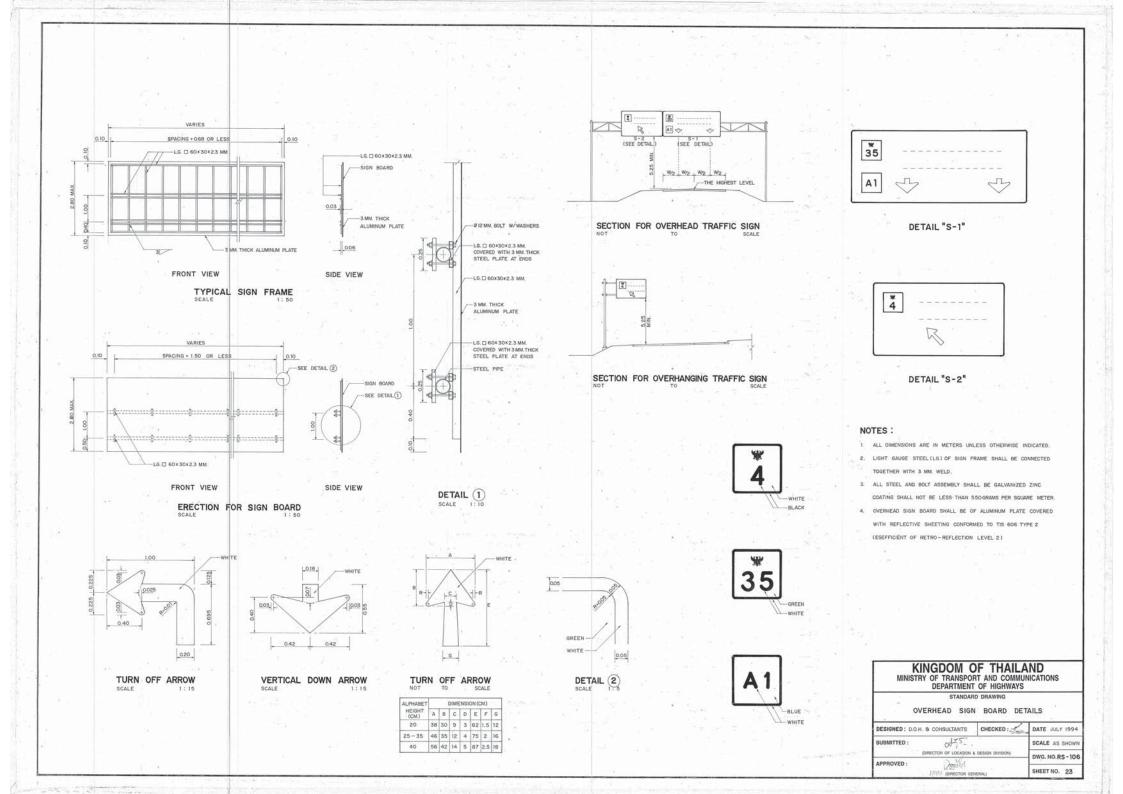
DEPARTMENT OF HIGHWAYS

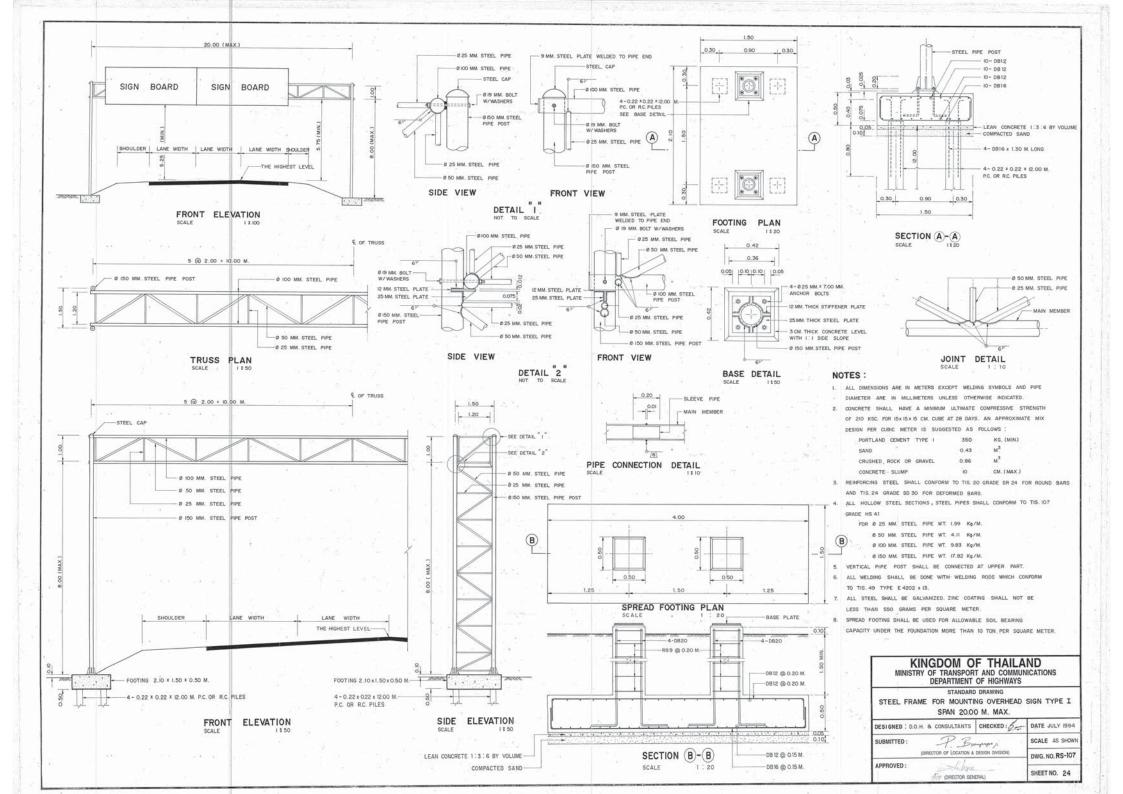
### TRAFFIC CHANNELIZATION INSTRUMENTS

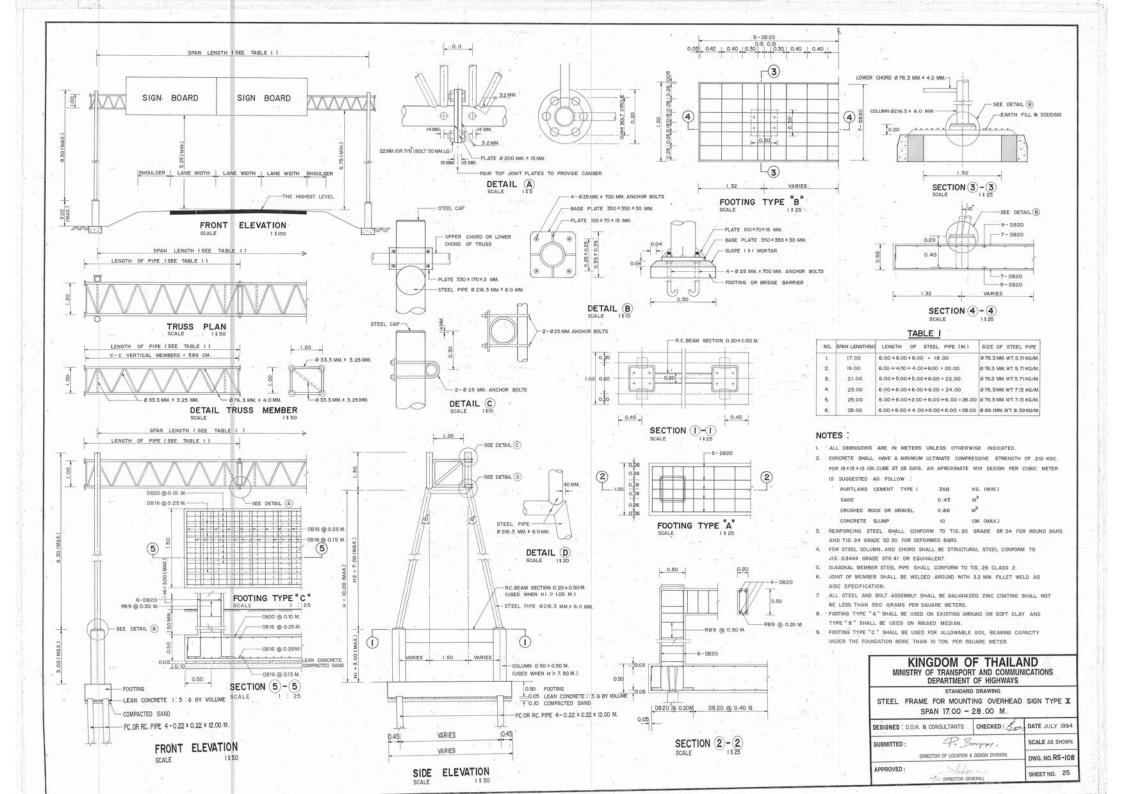
( RED - ORANGE PAINT OR RED - ORANGE PAINT ALTERNATE WITH WHITE PAINT)

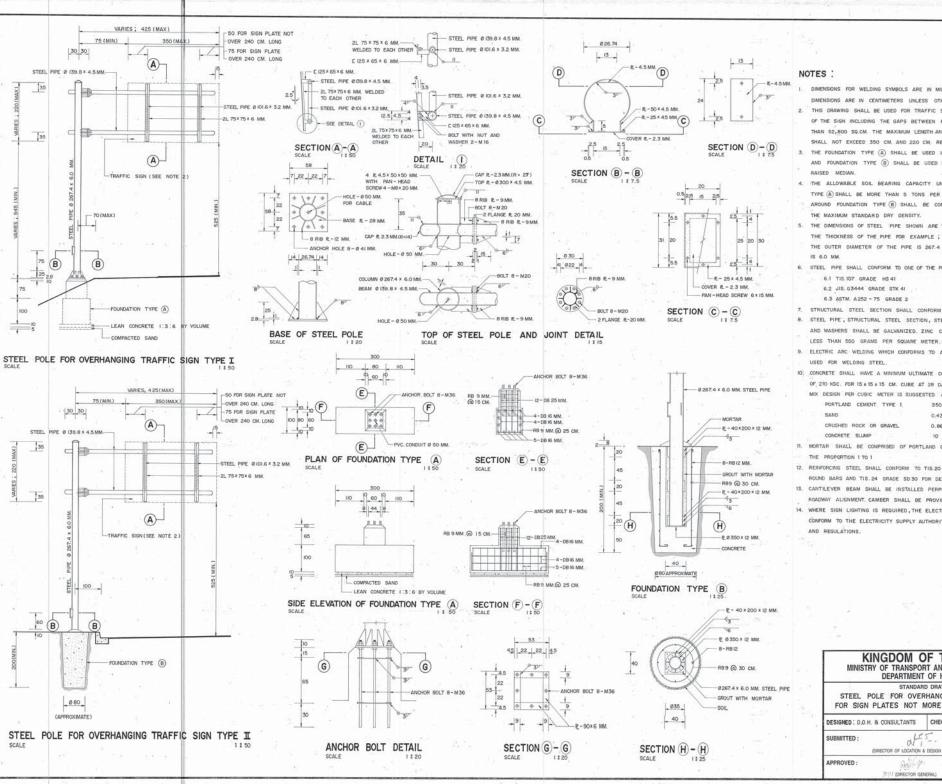












- DIMENSIONS FOR WELDING SYMBOLS ARE IN MILLIMETERS, ALL OTHER

- THE ALLOWABLE SOIL BEARING CAPACITY UNDER THE FOUNDATION TYPE (A) SHALL BE MORE THAN 5 TONS PER SQ.M. EMBANKMENT AROUND FOUNDATION TYPE (B) SHALL BE COMPACTED TO 90 % OF THE MAXIMUM STANDARD DRY DENSITY.
- THE DIMENSIONS OF STEEL PIPE SHOWN ARE THE OUTER DIAMETER AND THE THICKNESS OF THE PIPE FOR EXAMPLE ; Ø 267.4 × 6.0 MM. MEANS
- STEEL PIPE SHALL CONFORM TO ONE OF THE FOLLOWING SPECIFICATIONS 6.1 TIS.107 GRADE HS 41
  - 6.2 JIS. G3444 GRADE STK 41

  - STRUCTURAL STEEL SECTION SHALL CONFORM TO TIS 116 GRADE Fe 24
- STEEL PIPE, STRUCTURAL STEEL SECTION, STEEL PLATE, BOLTS, NUTS, WASHERS SHALL BE GALVANIZED. ZINC COATING SHALL NOT BE
- ELECTRIC ARC WELDING WHICH CONFORMS TO AISC STANDARD SHALL BE
- 10: CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF, 210 KSC. FOR 15 x 15 x 15 CM. CUBE AT 28 DAYS. AN APPROXIMATE MIX DESIGN PER CUBIC METER IS SUGGESTED AS FOLLOWS

PORTLAND CEMENT TYPE 1.	350	KG, (MIN.
SAND	0.43	M.3
CRUSHED ROCK OR GRAVEL	0.86	M. <sup>3</sup>
CONCRETE SLUMP	10	CM. (MAX

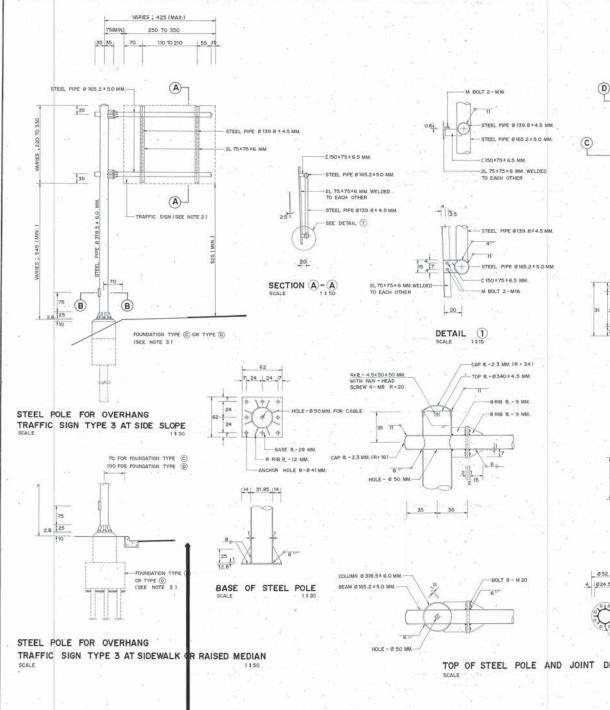
- 11. MORTAR SHALL BE COMPRISED OF PORTLAND CEMENT AND SAND IN
- 12. REINFORCING STEEL SHALL CONFORM TO TIS.20 GRADE SR 24 FOR ROUND BARS AND TIS. 24 GRADE SD 30 FOR DEFORMED BARS.
- 13. CANTILEVER BEAM SHALL BE INSTALLED PERPENDICULAR TO THE
- ROADWAY ALIGNMENT, CAMBER SHALL BE PROVIDED FOR BEAM DEFLECTION. 14. WHERE SIGN LIGHTING IS REQUIRED, THE ELECTRICAL COMPONENTS SHALL
- CONFORM TO THE ELECTRICITY SUPPLY AUTHORITY'S REQUIREMENTS

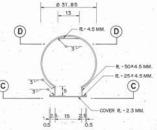
### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

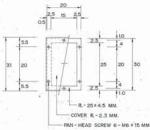
STEEL POLE FOR OVERHANGING TRAFFIC SIGN FOR SIGN PLATES NOT MORE THAN 52,800 SQ.CM.

CHECKED: 5-DESIGNED: D.O.H. 8. CONSULTANTS DATE JULY 1994 (DIRECTOR OF LOCATION & DESIGN DIVISION DWG. NO. RS-109 SHEET NO. 26 /HILL IDIRECTOR GENERA

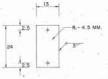




SECTION B - B



SECTION C - C



SECTION (D) - (D)

032.5 4 | 024.5 | 4 -2 FLANGE R - 20 MM

TOP OF STEEL POLE AND JOINT DETAIL

### NOTES :

- 1. DIMENSIONS FOR WELDING SYMBOLS ARE IN MILLIMETERS, ALL OTHER DIMENSIONS ARE IN CENTIMETERS UNLESS OTHERWISE INDICATED
- THIS DRAWING SHALL BE USED FOR TRAFFIC SIGNS, WHERE THE AREA OF THE SIGN IS NOT MORE THAN 108,000 SQ.CM. THE MAXIMUM LENGTH AND WIDTH OF THE SIGN SHALL NOT EXCEED 350 CM, AND 330 RESPECTIVELY.
- 3. GENERALLY, THE FOUNDATION SHALL BE TYPE (C) OR TYPE (D) . IN CASE OF THE PILES SHALL NOT BE DRIVEN TO THE SOIL OR THE ALLOWABLE SOIL BEARING CAPACITY UNDER THE FOUNDATION MORE THAN 10 TON PER SQUARE METER, THE FOUNDATION TYPE (D) WITH NO PILES SHALL BE USED.
- 4. THE DIMENSIONS OF STEEL PIPE SHOWN ARE THE OUTER DIAMETER AND THE THICKNESS OF THE PIPE FOR EXAMPLE ; Ø 318.5 × 6.0 MM. MEANS THE OUTER DIAMETER OF THE PIPE IS 318.5 MM. AND THE THICKNESS IS 6.0 MM
- STEEL PIPE SHALL CONFORM TO ONE OF THE FOLLOWING SPECIFICATIONS

5.1 TIS 107 GRADE HS 41 5.2 JIS.G3444 GRADE STK41 5.3 ASTM. A252-75 GRADE 2

- 6. STRUCTURAL STEEL SECTION SHALL CONFORM TO TIS. 116 GRADE Fe 24.
- 7. STEEL PIPE, STRUCTURAL STEEL SECTION, STEEL PLATE, BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. ZINC COATING SHALL NOT BE LESS THAN 550 GRAMS PER
- B. ELETRIC ARC WELDING WHICH CONFORMS TO AISC STANDARD SHALL BE USED FOR WELDING STEEL
- CONCRETE FOR REINFORCED CONCRETE PILE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 300 KSC. FOR 15 x 15 x 15 CM. CUBE AT 28 DAYS.

KG (MIN ) 0.43 CRUSHED ROCK OR GRAVEL 0.86 CONCRETE SLUMP

IO. CONCRETE FOR PRESTRESS CONCRETE PILE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 440 KSC. FOR 15 x 15 x 15 CM. CUBE AT 28 DAYS

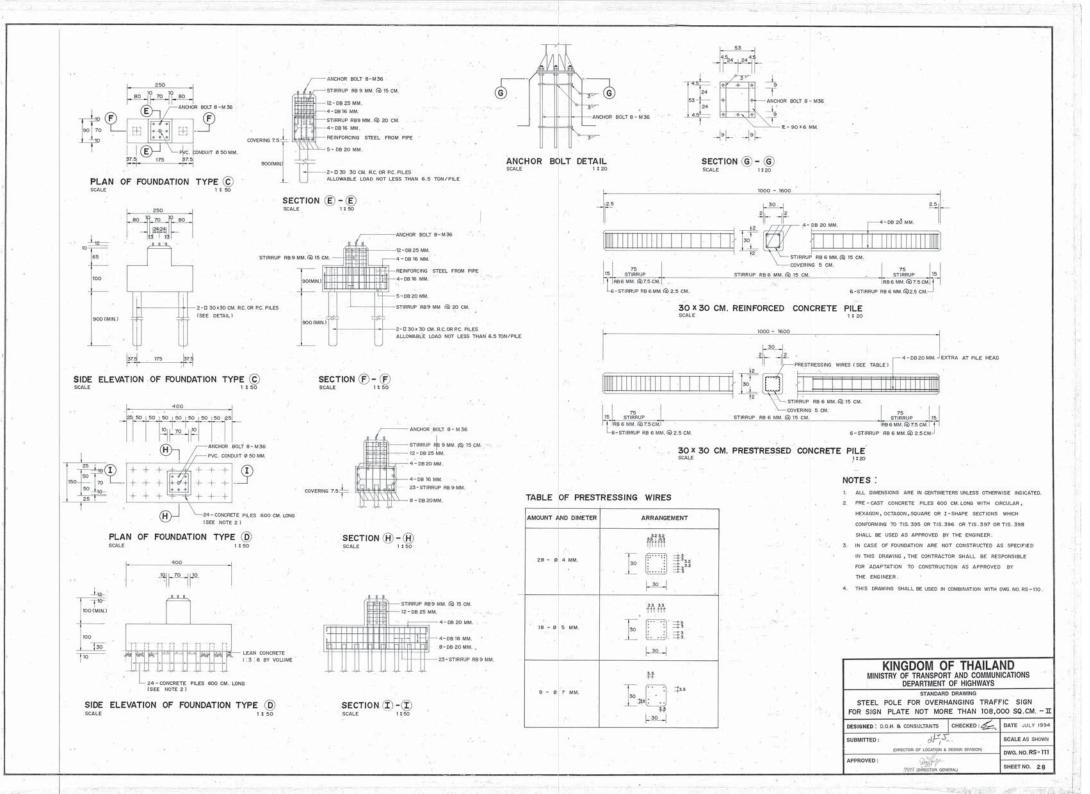
0.43 CRUSHED ROCK OR GRAVEL 0.86 CONCRETE SLUMP 10 CM.(MAX.)

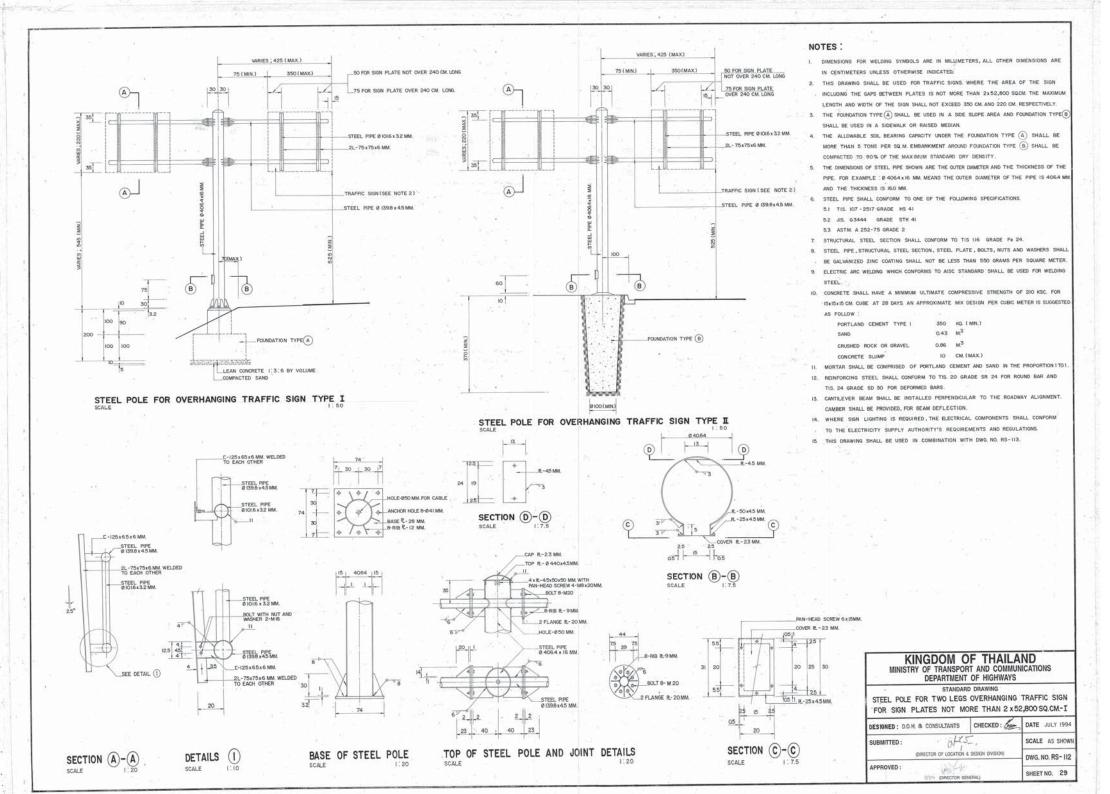
- 12. REINFORCING STEEL SHALL CONFORM TO TIS.20 GRADE SR24 FOR ROUND BARS AND TIS.24 GRADE SD 30 FOR DEFORMED BARS
- 13. PRESTRESSING WIRE SHALL CONFORM TO TIS. 95
- 14. NORMAL CLEAR CONCRETE COVER SHALL BE 5 CM. BUT FOR FOUNDATION SHALL BE 7.5 CM., UNLESS OTHERWISE INDICATED
- 15. CANTILEVER BEAM SHALL BE INSTALLED PERPENDICULAR TO THE ROADWAY ALIGNMENT, CAMBER SHALL BE PROVIDED FOR BEAM DEFLECTION
- 16. WHERE SIGN LIGHTING IS REQUIRED, THE ELECTRICAL COMPONENTS SHALL CONFORM TO THE ELECTRICITY SUPPLY AUTHORITY'S REQUIREMENTS AND REGULATIONS.
- 17. THIS DRAWING SHALL BE USED IN COMBINATION WITH DWG. NO. RS-111.

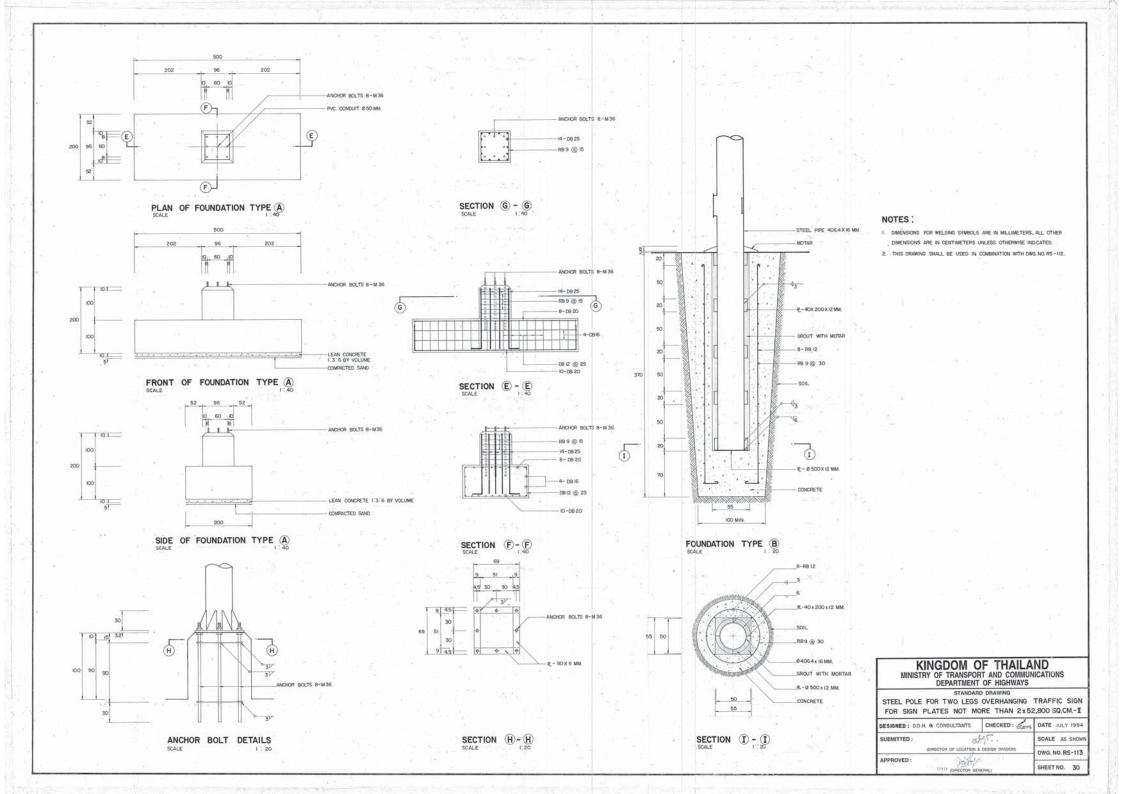
### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

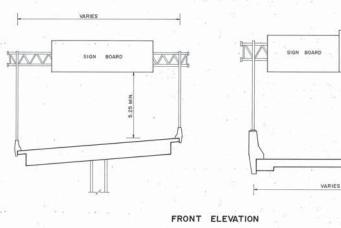
STANDARD DRAWING STEEL POLE FOR OVERHANGING TRAFFIC SIGN FOR SIGN PLATES NOT MORE THAN 108,000 SQ.CM. - I

DESIGNED: D.O.H. & CONSULTANTS CHECKED: 5 DATE JULY 1994 OFTS SCALE AS SHOW! DWG. NO. RS-110 APPROVED: SHEET NO. 27 1/1/1/ (DIRECTOR GENERAL

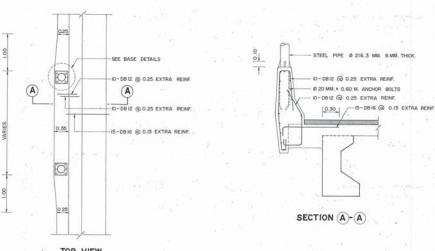


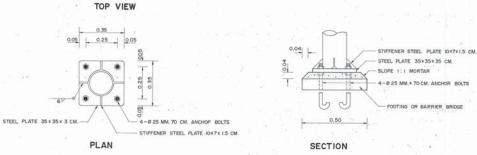






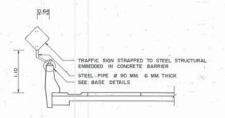
SIGN BOARD



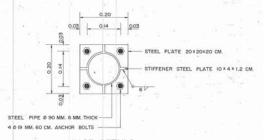


BASE DETAILS

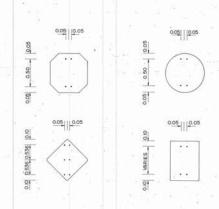
OVERHEAD SIGN AND FRAME ON BARRIER



### FRONT ELEVATION

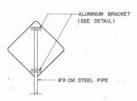


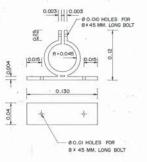
BASE DETAILS



### ASSEMBLY DETAILS

TRAFFIC SIGN AND FRAME ON BARRIER
NOT TO SCALE





ALUMINUM BRACKET ASSEMBLY DETAILS
NOT TO SCALE

### NOTES :

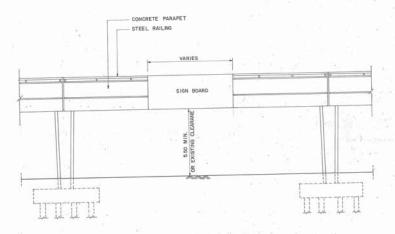
ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED

### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

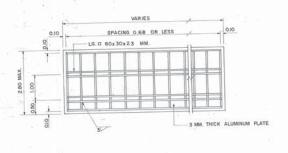
STANDARD DRAWING

INSTALLATION OF OVERHEAD SIGN AND FRAME TYPE IL & TRAFFIC SIGN AND FRAME ON BARRIERS

DESIGNED : D.C	.H. & CONSULTANTS	CHECKED:	DATE JULY 1994
SUBMITTED :	at-5.	SCALE NONE	
	DWG, NO. RS-114		
APPROVED:	MIN IDIRECTOR GE	NERAL)	SHEET NO. 31

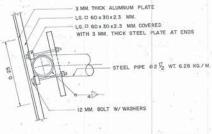


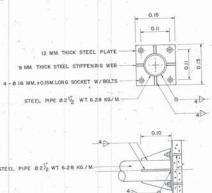
VARIES SPACING 1.50 OR LESS LG. D 60×30×2.3 MM. STEEL PIPE Ø 2 1/2 WT. 628 KG./ M. 0.50 STEEL PIPE COVERED WITH 3 MM. THICK STEEL PLATE AT ENDS



SIGN BOARD ERECTION ON BRIDGE DECK

TYPICAL SIGN FRAME





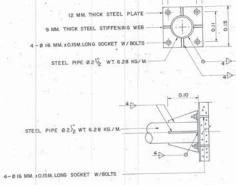


4 R IS MM XOISM LONG SOCKET W/BOLTS

SEE DETAIL "B" STEEL PIPE WT. 6.28 KG. 1.031

TYPICAL SIGN FRAME SUPPORT

SEE DETAIL "A



DETAIL "B"

### NOTES

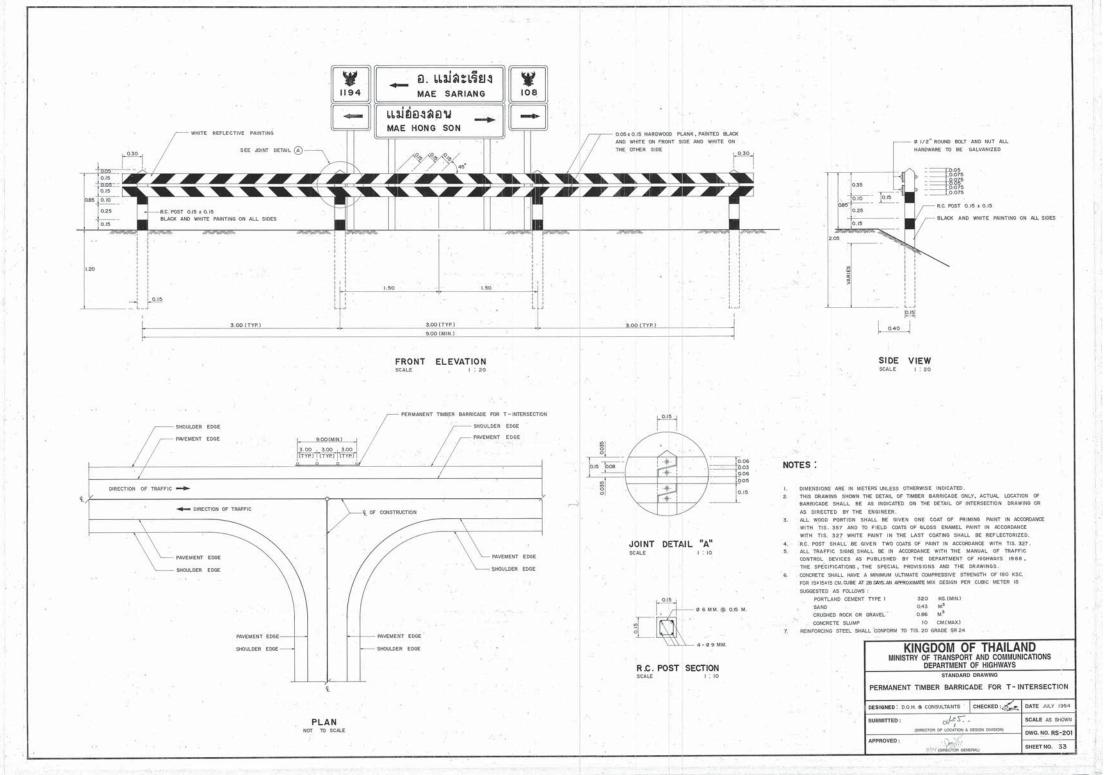
- DIMENSIONS SHOWN ARE IN METERS EXCEPT WELDING SYMBOLS ARE IN MILLIMETERS, OR OTHERWISE INDICATED.
- STRUCTURAL CARBON STEEL SHALL CONFORM TO TIS. H6 GRADE Fe 24.
- STEEL PIPE SHALL BE PLAIN ENDS, MEDIUM CLASS CONFORM
- VERTICAL PIPE POST SHALL BE CONNECTED AT UPPER PART.
- LIGHT GAUGE STEEL (L.G.) OF SIGN FRAME SHALL BE CONNECTED
- ALL STEEL AND BOLT ASSEMBLY SHALL BE GALVANIZED ZINC COATING SHALL NOT BE LESS THAN 550 GRAMS PER SQUARE METER.

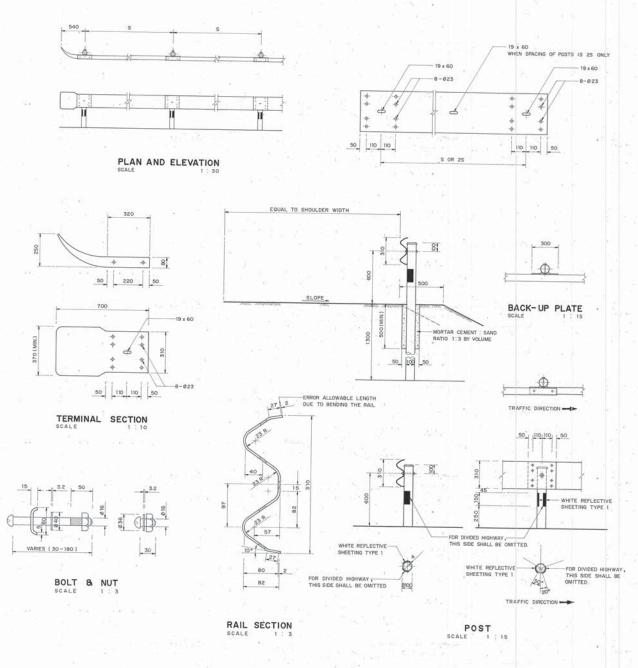
### KINGDOM OF THAILAND

MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING STEEL FRAME FOR MOUNTING OVERHEAD SIGNS AT BRIDGE DECK

CHECKED: DATE JULY 1994 DESIGNED: D.O.H. & CONSULTANTS SCALE AS SHOWN a4,5-. SUBMITTED : (DIRECTOR OF LOCATION & DESIGN DIVIS DWG. NO. RS-118 APPROVED: SHEET NO. 32





### NOTES :

- 1. ALL DIMENSION ARE IN MILLIMETERS UNLESS OTHERWISE INDICATED.
- 2. GUARDRAIL SHALL CONFORM TO THE FOLLOWING REQUIREMENT
  - 2.1 STEEL RAL SHALL BE MADE FROM STEEL OF THICKNESS NOT LESS THAN THE MINIMUM THICKNESS SPECIFIED FOR EACH CLASS OF RAIL AND SHALL BE GALVANZED WITH THE MINIMUM WEIGHT OF ZINC COATING ACCORDING TO THE TYPE OF RAIL AS SPECIFIED IN TABLE BELOW.

RA	RAIL MIN. RAIL THICKNESS  ASS TYPE (MILLIMETER)		MIN. WEIGHT OF ZINC COATING
CLASS			(GRAMS PER SQUAREMETER)
1	2	3.2 3.2	550 1,100
- 2	2	2.5	550 1,100

2.2 MECHANICAL PROPERTES OF RAIL :

RAIL		MIN. ULTIMATE		MAX. DEFLECTION				
		TENSILE STRENGTH		MAX.LOAD TRAFFIC FACE UP		MAX.LOAD TRAFFIC FACE DOW		
CLASS	TYPE	( KG/MM. <sup>2</sup> )	OF ELONGATION	KG.	DEFLECTION (MM.)	, KG.	DEFLECTION (MM.)	
1	2	41	21	910 1,360	50 75	720 1,090	50 75	
2	2	41	21	680 910	50 75 -	545 720	50 75	

- 2.3, POST SHALL BE GALVANIZED STANDARD STEEL PIPE
- 2.4 THE GALVANIZING OF BOLTS, NUTS, WASHERS AND SIMILAR THREADED FASTENERS SHALL BE IN ACCORDANCE WITH TIS. 171 CLASS 5.8
- UNLESS OTHERWISE SPECIFIED, GUARDRAIL CLASS 1 SHALL BE USED FOR HIGHWAY CLASS D AND CLASS 1
  FOR OTHER HIGHWAY CLASSES, GUARDRAIL CLASS 2 SHALL BE USED.
- IN CASE OF HIGH RESISTANCE TO CORROSION REQUIREMENT, GUARDRAIL TYPE 2 SHALL BE USED.
- 5. SPACING OF POSTS SHALL BE LOCATED AS FOLLOWS

RADIUS OF CURVE R (M.)	SPACING OF POSTS S (M.)		
ON TANGENT OR R > 700	4.00		
450 ≤ R < 700	3.00		
150 ≤ R < 450	2.00		
R < 150	1.00		

*HEIGHT OF FILL H (M.)	SPACING OF POSTS S (M.)
H ≤ 3.00	4.00
3.00 < H   4.00	3.00
4.00 < H ≤ 5.00	2.00
5.00 < H ≤ 7.00	1.00

\* FOR SIDE SLOPE 2: 1 OR STEEPER

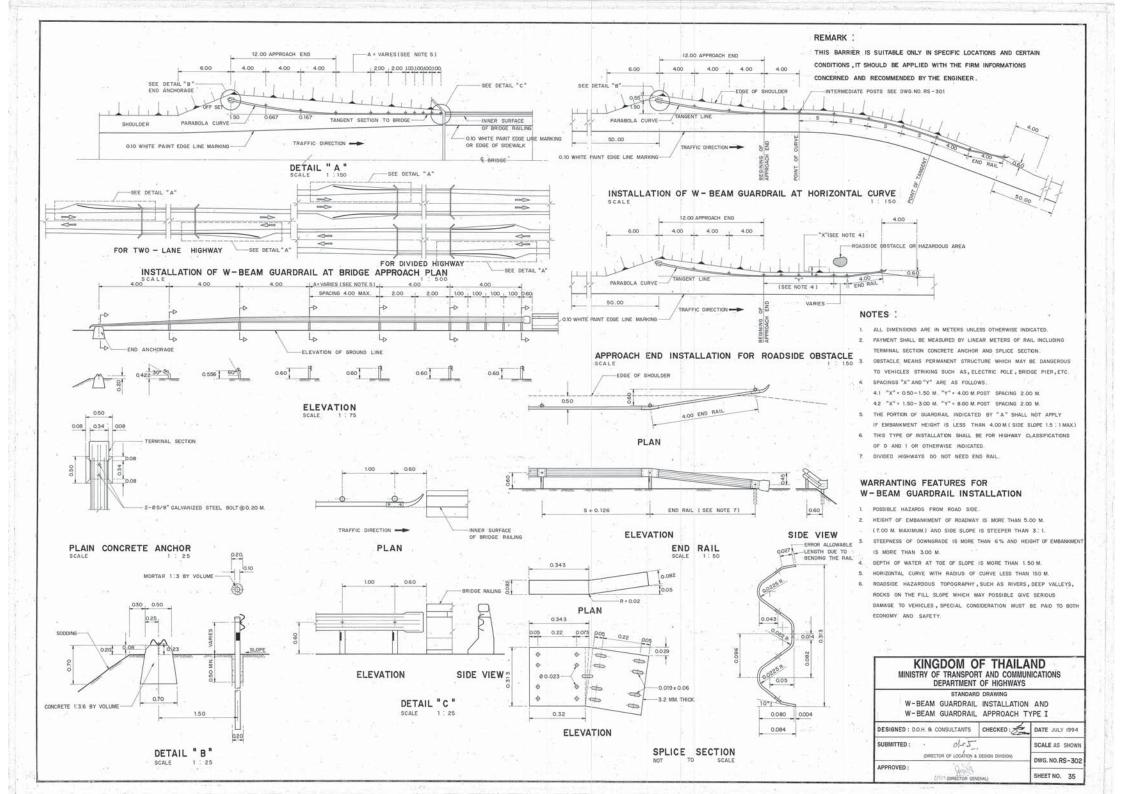
- FOR HORIZONTAL CURVE WITH HIGH FILL, THE MINIMUM SPACING VALUE OF THE ONE SHALL BE GOVERN.

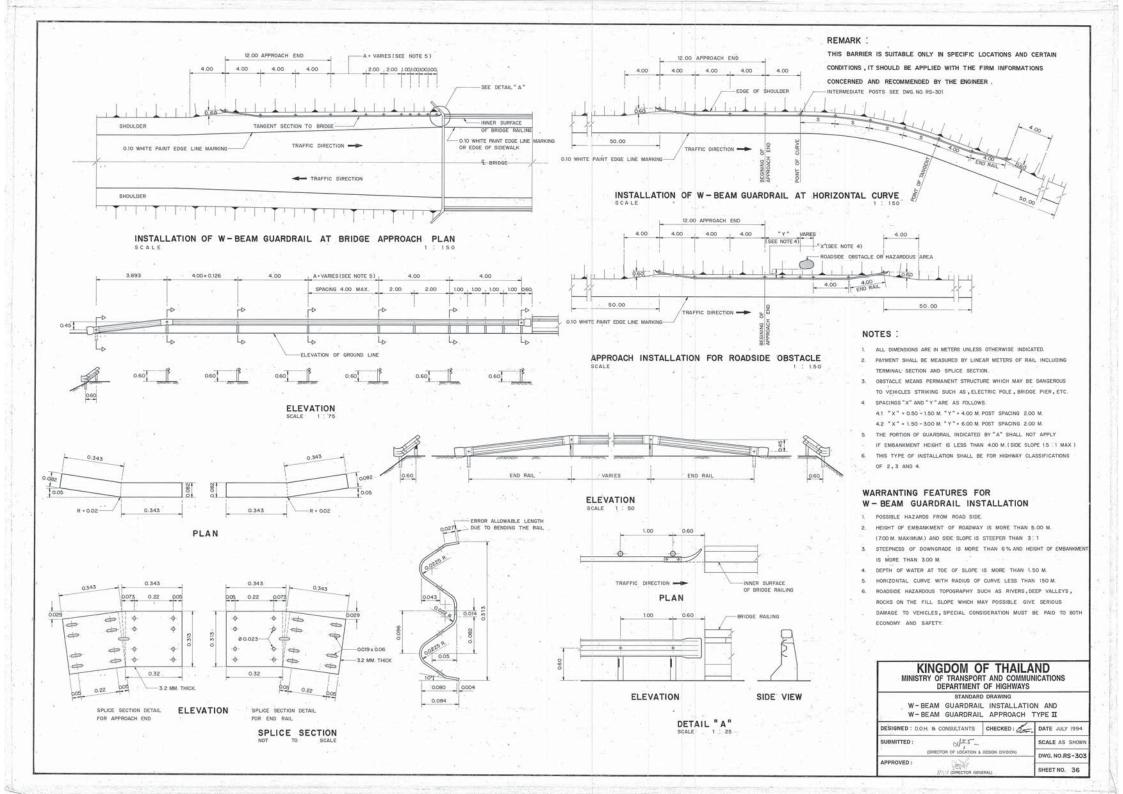
  G. GUARDRAE, IN GENERAL, IS MORE SUITABLE THAN GUIDE POST ON HORIZONTAL CURVE SECTION WITH HIGH FILL.
- MORE THAN 5.00 M. (AND SIDE SLOPE IS STEEPER THAN 3:1) AND NOT RECOMMENDED FOR HIGH FILL OVER ZOO M.
  Z. GUARDRAIL INSTALLED IN CURVE WHERE RADIUS IS LESS THAN 50 M. SHALL BE PRE-BENT FROM THE FACTORY.
- 8. BACK UP PLATE OF THE SAME CLASS AND TYPE AS THE RAIL WITH INTERMEDIATE POST SHALL BE PROVIDED WHERE SPACING OF POSTS IS 25 M.
- 9. UNLESS OTHERWISE SPECIFIED ON THIS DRAWING, GUARDRAIL SHALL BE IN ACCORDANCE WITH TIS 248
- 10 REFLECTIVE SHEETING SHALL CONFORM TO TIS. GOG TYPE 1. (ESEFFICIENT OF RETRO-REFLECTION LEVEL 1)

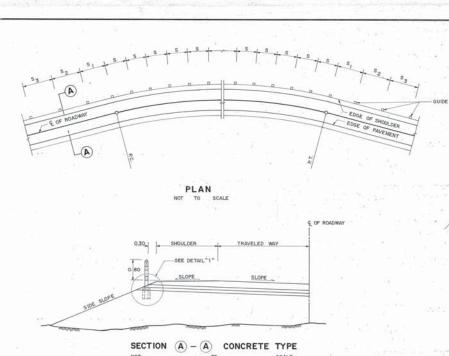
### REMARK :

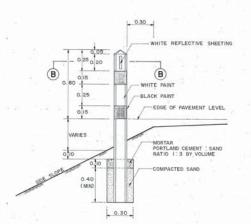
THIS BARRIER IS SUITABLE ONLY IN SPECIFIC LOCATIONS AND CERTAIN CONDITIONS, IT SHOULD BE APPLIED WITH THE FIRM INFORMATIONS CONCERNED AND RECOMMENDED BY THE ENGINEER.

### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS STANDARD DRAWING W - BEAM GUARDRAIL DESIGNED: D.O.H. & CONSULTANTS CHECKED: DIRECTOR OF LOCATION & DESIGN DIVISION) APPROVED: WILL DIRECTOR OF LOCATION & DESIGN DIVISION) DWG. NO.RS-301 SHEET NO. 34









DETAIL " 1 "

### TABLE SPACING OF GUIDE POST

RADIUS OF CURVE	SPACIN	SPACING OF GUIDE POST(METER)							
(METER)	S	SI	S <sub>2</sub>	S <sub>3</sub>					
15-74	4	7	12	24					
75-99	6	11	18	36					
100-149	. 7	13	21	42					
150 199	8	14	24	48					
200-299	9	16	27	54					
300 - 499	10	18	30	60					
500 - 999	15	27	45	60					
1000 - 1500	21	38	60	60					

# SECTION A — A TO FLEXIBLE TYPE SCALE

BRIDGE

€ OF ROADWAY

TRAVELED WAY

C OF ROADWAY

10.00 -

15.00

GUIDE POST

TRAVELED WAY

SLOPE

SHOULDER

GUIDE POST INSTALLATION AT BRIDGE APPROACH

SHOULDER

- SEE DETAIL "2"

### NOTES :

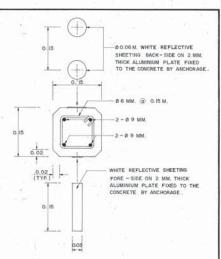
- I. DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- 2. REFLECTIVE SHEETING SHALL CONFORM TO TIS GOG TYPE I (ESEFFICIENT OF RETRO REFLECTION LEVEL 1 )

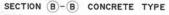
GUIDE POST-

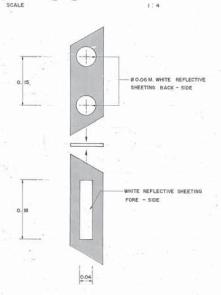
- 3. BLACK AND WHITE PAINTS SHALL CONFORM TO TIS 327 AND SHALL BE APPLIED ON ALL FACES IN TWO LAYERS.
- 4. POST SHALL BE ERECTED TRULY VERTICAL.
- CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 180 KSC.
   FOR 15 x 15 x I5 CM. CUBE AT 28 DAYS, AN APPROXIMATE MIX DESIGN PER CUBIC METER
   IS SUGGESTED AS FOLLOWS;

"龙地后以"一点。

6. REINFORCING STEEL SHALL CONFORM TO TIS, 20 GRADE SR. 24





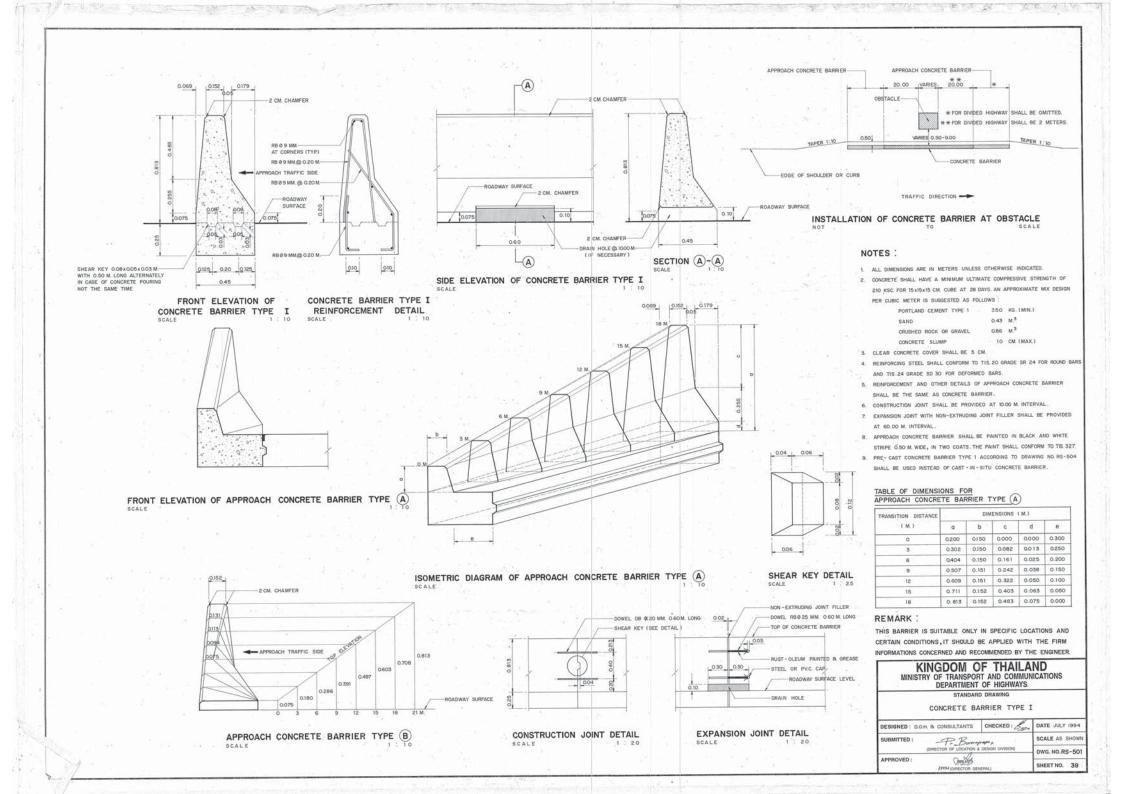


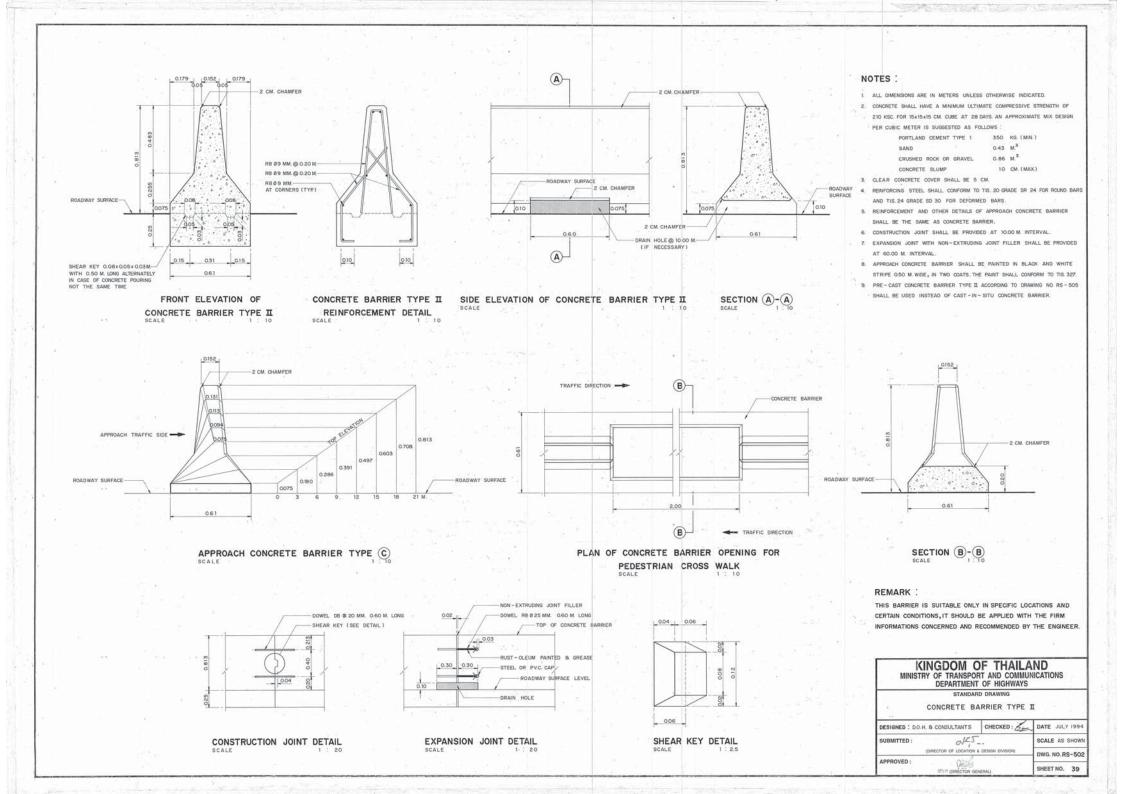
DETAIL "2" SCALE 1:4

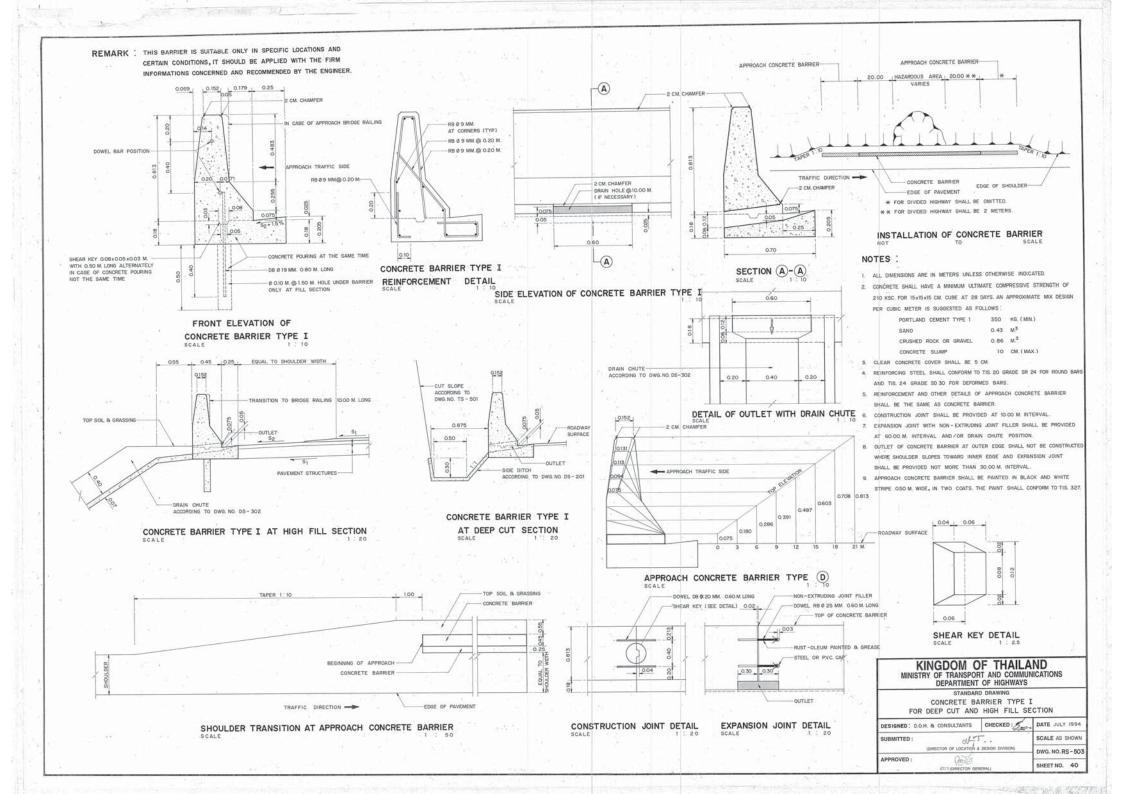
### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

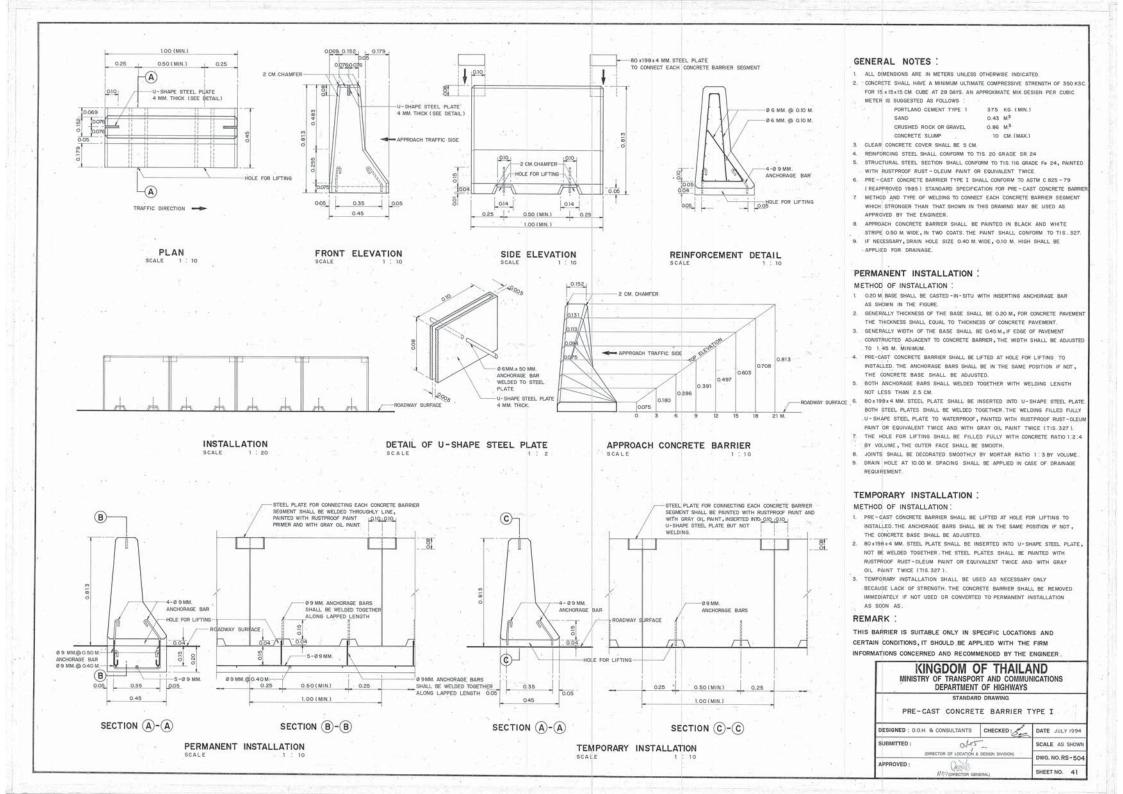
STANDARD DRAWING
GUIDE POST INSTALL ATION

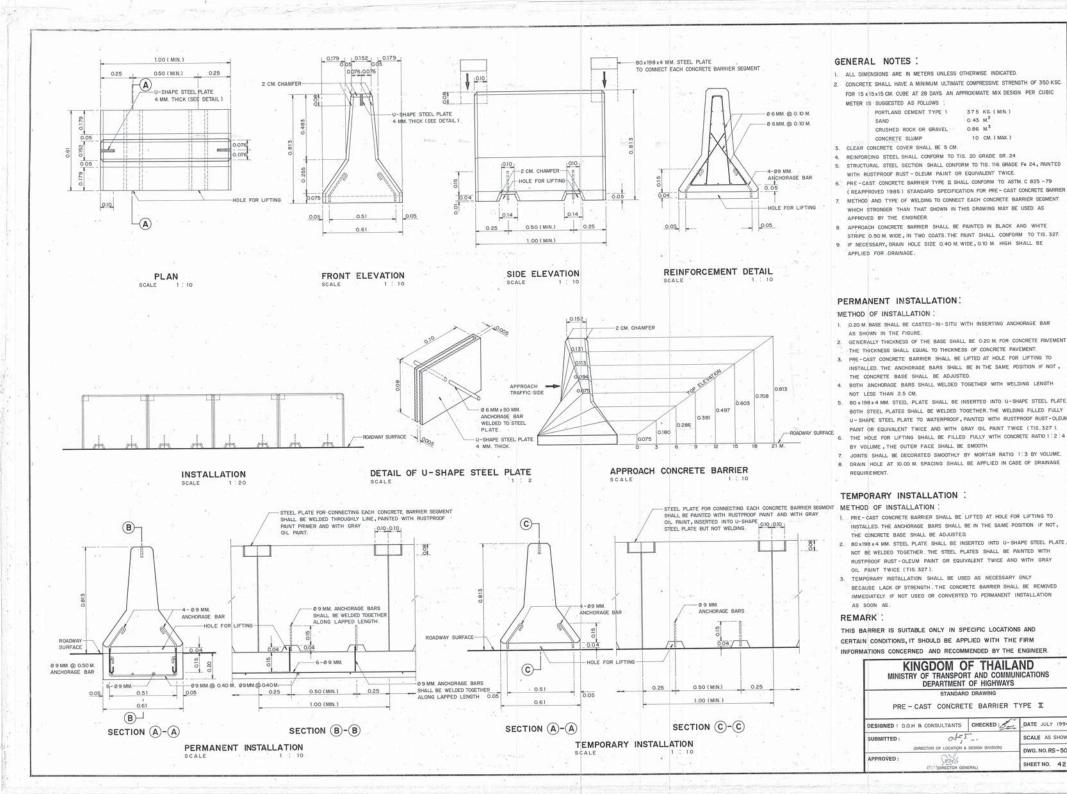
GOIDE FO	ST INSTALLATION
DESIGNED: D.O.H. & CONSULTANT	CHECKED : BATE JULY 1994
SUBMITTED:	SCALE AS SHOWN
	DWG. NO. RS-40
APPROVED:	SHEET NO. 37







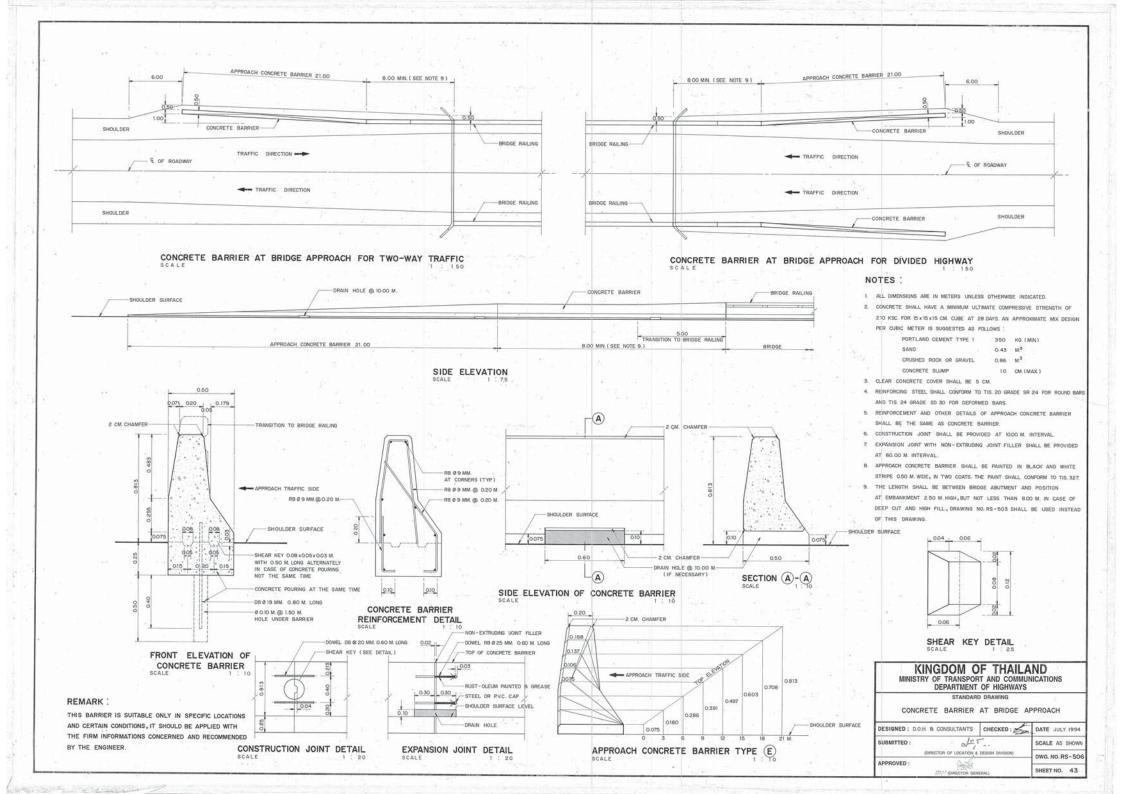




DATE JULY 1994

SCALE AS SHOW

DWG. NO. RS - 505



### WARRANTING FEATURES FOR CABLE GUARDRAIL INSTALLATION : 1. CABLE GUARDRAIL SHALL BE INSTALLED AT LOCATION FOR VISIBILITY PURPOSE. 1. THE SYSTEM OF CABLE GUARDRAIL CONSISTS OF 18 MM. (MINIMUM) DIAMETER 2. CABLE GUARDRAIL SHALL BE USED ONLY IF ADEQUATE DEFLECTION DISTANCE EXISTS. 3. NOT RECOMMENDED FOR USE IN URBAN AREAS. CABLE GUARDRAIL 4. NOT RECOMMENDED FOR USE IN MEDIANS NARROWER THAN 8.00 METERS. NOR IN MEDIANS WHICH CONTAIN RIGID OBJECTS. 5. CABLE GUARDRAIL SYSTEMS AT MEDIANS ARE RECOMMENDED FOR USE ON MORE THAN IRREGULAR TERRAIN, AND ON WIDER MEDIANS WHERE THE NEED IS ONLY MAXIMUM DYNAMIC DEFLECTION MAXIMUM DYNAMIC DEFLECTION TO PREVENT POTENTIALLY CATASTROPHIC CROSS MEDIAN ACCIDENTS. SHOULDER 6. NOT RECOMMENDED FOR USE IN AREAS WHERE IT IS LIKELY TO BE HIT EDGE OF PAVEMENT EDGE OF PAVEMENT FREQUENTLY BECAUSE CABLE GUARDRAIL SYSTEMS REQUIRE MORE MAINTENANCE LOPE 4:1 TO 6: PER HIT THAN ANY OF THE SYSTEM. 7. NOT RECOMMENDED FOR HIGH FILL OVER 7.00 M. CABLE GUARDRAIL AT MEDIAN OF DIVIDED HIGHWAYS 1 DGE OF PAVEMENT MORE THAN MAXIMUM PLAN SHOULDER TRAVELED WAY DYNAMIC DEFLECTION ( SEE NOTE 7) 7.00 A SLIP BASE DESIGN ( SEE NOTE 4 ) ELEVATION STRONG CONCRETE BLOCK ( SEE NOTE 5 ) CABLE GUARDRAIL AT OUTER SHOULDER CABLE GUARDRAIL TERMINAL WHITE REFLECTIVE EDGE OF PAVEMENT SHEETING TYPE 1 MORE THAN MAXIMUM DYNAMIC DEFLECTION ( SEE NOTE 7 ) WHITE REFLECTIVE WHITE REFLECTIVE SHEETING TYPE 1 SHEETING TYPE TRAFFIC DIRECTION ---CABLE GUARDRAIL AT RIGID OBJECTS SCALE DETAIL OF REFLECTIVE SHEETING ATTACHMENT SCALE

### NOTES :

- STEEL CABLES SPACED 7.5 CM. TO 10 CM. APART AND MOUNTED ON WEAK POSTS. THE TOP CABLE HEIGHT RANGES FROM 67.5 CM. TO 75 CM. CABLE GUARDRAIL HAVE BEEN SUCCESSFULLY CRASH - TESTED (SEE REMARK 1) OR OTHER CRITERIA OF ACCEPTED INSTITUTE.
- 2. IMPACT PERFORMANCE : THIS SYSTEM WILL GENERALLY REDIRECT VEHICLES IN THE 1,800 - 4,500 ib. RANGE , THE MAXIMUM DYNAMIC DEFLECTION OBSERVED DURING THE STANDARD STRENGTH TESTING IS 11.5 FEET ( 3.50 METERS ).
- 3. THE CABLE GUARDRAIL IS FLARED BACKWARD AT FULL HEIGTH TO AN END POST OFFSET 1.10 METER FROM THE TANGENT LINE, FROM THE END POST, ALL CABLE STRANDS ARE THEN TURNED DOWN AND ANCHORED TO A CONCRETE BLOCK IN THE GROUND.
- 4. THE CABLE SYSTEM IS PLACED ON SUPPOSEDLY " WEAK" POSTS, THE END POST IS FURTHER WEAKENED BY USED OF A SLIP BASE.
- 5. A CRASHWORTHY END TERMINAL IS CRITICAL IN EACH OF THE CABLE SYSTEMS. BOTH TO PROVIDE ADEQUATE ANCHORAGE TO DEVELOP FULL TENSILE STRENGTH IN THE CABLE AND TO MINIMIZE VEHICLE DECELERATIONS FOR IMPACTS ON EITHER END OF AN INSTALLATION.
- 6. CABLE GUARDRAIL WHEN USED IN A MEDIAN, ONE (OR TWO) OF THE CABLES SHALL BE INSTALLED ON THE OPPOSITE SIDE OF EACH POST FROM THE OTHER
- THE DISTANCE SHALL NOT BE LESS THAN THE DYNAMIC DEFLECTION OF THE BARRIER FOR IMPACT BY A FULL - SIZE AUTOMOBILE AT IMPACT CONDITIONS OF APPROXIMATELY 25° AND 60 MPH. (96 KPH.), SOME REDUCTION IN DEFLECTION DISTANCE MAY BE JUSTIFIED IF THE OPERATING SPEED IS LESS THAN 60 MPH. (96 KPH.).
- 8. ALL STEEL SHALL BE GALVANIZED. ZINC COATING SHALL NOT BE LESS THAN 550 GRAMS PER SQUARE METER
- 9. REFLECTIVE SHEETING SHALL CONFORM TO TIS, 606 TYPE 1. ( ESEFFICIENT OF RETRO - REFLECTION LEVEL 1 )
- 10 SPACING OF POSTS SHALL BE REDUCED WITH DECREASING RADIUS OF CURVES AS RECOMMENDED BY THE MANUFACTURER OR PROPORTION ACCORDING TO DWG. NO. RS - 301.
- 11: SPACING OF POSTS SHALL BE REDUCED WITH INCREASING HEIGHT OF FILL IN PROPORTION ACCORDING TO DWG. NO. RS - 301.
- 12. PAYMENT OF CABLE GUARDRAIL SHALL BE MEASURED BY METER INCLUDING END TERMINAL

### REMARK 1 : CRASH TEST CRITERIA

STANDARD CRASH TESTS ARE PRESENTED IN NATIONAL COOPERATIVE HIGHWAY RESEARCH PROGRAM (NCHRP) PEPORT NO. 230 "RECOMMENDED PROCEDURES FOR THE SAFETY PERFORMANCE EVALUATION OF HIGHWAY APPURTENANCES" THIS REPORT CURRENTLY RECOMMENDS TWO TESTS ON STANDARD SECTIONS OF LONGITUDINAL BARRIERS, ONE WITH AN 1,800 Ib (816 KG.) VEHICLE IMPACTING AT 60 MPH. ( 96 KPH. ) AND 15" TO EVALUATE OCCUPANT RISK, AND ONE WITH A 4,500 lb. ( 2,041 KG.) VEHICLE IMPACTING AT 60 MPH. (96 KPH.) AND 25\* TO EVALUATE THE STRUCTURAL INTEGRITY OF THE CABLES.

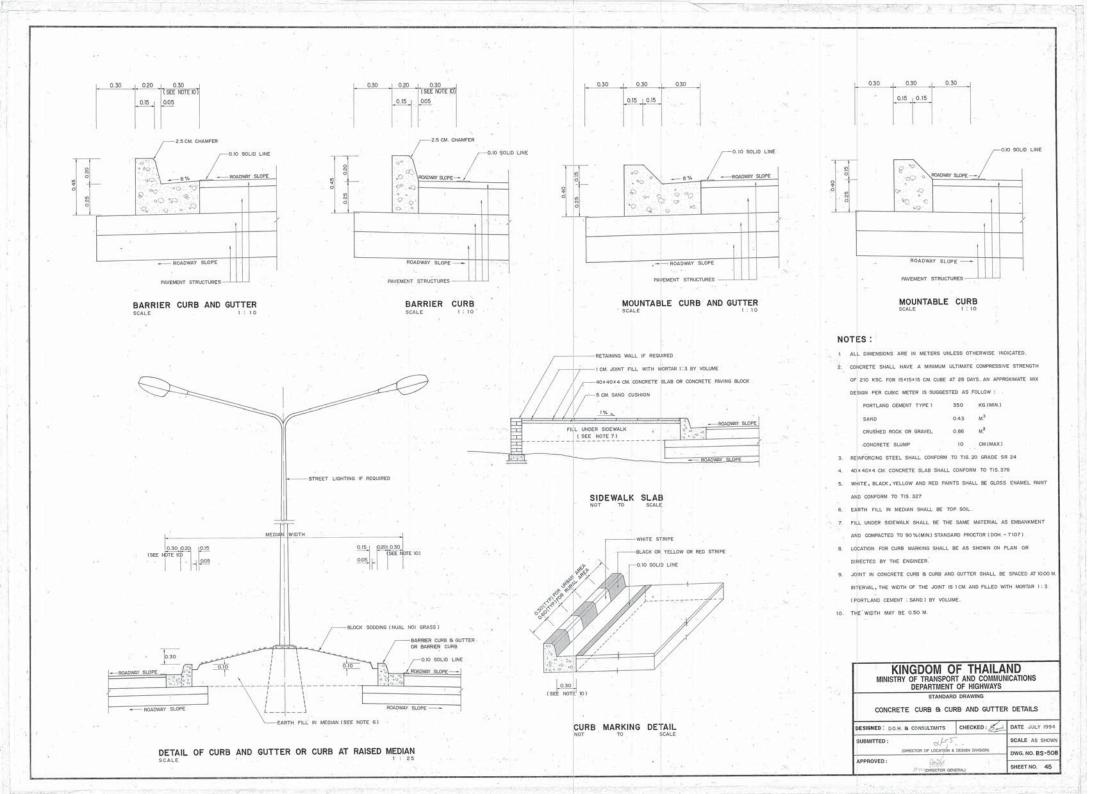
### REMARK 2 :

THIS BARRIER IS SUITABLE ONLY IN SPECIFIC LOCATIONS AND CERTAIN CONDITIONS, IT SHOULD BE APPLIED WITH THE FIRM INFORMATIONS CONCERNED AND RECOMMENDED BY THE ENGINEER.

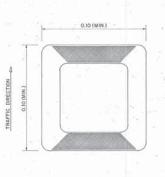
### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS STANDARD DRAWING IMPLEMENT OF CABLE GUARDRAIL

DESIGNED : DO.H. & CONSULTANTS | CHECKED : 8-DATE JULY 1994 P. Burgey.
DIRECTOR OF LOCATION & DESKIN D SCALE NONE DWG. NO. RS-507

Shee PROCIDIRECTOR GENERAL



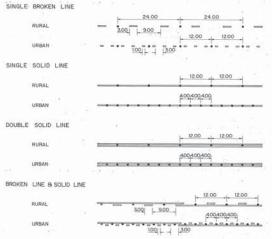
# PLAN OF UNI-DIRECTIONAL TYPE ROAD STUD NOT SINGLE BROKEN LINE RURAL URBAN SINGLE SOLID LINE RURAL URBAN

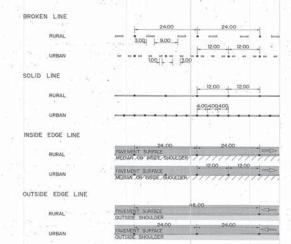




### PLAN OF BI-DIRECTIONAL TYPE ROAD STUD NOT TO SCALE



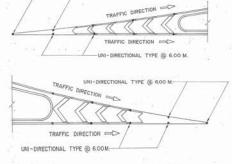


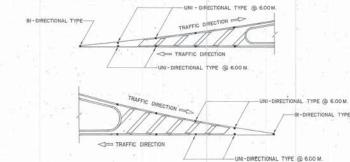


### ROAD STUD AT CENTER LINE OF ROADWAY

-UNI - DIRECTIONAL TYPE @ 6.00 M

### ROAD STUD AT LANE LINE & EDGE LINE





### ROAD STUD AT CHEVRON HATCHING

ROAD STUD AT CROSS HATCHING

### NOTES:

- 1, ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- ROAD STUD SHALL BE MADE OF ALLUMINIUM OR ALLUMINIUM ALLOY.
   SIZE OF THE BASE SHALL BE NOT LESS THAN 100×100 MM. WITH 20-35 MM. HIGH.
   LENGTH OF THE SHANK SHALL BE NOT LESS THAN 50 MM. ROAD STUD SHALL
   WITHSTAND TO IMPACT LOAD WITHOUT DISTORTION OR CRACK.
- REFLECTOR SHALL BE YELLOW OR WHITE REFLECTIVE BEADS WHICH MANUFACTURING EXCLUSIVELY FOR REFLECTOR. AMOUNT OF BEADS SHALL BE NOT LESS THAN 40 PER SIDE, EMBEDED TO THE ROAD STUD.
- THE PROCEDURES OF INSTALLATION
- 4.1 THE HOLE SHALL BE DRILLED WIDER THAN DIAMETER OF THE SHANK ABOUT 3 MM.
- 4.2 MATERIALS IN THE HOLE SHALL BE REMOVED.
- 4.3 EPOXY ADHESIVE SHALL BE APPLIED FULLY INTO THE HOLE.
- 4.4 THE SHANK SHALL BE EMBEDED INTO THE HOLE, THE ROAD STUD SHALL BE
  PRESSED UNTIL EPOXY ADMESIVE SHALL BE BLEEDED TO ADHERE BETWEEN
  BRUSHMENT SUBFACE AND ROAD STUD.
- ROAD STUDS AT CENTER LINE OF ROADWAY SHALL BE BI-DIRECTIONAL TYPE OTHERS SHALL BE UNI-DIRECTIONAL TYPE.
- S. ROAD STUDS SHALL BE INSTALLED BEFORE PC. STATION OF CURVE AND AFTER
  PT. STATION OF CURVE ABOUT 65 METERS.
- INSTALLATION OF ROAD STUDS IN CURVE WITH RADIUS MORE THAN 320 METERS SHALL BE INSTALLED AS TANGENT INSTALLATION.

### TABLE 1 INSTALLATION OF ROAD STUD ON TANGENT

TYPE OF LINE	COLOUR OF	SPACING OF ROAD STUD		LOCATION		
THE OF LINE	REFLECTOR	RURAL	URBAN	LOCATION		
CENTER LINE						
SINGLE BROKEN LINE	YELLOW	24.00	12.00	BETWEEN BROKEN LINE		
SINGLE SOLID LINE	YELLOW	12.00	4.00	ON SOLID LINE		
DOUBLE SOLID LINE	YELLOW	12.00	4.00	BETWEEN DOUBLE SOLID		
FOR MULTI - LANE :	1.00			LINES		
LANE LINE		15				
BROKEN LINE	WHITE	24.00	12.00	BETWEEN BROKEN LINE		
SOLID LINE	WHITE	12.00	6.00	ON SOLID LINE		
EDGE LINE	-					
INSIDE EDGE LINE,	YELLOW	24.00	12.00	ON EDGE LINE		
OUTSIDE EDGE LINE	WHITE	48.00	24.00	ON EDGE LINE		

### TABLE 2 INSTALLATION OF ROAD STUD IN CURVE

TYPE OF LINE	COLOUR OF		ROAD STUD	LOCATION
TIPE OF LINE	REFLECTOR	RADIUS OF CURVE BETWEEN 100 - 300 M.	RADIUS OF CURVE LESS THAN IOOM	LOCATION
BROKEN LINE	THE SAME	12,00	17/1	BETWEEN BROKEN LINE
SINGLE SOLID LINE	COLOUR AS	12.00	4.00	ON SOLID LINE
DOUBLE SOLID LINE	THE LINE	12.00	4.00	BETWEEN DOUBLE SOLID LINES
BROKEN LINE & SOLID LINE	)	12.00	4.00	BETWEEN THE LINES

KINGDOM OF THAILAND
MINISTRY OF TRANSPORT AND COMMUNICATIONS
DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

ROAD STUD

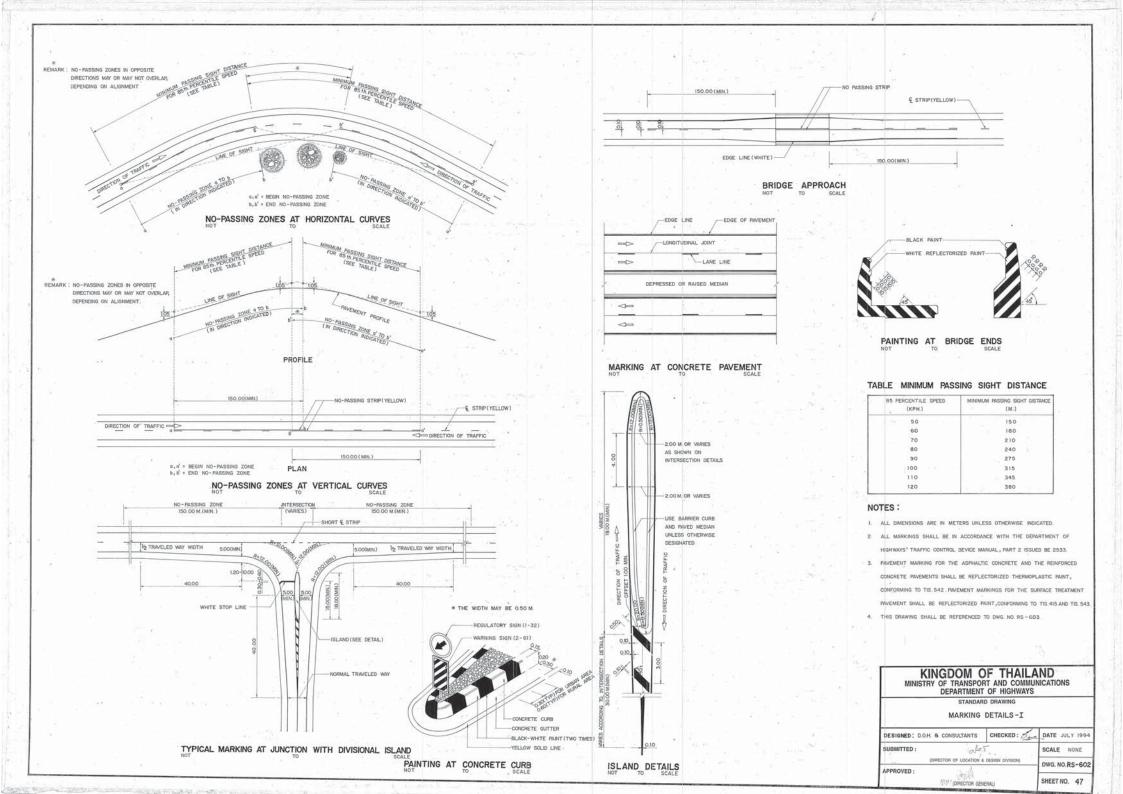
DESIGNED: D.O.M. & CONSULTANTS CHECKED:

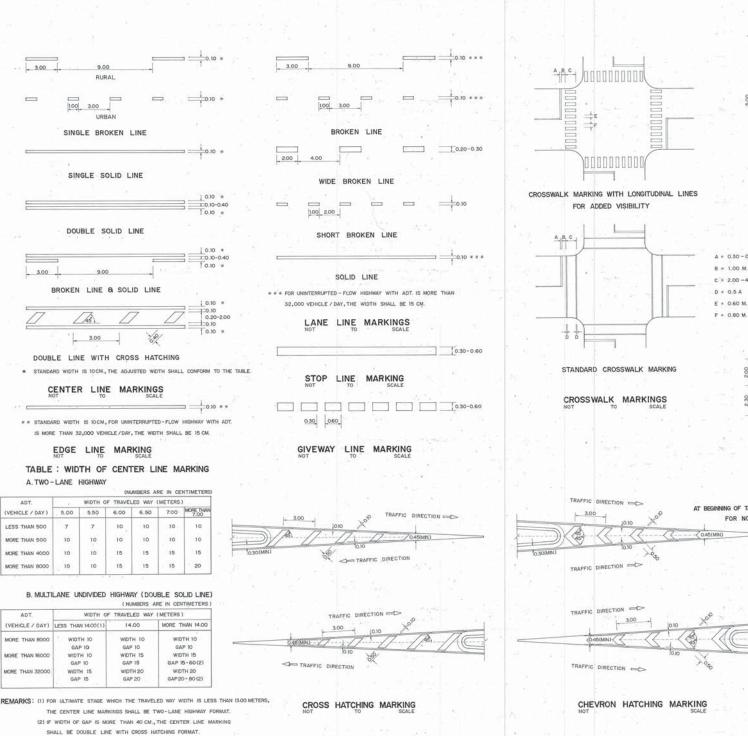
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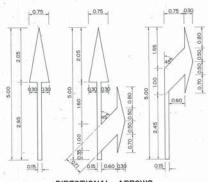
SUBMITTED:

DESIGNED OF LOCATION & DESIGN DIVISION)

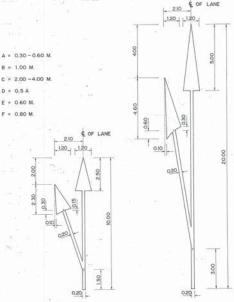
DWG, NO.RS-60







DIRECTIONAL ARROWS



AT BEGINNING OF TAPER FOR DECELERATION LANE, AT BEGINNING OF TAPER FOR DECELERATION LANE, FOR NORMAL INSTALLATION FOR DIVIDED HIGHWAY

LONG DIRECTIONAL ARROWS

### NOTES:

- I. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- 2. THIS DRAWING SHALL BE REFERENCED TO DWG. NO. RS-602

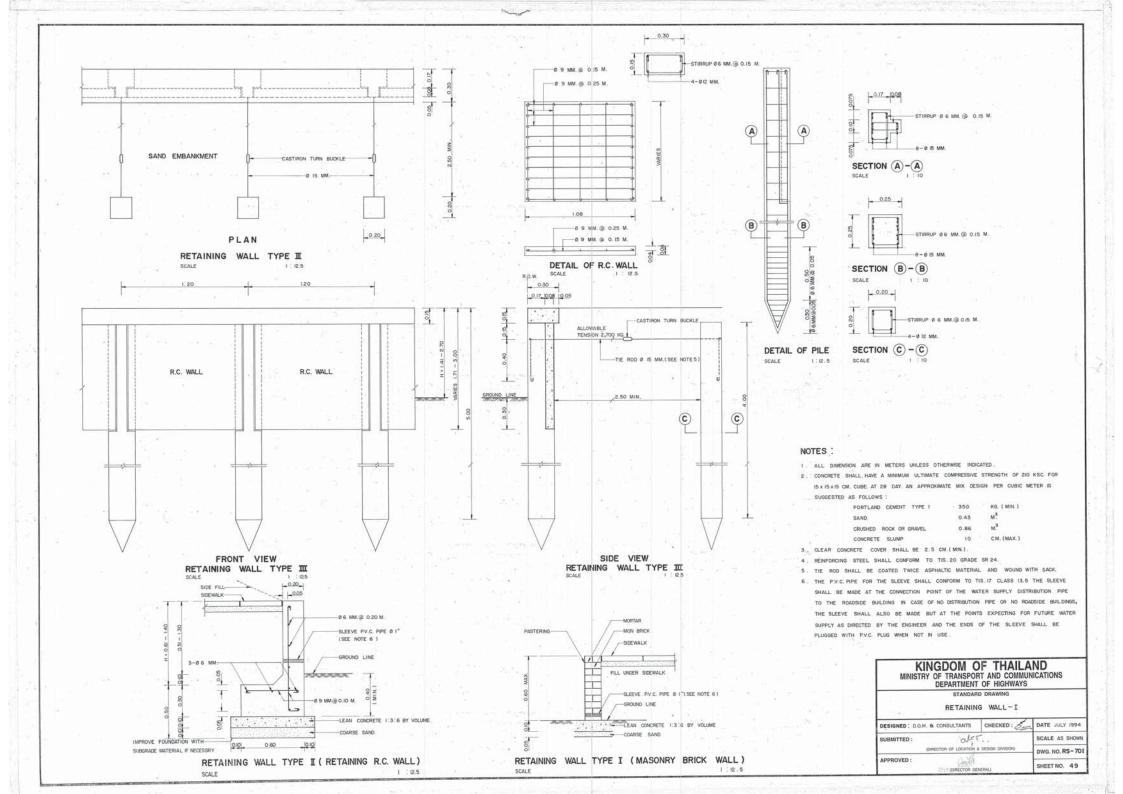
### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

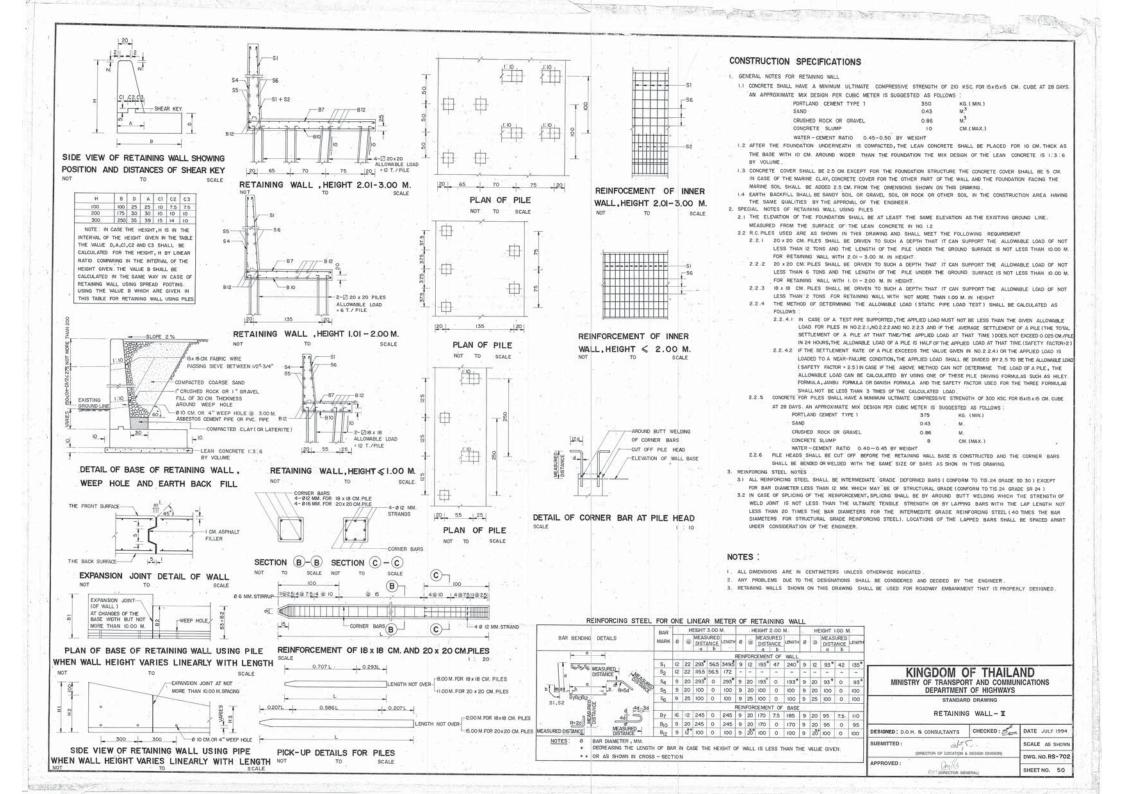
STANDARD DRAWING
MARKING DETAILS-I

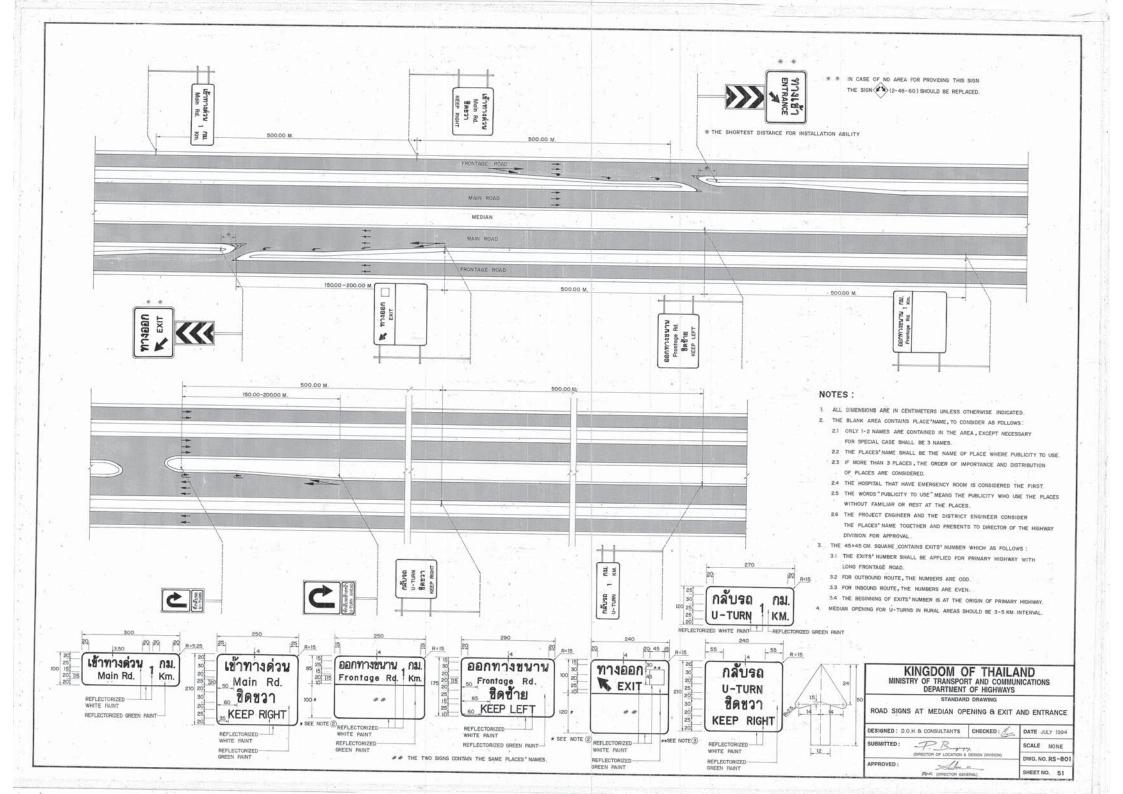
DESIGNED: D.O.H. & CONSULTANTS CHECKED: DATE JULY 1994

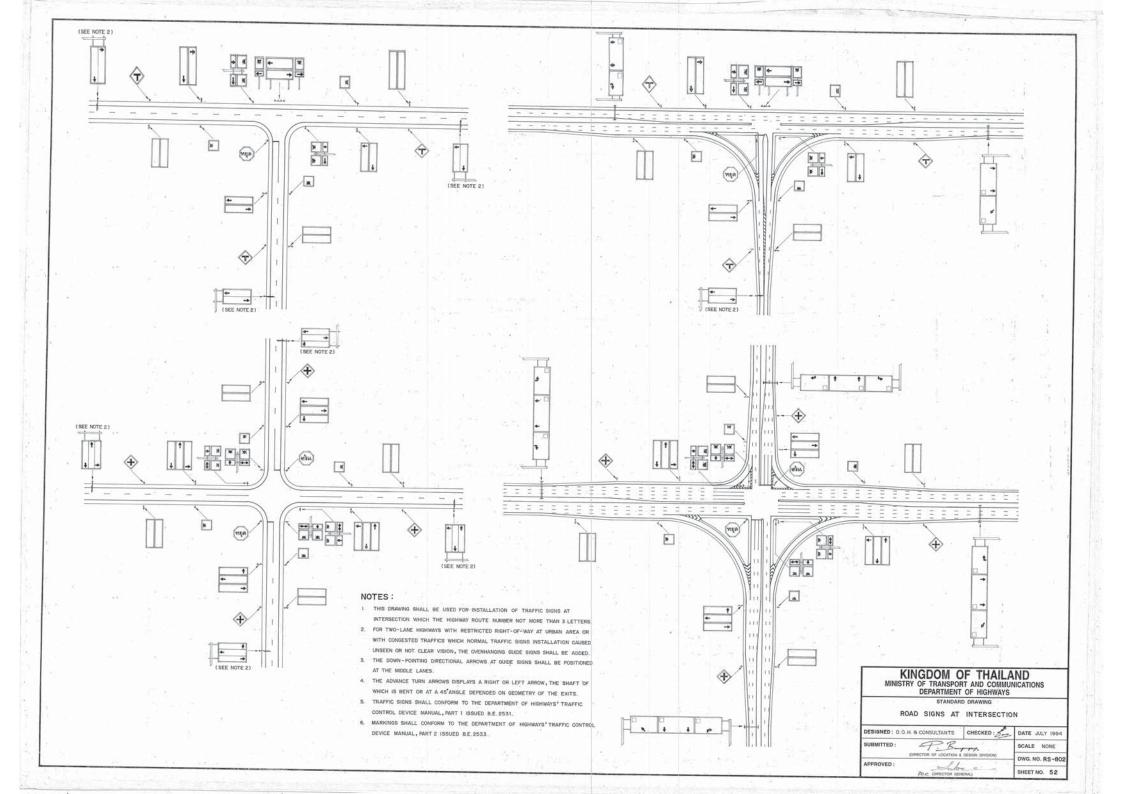
SUBMITTED: SCALE AS SHOWN
prinction of LOCATION A DEBION LOVISION
DWG, NO.RS-603

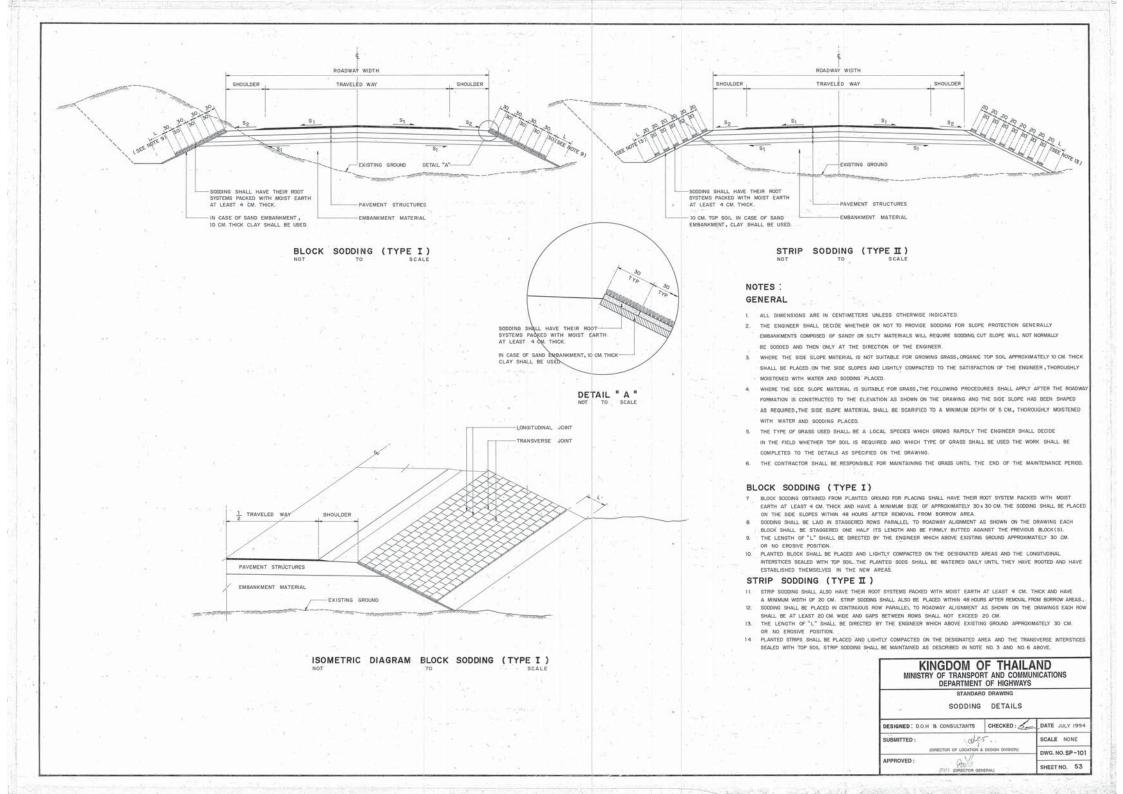
APPROVED: SHEET NO. 48

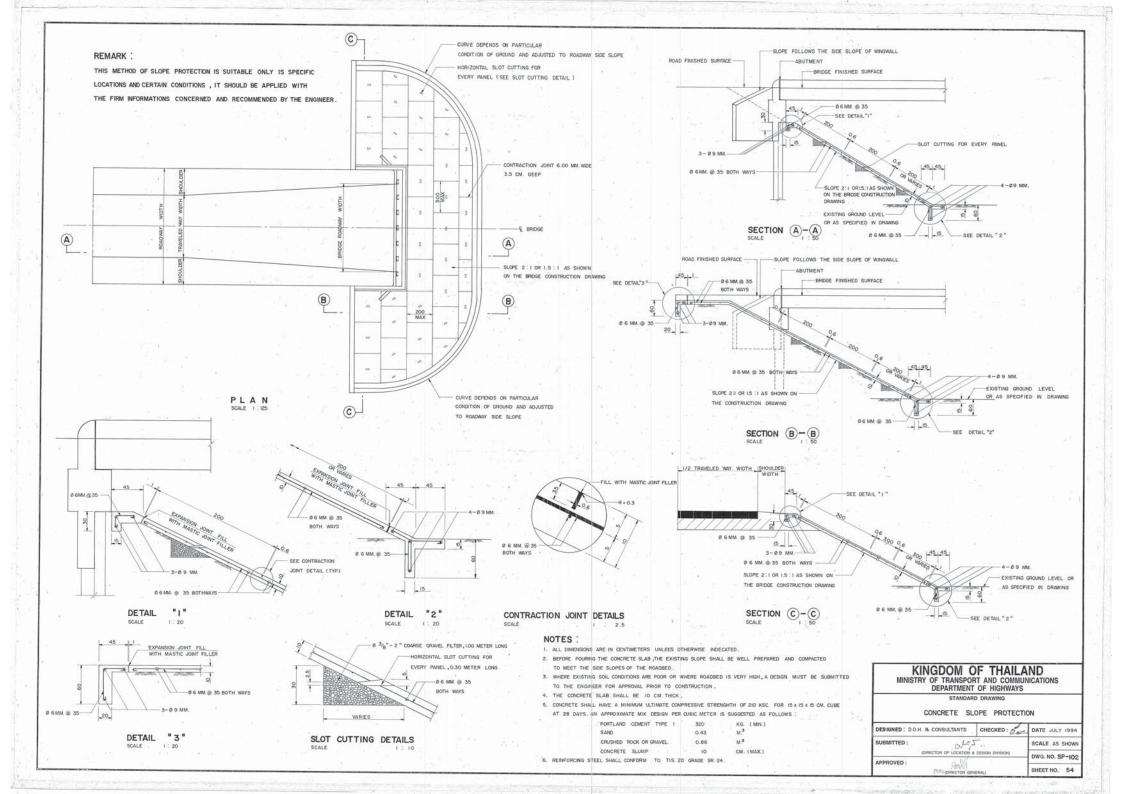


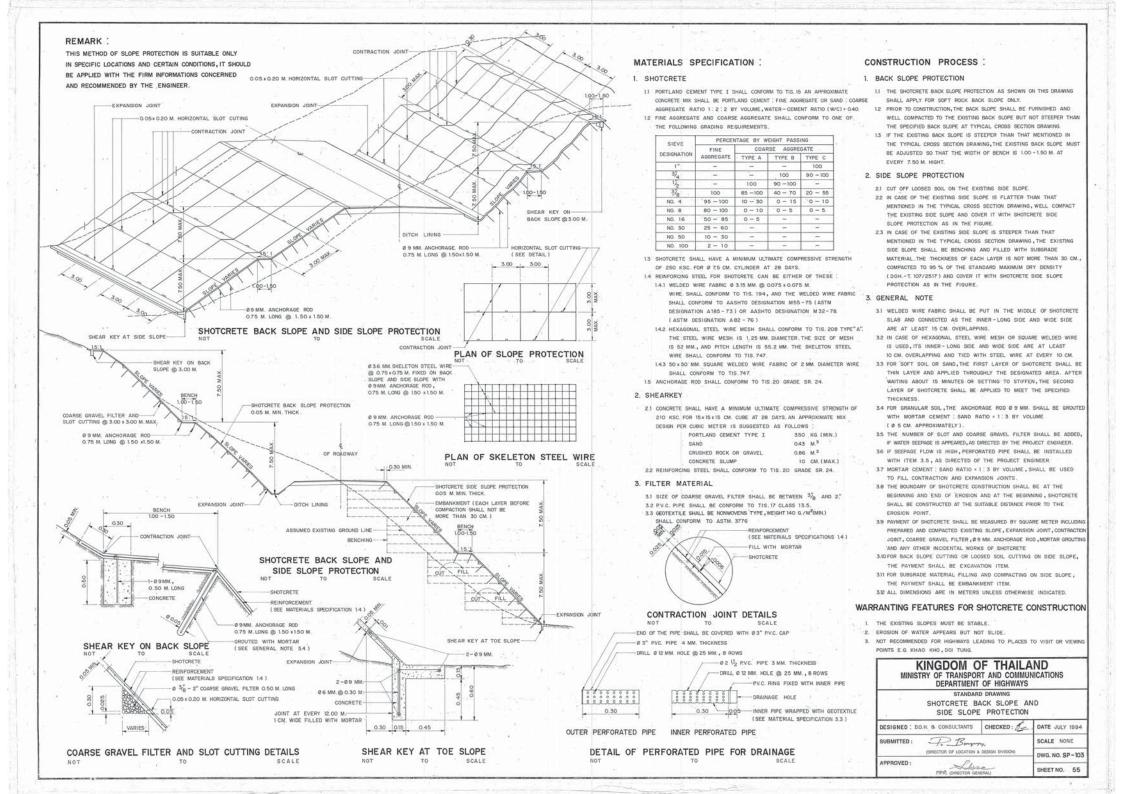


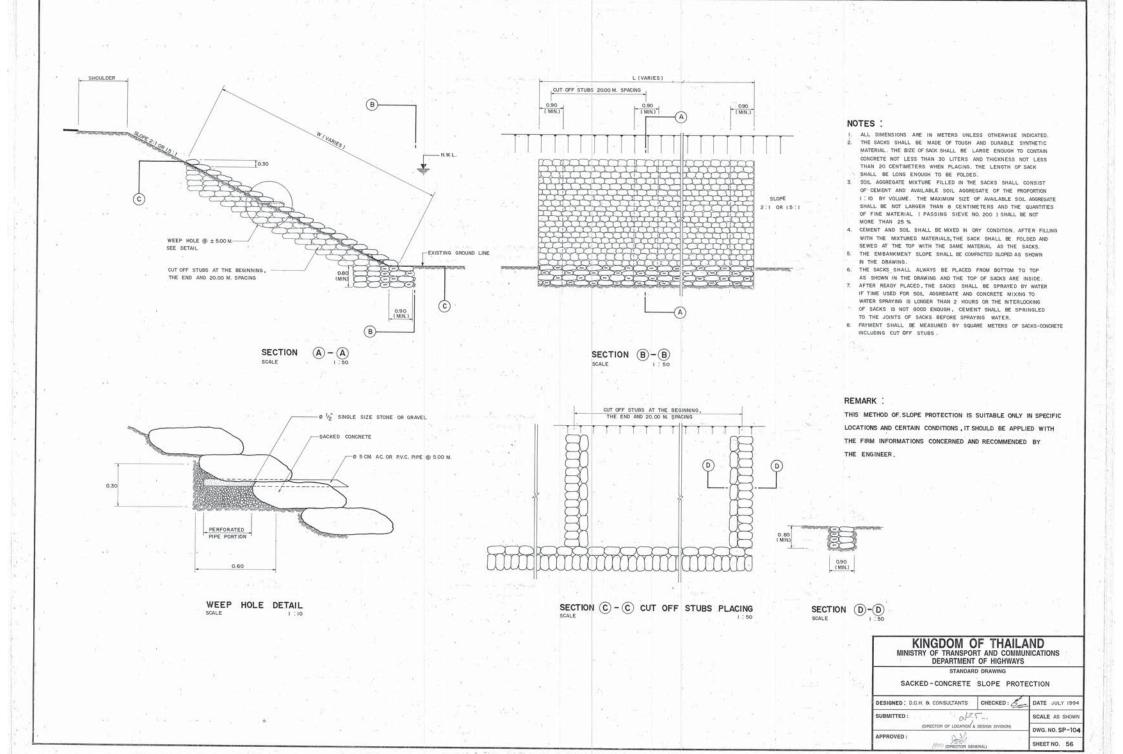


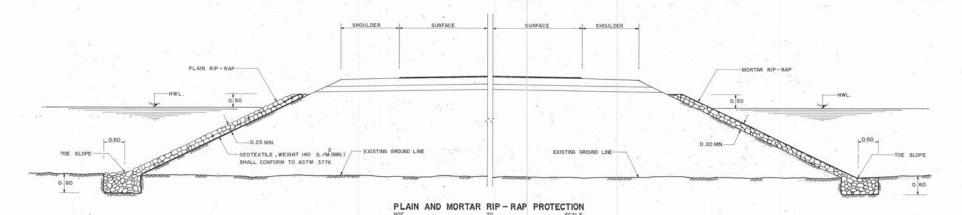












### PLAIN RIP-RAP CONSTRUCTION

- I. EMBANKMENT SLOPES SHALL BE COMPACTED AND SHAPED TO TYPICAL CROSS-SECTION SPECIFIED.
- STONE USED FOR PLAIN RIP-RAP SHALL WEIGH BETWEEN 25-70 KG. EACH AND AT LEAST 75 PERCENT SHALL WEIGH MORE THAN 45 KG. WITH KEY STONE SIZE IN SUITABLE.
- THE LARGEST STONE SHALL BE PLACED FIRST. THE INTERSTICES SHALL BE FILLED WITH SMALL STONES.
   THE THICKNESS OF PLAIN RIP -RAP SHALL NOT LESS THAN 25 CENTIMETERS.
- 4. THE PLACING OF STONES SHALL BE AT LEAST 60 CM. UNDER EXISTING GROUND LEVEL.
- 5. STONES SHALL BE CONSIDERED TO FILL AT TOE SLOPE AS SUITABLY IN CASE OF EXISTING GROUND ARE SOFT.

### MORTAR RIP-RAP CONSTRUCTION

- I. EMBANKMENT SLOPES SHALL BE COMPACTED AND SHAPED TO TYPICAL CROSS SECTION SPECIFIED.
- STONE USED FOR MORTAR RIP-RAP SHALL WEIGH BETWEEN 15-40 KG, EACH AND AT LEAST SO PERCENT SHALL WEIGH MORE THAN 30 KG. WITH KEY STONE SIZE IN SUITABLE.
- THE LARGEST STONE SHALL BE PLACED FIRST. THE INTERSTICES SHALL BE FILLED WITH SMALL STONES.
   THE THICKNESS OF MORTAR RIP-RAP SHALL NOT LESS THAN 20 CENTIMETERS.
- 4: THE VOIDS BETWEEN STONE SHALL BE FILLED WITH MORTAR CEMENT : SAND RATIO 1:1 BY VOLUME .
- 5. THE PLACING OF STONES SHALL BE AT LEAST 60 CM. UNDER EXISTING GROUND LEVEL.
- 6. STONE SHALL BE CONSIDERED TO FILL AT TOE SLOPE AS SUITABLY IN CASE OF EXISTING GROUND ARE SOFT.

### NOTES :

I. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.

### REMARK :

THIS METHOD OF SLOPE PROTECTION IS SUITABLE ONLY IN SPECIFIC LOCATIONS AND CERTAIN CONDITIONS, IT SHOULD BE APPLIED WITH THE FIRM INFORMATIONS CONCERNED AND RECOMMENDED BY THE ENGINEER.

### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS

MINISTRY OF TRANSPORT AND COMMUNICATIONS
DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

RIP - RAP PROTECTION FOR EMBANKMENT SLOPE

DESIGNED: DO.H & CONSULTANTS CHECKED: DATE JULY 1994

SUBMITTED: F. B. GENERAL PARTIES.

SCALE NONE

DWG. NO. SP-105

SHEET NO. 57

### SPECIAL PROVISION :

- I, GABIONS SHALL BE AS SPECIFIED IN THE "SPECIFICATION OF GABIONS", AND BE OF THE SIZES STATED IN THE BILL OF QUANTITIES .
- 2. ASSEMBLY AND ERECTION OF GABIONS SHALL BE AS SPECIFIED IN THE " SPECIFICATION FOR GABIONS CONSTRUCTION ".

### 3. FILLING MATERIALS :

FILLING MATERIALS SHALL NOT BE LESS THAN 100 MM. OR LARGER THAN 250 MM. AND BE OF TYPES A OR B AS SPECIFIED IN THE "TABLE OF FILLING MATERIALS " WHERE THE SPECIFIED FILLING MATERIALS CAN NOT BE OBTAINED, FILLING MATERIALS OF TYPES C , D OR E MAY BE USED BY THE APPROVAL OF THE ENGINEER OF DESIGN .

### 4. PERVIOUS BACKFILL :

TYPES OF PERVIOUS BACKFILL MATERIALS SHALL BE COARSE-GRAINED SOILS OR GRANULAR MATERIALS I.E. CLEAN SAND, GRAVEL OR CRUSHED ROCK, SHALL BE HARD DURABLE AND CLEAN AND SHALL BE FREE FROM ORGANIC MATERIALS, CLAY BALLS AND OTHER DELETERIOUS SUBSTANCE LATERITE OR CONCRETIONARY MATERIAL SHALL NOT BE USED .

SAND USED FOR PERVIOUS BACKFILL MATERIAL SHALL CONFORM TO THE FOLLOWING GRADING REQUIRMENTS.

SIEVE DESIGNATION	PERCENTAGE	BY WEIGHT PASSING
.3/8*		100
NO. 4	95	- 100
NO. 16	45	- 80
NO. 50	10	- 30
NO. 100	2	- 10

GRAVEL AND CRUSHED ROCK SHALL CONFORM TO ONE OF THE FOLLOWING GRADING REQUIREMENTS

SIEVE DESIGNATION	PERCENTAGE BY WEIGHT PASSING								
	TYPE B	TYPE C	TYPE D	TYPE E					
2"	100	8	b 4.	- 140					
1 1/2"	70-100	100	-	-					
42.	55-85	75-100	100	-					
3/4"	50-80	60 – 90	70 -100	100					
3/8"	40-70	45 - 75	58 -75	-					
NO. 4	30 - 60	30 - 60	35 -65	45 - 80					
NO. 10	20 - 50	20 - 50	25 -50	30 - 60					
NO. 40	10 -30	10 -30	15 -30	20 - 35					
NO. 200	0 - 2	0 - 2	0 - 2	0 - 2					

### 5. BACKFILL COMPACTION

- d) BACKFILL MATERIALS SHALL BE PLACED IN THIN LAYERS NOT THICKER THAN 20 CM. EACH. EACH LAYER SHALL BE COMPACTED BEFORE THE NEXT ONE IS PLACED.
- b) BACKFILL MATERIALS SHALL BE COMPACTED BY HAND- OPERATED OR OTHER LIGHTWEIGHT COMPACTORS CARE MUST BE MADE NOT TO OVERCOMPACT THE BACKFILL SINCE OVERCOMPACTION WILL CAUSE EXCESSIVE EARTH PRESSURE.
- 6. THE ENGINEER OF DESIGN SHALL MAKE ANY VARIATION OF THE FORM QUALITY OR QUANTITY OF THE WORKS OR ANY PART THERE OF THAT MAY BE NECESSARY.
- 7. NO ANY VARIATION OR AMENDMENT SHALL BE MADE WITHOUT THE APPROVAL OF THE ENGINEER OF DESIGN.

### REMARK :

THIS METHOD OF SLOPE PROTECTION IS SUITABLE ONLY IN SPECIFIC LOCATIONS AND CERTAIN CONDITIONS , IT SHOULD BE APPLIED WITH THE FIRM INFORMATIONS CONCERNED AND RECOMMENDED BY THE ENGINEER

	DESCRIPTION	DESCRIPTION			QUANTITY					
ITEM.	TYPES	SIZES	UNIT.	APRON LAYER	lst LAYER	2nd LAYER	3 rd LAYER	4th LAYER	5th LAYER	TOTAL
L	GABIONS GALVANIZED 80 x	4 x 1 x 0.5 M.	NO.	292	-	V. <del></del> -	-	-	-	292
2	100 MM. MESH, 2.7 MM. WIRE	2 x 1x 0.5 M.	NO.	4	-	-	-		=	4
3	CORE WITH NECESSARY BINDING WIRE	1.5x1x0.5M.	NO.	36	-		-	=	- 1	36
4.7	GABIONS GALVANIZED 100 x	4x1x I M.	NO.		152	70	105	36	68	431
5	120 MM. MESH, 2.7 MM. WIRE	3x1x1 M.	NO.	-	12	. 6	8	-	25	. 14
6	CORE WITH NECESSARY	2x Ix I M.	NO.	-	-	1	-	1.	8	10
7.	BINDING WIRE	1.5 x l x l M.	NO.	-	0-	150	77	146		296
8.	ASSEMBLE, PLACE IN POSITION AND PACK WITH FILLING MATERIALS IN ITEM 1 TO 7 INCLUDING FOR TYING		CU.M.		t d				i s	2,845
	DOWN LIDS AFTER FILLING									
9.	PERVIOUS BACKFILL MATERIALS PLACE IN POSITION AND COMPACT		CU.M				-			1,800
10. ,	EMBANKMENT MATERIAL		CU.M		0.5	- 4	(4)			APPROX 3,700

	FILLING MATERIAL	WEIGHT OF MATERI
TYPE	FILLING MATERIAL	( KG. /CU.M.)
Δ	BASALT	1,650
	GRANITE	1,600
В	SHINGLE & SLAG	1,500
ь	LIME STONE	1,440
c	SAND STONE	1,390
	BROKEN CONCRETE	1,340
D	BRICK	1,240
Ε,-	ORTHERS AS APPROVAL	OF THE LOCATION AND
	DESIGN DIVISION	

### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

SPECIAL PROVISION OF GABIONS

DESIGNED: D.O.H. & C	ONSULTANTS CHECKED:	DATE JULY 1994
SUBMITTED:	of 15 -	SCALE NONE
	OR OF LOCATION & DESIGN DIVISION	DWG. NO.SP-106
APPROVED:	DIRECTOR GENERAL)	SHEET NO. 58

### TABLES OF WEIGHTS AND MEASUREMENTS

### GALVANIZED BOX GABIONS WITH DIAPHRAGMS MESH TYPE 100×120 MM. GALVANIZED TO SPECIFICATION

DIM	ENSIONS (	M.)	20202000	APPROXIMATE	WEIGHT (KG)
LENGTH	WIDTH	HEIGHT	DIAPHRAGMS	WIRE Ø 2.7 MM.	WIRE Ø 3.0MM
1.5	1 -	0.5	=	8.600	10.300
2	- 1	0.5	1	10.800	13.500
3	1 -	0.5	2	14.400	19.200
4	- 1	0.5	3	19.000	25.000
1.5	1	1	1577	11.800	13.800
2.	1	1, ,	1,	15.200	18.200
3	1 .	1	2	20.400	25.800
4	1	1	3	27,000	34.000

THE WEIGHTS INDICATED IN THE TABLE DO NOT INCLUDE BINDING AND CONNECTING WIRE, AND ARE SUBJECT TO THE USUAL TOLERANCES.

### MESH TYPE 80×100 MM. GALVANIZED TO SPECIFICATION

DIN	MENSIONS	(M.)	1	APPROXIMATE	WEIGHT (KG.)
LENGTH	WIDTH	HEIGHT	DIAPHRAGMS	WIRE Ø 2.7MM.	WIRE Ø 3.0 MM
1.5.	1	0.3	1753000	8.600	10.500
2	1	0.3	1	10, 1 00	12.500
3	1	0.3	2	14,600	17,800
4	1	0.3	3	18,800	23.100
1.5	1	0.5	_	9.200	11.500
2	1	0.5	1	12.600	15.400
3	1	0.5	2	18.000	21.800
4	1	0.5	3	23.200	28.200
1.5	1	- 1		12,800	16.000
2	1	1	1.	17.300	21.300
3	1	1	2	24.700	30.000
4	1	1	3	32.000	39.000

### PVC. COATED BOX GABIONS MESH TYPE 100 × 120 MM. GALVANIZED TO SPECIFICATION

DI	MENSIONS (	M.)		APPROXIMATE	
LENGTH	WIDTH	HEIGHT	DIAPHRAGMS	WEIGHT (KG.)	
1.5	1	0.5		8,800	
2	1	0.5	1	12.800	
3	.1	0.5	2	18.000	
4	1	0.5	3	23.200	
1.5	1	19		12.800	
_ 2	1	1.	1	17.000	
-3	1	. 1	2	24.000	
4	1	1	3	31.500	

THE WEIGHTS INDICATED IN THE TABLE
DO NOT INCLUDE BINDING AND
CONNECTING WIRE, AND ARE SUBJECT
TO THE USUAL TOLERANCES.

### MESH TYPE 80×100 MM GALVANIZED TO SPECIFICATION

DI	MENSIONS (	M)		APPROXIMATE	
LENGTH	WIDTH	HEIGHT	DIAPHRAGMS	WEIGHT (KG)	
1.5	1	0.3	-	9.100	
2	1	0.3	2 11 11	11.500	
3	1	0.3	2	16.700	
4	1	0.3	3	21.700	
1.5	1	0.5	-	10.800	
2	1	0.5	1	14.500	
3	1	0.5	2	21,000	
4	1	0.5	3 '	27.000	
1.5	1	1	_	15,400	
2	1	1.	331	20.000	
3	-1	1.	2	28.800	
4	1	1	3	37.200	

### 1. GENERAL

GABIONS SHALL CONSIST OF WOVEN STEEL WIRE MESH BOXES
OF APPROVED MODULE AS SPECIFIED IN THE CIRCULAR OF THE
SENIOR COUNCIL OF THE ITALIAN LL.PP.NO. 2078 OF THE 278.1962
OR BY FEDERAL SPECIFICATION QQ.-W-461g IN THE UNITED STATES
OF AMERICA, OR BY THE BRITISH STANDARD INSTITUTION
443-1961 AND OR SIMILAR APPROVED, AND BE OF
THE SIZES STATED IN THE "TABLES OF WEIGHTS AND MEASUREMENTS."
PARTICULAR CARE SHALL BE EXERCISED THROUGHOUT CONSTRUCTION
TO ENSURE TIGHTNESS OF MESH, WELL PACKED FILLING WITH
MINIMUM OF VOIDS, AND SECURE LACING. THE EXPOSED FACES OF
COMPLETED WORK SHALL PRESENT A NEAT FACE AND LINE, FREE OF
EXCESSIVE BULGES OR DEPRESSIONS.

### 2. GABION FABRIC

- (a) MESH: MESH OPENINGS SHALL BE HEXAGONAL IN SHAPE, THE MINIMUM DIMENSION OF WHICH SHALL NOT EXCEED 105 MM. IN THE CASE OF 100 x 120 MM. MESH, AND NOT EXCEED 83 MM. IN THE CASE OF 80 x 100 MM. MESH.
- (b) MESH JOINTS: ALL JOINTS SHALL BE FLEXIBLE AND CONSIST OF NOT LESS THAN ONE AND ONE HALF FULL TURNS.
- (c) GALVANIZING: ALL WIRE USED SHALL BE GALVANIZED TO BSS 443/1969 OR EQUIVALENT, PRIOR TO WEAVING OF THE MESH.

### GALVANIZED GABIONS

- (d) MESH WIRE: THE DIAMETER OF THE MESH WIRE SHALL BE THAT STATED IN THE RELEVANT ITEM IN THE BILL OF QUANTITIES (± 2.5%)
- (e) BINDING WIRE: THE DIAMETER OF THE WIRE USED FOR BINDING AND LACING SHALL BE 22MM. (±2.5%)
- (f) SELVEDGE WIRE; SELVEDGE WIRE 3.4 MM. DIAMETER WHERE THE MESH WIRE DIAMETER IS 2.7 MM. AND 3.9 MM. DIAMETER WHERE THE MESH WIRE DIAMETER IS 3.00 MM. SHALL BE INCORPORATED ALONG THE EDGES OF THE WIRE MESH.

### PVC. COATED GABIONS

- (9) PVC. COATING: WHERE STATED IN THE BILL OF QUANTITIES OR WHERE SPECIFIED BY THE ENGINEER, ALL WIRE SHALL BE ADDITIONALLY COATED WITH A MINIMUM 0.55 MM. THICKNESS OF BLACK PVC. WHICH SHALL BE CAPABLE OF RESISTING DELETERIOUS EFFECTS OF IMMERSION IN SALT WATER, EXPOSURE TO ULTRA VIOLET LIGHT AND ABRASION TO THE SATISFACTION OF THE ENGINEER, AND RETAIN THESE CHARACTERISTICS AFTER A PERIOD OF NOT LESS THAN 3,000 HOURS UNDER TEST IN ACCORDANCE WITH ASTM. E 42-65.
- (h) MESH WIRE: THE DIAMETER OF THE MESH WIRE CORE SHALL BE THAT STATED IN THE BILL OF QUANTITIES.
- BINDING WIRE: THE DIAMETER OF THE CORE OF THE PVC. COATED WIRE USED FOR BINDING AND VACING SHALL BE 2.2 MM.
- (j) SELVEDGE WIRE; SELVEDGE WIRE WHOSE CORE DIAMETER IS 3.4 MM. SHALL BE INCORPORATED ALONG THE EDGES OF THE WIRE MESH EXCEPT IN THE CASE OF THE 80 x 100 MM. MESH IN 2.2 MM. WIRE CORE, WHERE IT SHALL BE 2.7 MM.

### REMARK :

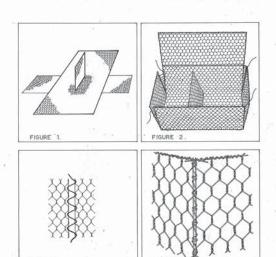
THIS METHOD OF SLOPE PROTECTION IS SUITABLE ONLY IN SPECIFIC LOCATIONS AND CERTAIN CONDITIONS, IT SHOULD BE APPLIED WITH THE FIRM INFORMATIONS CONCERNED AND RECOMMENDED BY THE ENGINEER.

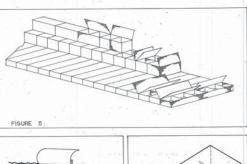
### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

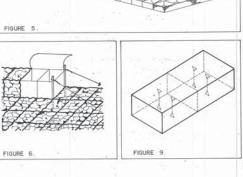
STANDARD DRAWING

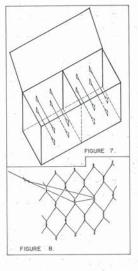
SPECIFICATION OF GABIONS

DESIGNED : D.O.H.	& CONSULTANTS	CHECKED:	DATE JULY 1994
SUBMITTED:	SCALE NONE		
NOT COMPANY OF	HECTOR OF LOCATION	A DESIGN DIVISION)	DWG. NO. SP - 107
APPROVED:	Ith to operation of	MERALI	SHEET NO. 59









### GENERAL NOTES :

FIGURE 3

- I. THE SPECIFICATION COVERS CONSTRUCTION IN BOTH GALVANIZED AND PVC. COATED GABIONS.
- CLAUSE 6 SHOULD ONLY BE INCLUDED WHERE THE GABION MESH MUST BE CUT AND FOLDED TO FORM MITTE JOINTS, ANGLES, CURVES OR SLOPES IN THE STRUCTURES WHICH ARE NOT POSSIBLE TO OBTAIN WITH THE STANDARD RECTANGULAR GABIONS.

EIGHDE 4

### ASSEMBLY

PRIOR TO ASSEMBLY, THE GABION MATERIAL SHALL BE OPENED OUT FLAT ON THE GROUND AND STRETCHED TO REMOVE ALL KINKS AND BENDS. (FIGURE.)

THE GABION BOXES SHALL THEN BE ASSEMBLED INDIVIDUALLY, BY RAISING THE SIDES, ENDS AND DIAPHRAGMS, ENSURING THAT ALL CREASES ARE IN THE CORRECT POSITION AND THAT THE TOPS OF ALL FOUR SIDES ARE EVEN.

THE FOUR CORNERS OF THE GABION BOXES SHALL BE LACED FIRST, FOLLOWED BY THE EDGES OF INTERNAL DIAPHRAGMS TO THE SIDES (FIGURE 2)

IN ALL CASES , LACING SHALL COMMENCE AT THE TOP OF THE BOX BY TWISTING THE END OF THE LACING WIRE AROUND THE SELVEDGE. IT SHALL THEN BE PASSED ROUND THE TWO EDGES BEING JOINED, THROUGH EACH MESH IN TURN, AND SECURELY TIED OFF AT THE BOTTOM. THE ENDS OF ALL LACING WIRES SHALL BE TURNED TO THE INSIDE OF BOX ON COMPLETION OF EACH LACING OPERATION. (FIGURE 3 & FIGURE 4.)

### 2. ERECTION

ONLY ASSEMBLED BOXES, OR GROUPS OF BOXES, SHALL BE POSITIONED IN THE STRUCTURE.

THE SIDE, OR END, FROM WHICH WORK IS TO PROCEED, SHALL BE SECURED EITHER TO COMPLETED

WORK, OR BY ROOS OR STAKES DRIVEN INTO THE GROUND AT THE CORNERS. THESE MUST BE

SECURED AND REACH AT LEAST TO THE TOP OF THE GABION BOX.

FURTHER GABION BOXES SHALL THEN BE POSITIONED IN THE STRUCTURE AS REQUIRED, EACH
BEING SECURELY LACED TO THE PRECEEDING ONE AT ALL CORNERS AND DIAPHRAGM POINTS.

(FIGURE 5 )

### 3. STRETCHING

FINAL STRETCHING OF THE GABION BOXES SHALL BE CARRIED OUT USING A WIRE STRAINER OR WINCH OF AT LEAST ONE TON CAPACITY, FIRMLY SECURED TO THE FREE END OF THE ASSEMBLED GABION BOXES. (FIGURE, 6.)

WHILST UNDER TENSION THE GABION BOXES SHALL BE SECURELY LAGED ALONG ALL EDGES, (TOP, BOTTOM AND SIDES) AND AT DIAPHRAGM POINTS, TO ALL ADJACENT BOXES.

TIGHTNESS OF MESH, WELL PACKED FILLING AND SECURE LACING IS ESSENTIAL IN ALL STRUCTURES.

### 4. FILLING

FILLING SHALL BE CARRIED OUT ONLY WHILST GABION BOXES ARE UNDER TENSION.

FILLING MATERIAL SHALL NOT BE LESS THAN 100 MM. OR LARGER THAN 250 MM. AND

SHALL BE SO PLACED TO PRODUCE A NEAT FACE AND LINE, WITH A MINIMUM OF VOIDS.

WHERE SUITABLE FILLING MATERIAL CANNOT BE OBTAINED, THE ENGINEER MAY APPROVE

A PERCENTAGE OF SMALLER FILLING MATERIAL IN WHICH CASE LARGE MATERIAL SHALL BE

A PERCENTAGE OF SMALLER FILLING MATERIAL IN WHICH CASE LARGE MATERIAL SHALL BE
PLACED ON OUTER EXPOSED FACES, AND SMALLER MATERIAL ON THE INSIDE AS A "CORE".

ONLY MECHANICAL EQUIPMENT APPROVED BY THE ENGINEER MAY BE USED FOR FILLING OPERATIONS.

INTERNAL HORIZONTAL BRACING WIRE (USE BINDING WIRE) SHALL BE PROVIDED AT 330 MM. VERTICAL CENTERS IN I M. DEEP UNITS, AT A RATIO OF 4 TO 1 CU.M. (FIGURE.7) THESE BRACING WIRES SHALL BE WRAPPED ROUND TWO MESH WIRES (FIGURE.8) AND EXTENDED FROM FRONT TO BOCK, SO POSITIONED TO ENSURE A NEAT FACE AND LINE, FREE OF EXCESSIVE BULGES AND DEPRESSIONS ON COMPLETION TO THE SATISFACTION OF THE ENGINEER. GABION BOXES SHALL BE FILLED IN STAGES AND HORIZONTAL BRACING. WIRES INSERTED AS. FILLING IS BROUGHT UP.

SIMILAR BRACING WIRES, USED VERTICALLY (FIGURE 9) MAY BE REQUIRED IN 0.5 M. DEEP GABIONS, AT SPACINGS SPECIFIED BY THE ENGINEER, WHERE SUCH UNITS ARE USED IMMEDIATELY DOWN - STREAM OF WEIRS OR IN OTHER CASES WHERE WATER FALLS DIRECTLY ON TO THE GABIONS OR WHERE A NEAT FACE IS REQUIRED.

TENSION ON THE GABION BOXES SHALL BE RELEASED ONLY WHEN SUFFICIENTLY FULL TO PREVENT THE MESH FROM SLACKENING.

GABION BOXES SHALL BE OVERFILLED BY 20 TO 50 MM. ABOVE THEIR TOPS TO ALLOW FOR SUBSEQUENT SETTLEMENT, IOO MM. FILLING MATERIAL MAY BE USED FOR THIS PURPOSE.

### 5. FINAL WIRING

CLOSING AND WIRING DOWN OF LIDS SHALL PROCEED AS SOON AS PRACTICABLE AFTER

FILLING OPERATIONS, AND CERTAINLY IN THE LIKELIHOOD OF STORMS OR FLOODS DURING CONSTRUCTION

LIDS SHALL BE STRETCHED TIGHT OVER THE FILLING WITH BARS AND WIRED DOWN SECURELY

THROUGH EACH MESH ALDING ALL EDGES, ENDS AND DIAPHRAGMS. THE ENDS OF ALL TYING AND

BRACING WIRES SHALL BE TURNED INTO THE GABION BOX ON COMPLETION OF ALL LACING OPERATIONS.

TIGHTNESS OF MESH, WELL PACKED FILLING AND SECURE LACING IS ESSENTIAL IN ALL STRUCTURES.

### 6. CUTTING AND FOLDING MESH

WHERE SHOWN ON THE DRAWINGS OR OTHERWISE DIRECTED BY THE ENGINEER, THE GABION MESH SHALL BE CUT, FOLDED AND WIRED TOGETHER TO FORM MITER JOINTS, ANGLES, CURVES OR SLOPES WHICH ARE NOT POSSIBLE TO OBTAIN IN THE STRUCTURES WITH THE STANDARD RECTANGULAR GABIONS. THE MESH MUST BE CLEANLY CUT, AND THE SURPLUS MESH CUT COMPLETELY OUT, OR FOLDED BACK OR ON TO, AND NEATLY WIRED TO AN ADJACENT GABION FACE. THE CUT EDGES OF THE MESH SHALL BE SECURELY LACED TOGETHER WITH BINDING WIRE IN THE MANNER SPECIFIED UNDER ASSEMBLY ABOVE.

THE ASSEMBLY, ERECTION, STRETCHING, FILLING AND FINAL WIRING OF THE RE-SHAPED GABIONS SHALL OTHERWISE BE CARRIED OUT AS SPECIFIED ABOVE.

### 7. SPECIAL FINISH

WHERE SPECIFIED BY THE ENGINEER THE OUTER FACE OF THE GABIONS SHALL BE CAREFULLY PACKED OUT BY HAND WITH SELECTED (IGNEOUS OR LIMESTONE, ETC.) STONE OF A SIZE IOO MM. TO 250 MM.

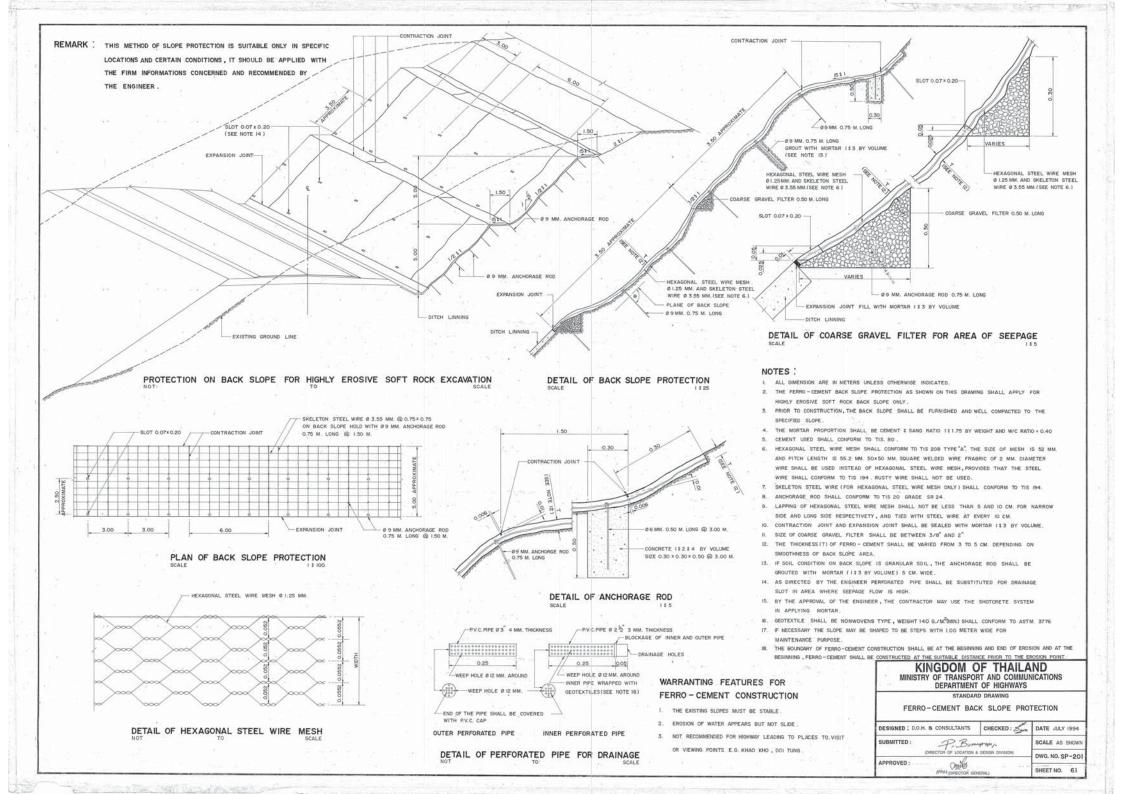
### REMARK :

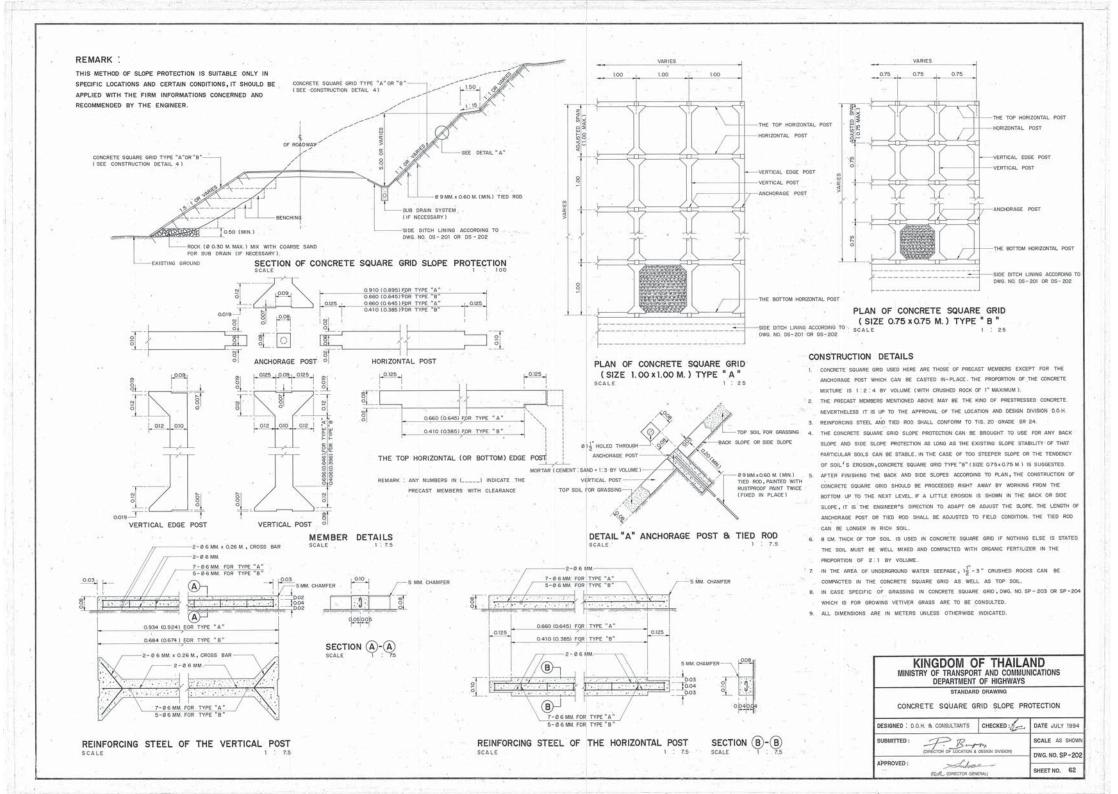
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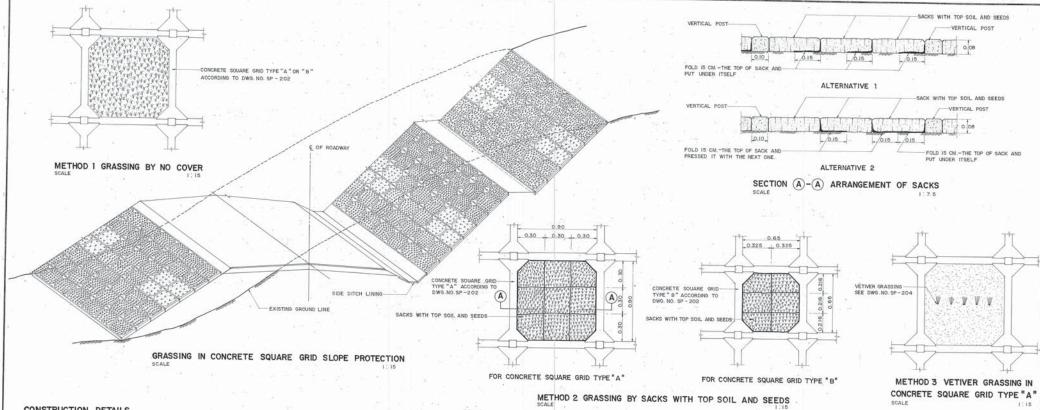
### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING
SPECIFICATION FOR GABIONS CONSTRUCTION

DESIGNED : D.O.H. & CONS	ULTANTS	CHECKED:	DATE JULY 1994
SUBMITTED:	SCALE, NONE		
(DIRECTOR C	F LOCATION	& DESIGN DIVISION)	DWG. NO. SP-108
APPROVED:	(Pent)		SHEET NO. 60







### CONSTRUCTION DETAILS

- PERIOD FOR GRASSING: AFTER THE CONSTRUCTION OF CONCRETE SQUARE GRID IS FINISHED, GRASSING SHOULD BE QUICKLY PROCEEDED ACCORDING TO CERTAIN GRASSING METHODS AND SUITABLE PERIOD WHICH IS THE TIME OF 1-2 WEEKS BEFORE RAINY SEASON.
- 2. TOP SOIL TOP SOIL IS COMPOSED OF RICH SOIL (NO STONES OR EXISTING BUILDING MATERIALS ) AND GOOD ORGANIC FERTILIZERS SUCH AS BKK. 2 (BANKOKC 2) ORGANIC FERTILIZER OR DRY OW MANURE . THE PROPORTION 13 2: 18 YOUNG WITH EXCORP MOISTURE FOR GRASS GROWNG.
- 3. GRASSING METHOD I (NO COVER) THERE ARE 3 WAYS SEEDING, NODING, BULBING, AS FOLLOW
- 3.1 SEEDING
  - 3.1.1 PUT 6-7 CM, THICK OF TOP SOIL IN THE CONCRETE SQUARE GRID AND WELL COMPACTED.
  - 3.1.2 SCATTER THE SELECTED SEEDS ACCORDING TO ITEM 6 INTO THE SQUARE GRID.
  - 3.1.3 FILL I-2 CM. OF TOP SOIL OVER THE SEEDS AND WELL COMPACTED IT, MAKING EQUAL THICK TO CONCRETE SQUARE GRID (8 CM.)
- 3.2 NODING
- 3.2.1 FOLLOW THE STEP 3.1.1 EXCEPT FOR THE THICKNESS OF THE TOP SOIL 8 CM.
- 3.2.2 USE THE WELL SELECTED AND PREPARED GRASS NODES ACCORDING TO STATED AMOUNTS IN ITEM 6. SCATTERED PLANTING THE NODES.
- 3.3 BULBING
  - 3.3.1 FOLLOW THE STEP 3.2.1
  - 3.3.2 USE THE WELL SELECTED AND PREPARED BULBS ACCORDING TO STATED AMOUNTS IN ITEM 6. SCATTERED PLANTING THE NODES.

THE CONTRACTOR MAY EXPLOIT EITHER WAY OUT OF THOSE THREE WAYS MENTIONED ACCORDING TO THE AREA AND CONVENIENCE OF FINDING THE GRASS. THE WAYS 3.1 AND 3.2 AND 3.3 CAN BE MIXED. IF SO. THE AMOUNTS OF SEEDS, NODES, AND BULBS SHOULD BE REDUCED UP TO THE SUITABILITY AS DIRECTED BY THE ENGINEER AND APPROVED BY LOCATION AND DESIGN DIVISION D.O.H.

- 4. GRASSING METHOD 2 (WITH SACKS OF TOP SOIL AND SEEDS); DO AS FOLLOWS.
- 4.1 PREPARE TOP SOIL AS IN ITEM 2.
- 4.2 GET CERTAIN SACKS AS FROM ITEM 5
- 4.3 MIX THE SELECTED SEEDS AS FROM ITEM 6 WITH TOP SOIL. THE SEEDS USED HERE MUST BE TWICE THE AMOUNTS OF STEPS 3.1, THE AMOUNT OF SOIL IS 17.5 KG./SACK AND IS.75 - KG./SACK FOR CONCRETE SQUARE GRID TYPE "A" AND "B" RESPECTIVELY.
- 4.4 ARRANGE THE SACKS OF TOP SOIL AND SEEDS IN THE CONCRETE SQUARE GRID BY LAYING THEM DOWN AND FOLDING THE SACKS AS SHOWN IN THE FIGURE, ADJUST THE SACKS AND MAKE THEM LOOK LIKE THE SHOWN FIGURE (THE APPROXIMATE THICKNESS OF 8 CM.) THE QUANTITY OF TOP SOIL IN THE SACKS CAN BE ADJUSTED SO THAT THE SACKS CAN WELL FIT THE GRID.

- 5. DETAIL OF SACKS : THE SACKS SHOULD BE AS FOLLOW
  - 5.1 SACKS MADE FROM NATURAL FIBRE: THE SACKS SHOULD HAVE THEIR NETS THAT ARE NOT TOO LOSSE OR TOO TIGHT, THEY SHOULD BE STRONG ENOUGH SO THAT THEY WILL NOT BE BROKEN WHILE BERING REMMED. ALSO THEY SHOULD DECAY IN THE COURSE OF ONE YEAR. THUS THE JUTE BAGS ARE INTRODUCED AS THEIR I INCH WOVEN THREADS CONTAIN 7.5 WARP STRINGS AND 12 WEFT STRINGS (\$\frac{1}{2}\$ O.5 STRING DEVIATION), THE SIZE OF SACK FOR CONCRETE SQUARE GRID TYPE "A" \$13 \$39 x 50 CM. (\$\frac{1}{2}\$ I CM. OEVIATION). AND THE SIZE FOR THE TYPE "B" IS 30.5 x 52 CM. (\$\frac{1}{2}\$ I CM. OEVIATION).
  - 5.2 SACKS MADE FROM SYNTHETIC FIBERS! THE SACKS MUST POSSES THE QUALITIES ACCORDING TO ITEM 5.1 AND MUST COMPLETELY DECAY WITHIN ONE YEAR. YET THE APPROVAL OF THE LOCATION AND DESIGN DIVISION D.O.H.
- 6. KINDS OF GRASS AND WAYS OF REPRODUCING
  - 6.1 GENERAL SPECIFICATION: USE ANY OF LOCAL KINDS OR ANY THAT CAN BE FOUND IN THAILAND AS THEY LIVE LONG AND CAN HOLD VERY WELL TO THE GROUND. THEY ALSO SURVIVE IN ANY BAD STATE OF SOIL AND ENVIRONMENT. INFORMATION IS AS FOLLOW;

	electron energy	A STATE OF THE STA		SEEDS "	QUALITY
NO.	NO. COMMON NAMES	SCIENTIFIC NAMES	WAYS OF REPRODUCING	PURITY NOT LESS THAN (%)	RATE OF GROWTH NOT LESS THAN (%)
1	หญ้ารูซี	BRACHIARIA RUZIZIENSIS	SEEDS	70	30
2	หญ้าหลืนคททูสัม	PASPALUM PLICATULUM	SEEDS	60	40
3	หญ้าแพรก	CYNODON DACTYLON	SEEDS OR NODES	40	30
4	หญ้าลดาร์	CYNODON PLECTOSTACHYUS	NODES OR BULBS		
5	หญ้าชิกแนลเลี้ยย	BRACHIARIA HUMIDICOLA	NODES OR BULBS		141.64
6	หญ้าบาเฮีย	PASPALUM NOTATUM	OFFSHOOTS OR SEEDS		
7	หญ้าดีนกา .	ELEUCINE INDICA	SEEDS	40	25
	พีชตระกูลถั่ว		2 22200		
8	ถั่วลาย	CENTROSEMA PUBESCENS	SEEDS	90	50
9	ถั่วอามาตัา	STYLOSANTHES HAMATA	SEEDS	70	25
10	กัวสีราโต	MACROPTILIUM ATROPURPUREUM	12.03.035	60	25
				1	

AND SEEDS MOISTURE IS NOT MORE THAN 10 %

6.2 SEEDING : QUALITY OF SEEDS SHOULD CONFORM TO ITEM 6.1 (10-15 GM / M2)

6.3 NODING ; USE RATHER OLD BUT STRONG STEMS WHICH HAVE AT LEAST 4 NODES EACH (100-120 STEMS/ $N_{\rm c}^2$ ) 6.4 BULBING : USE STRONG BULBS (120-140 BULBS/ $N_{\rm c}^2$ )

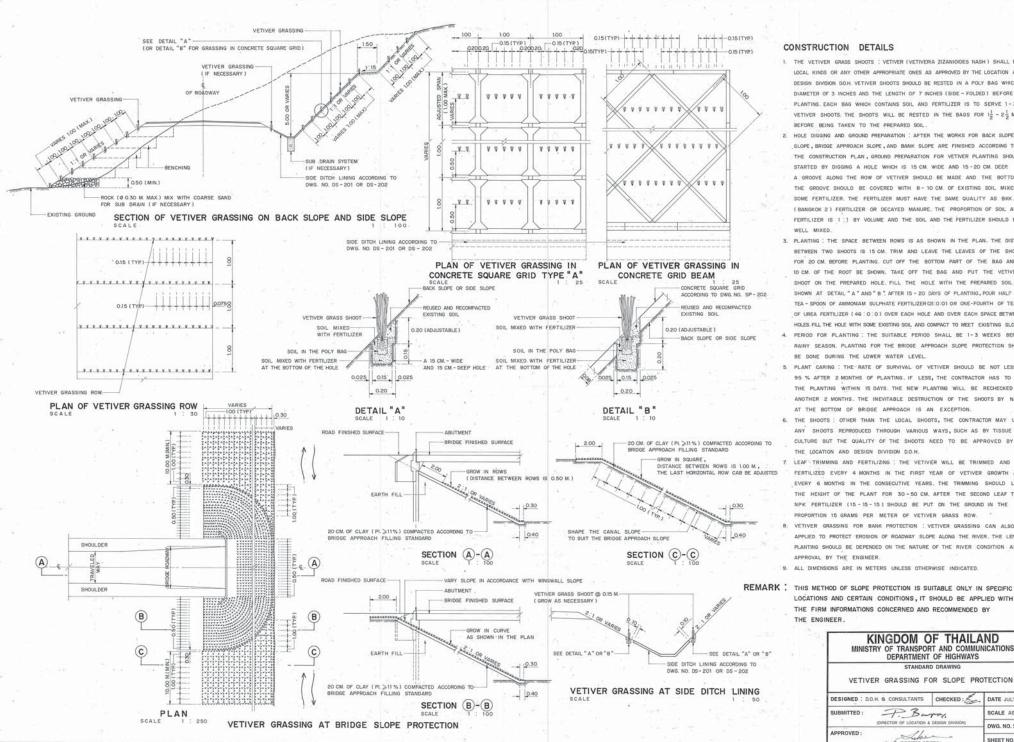
THE REPORT OF THE PROPERTY OF THE PARTY OF T

- THE CONTRACTOR CAN USE ONLY ONE KIND OF GRASS OR MANY DIFFERENT KINDS, BUT THE NUMBERS OF EACH KIND NEED TO BE ADJUSTED ACCORDING TO SUITABLE PROPORTION. GENERALLY, BEANS WITH THE RATE OF GROWTH 10-15 STEMS /M. ARE USED TO MIX WITH OTHER GRASS, ANY OTHER KINDS OF GRASS (NOT MENTIONED IN THE TABLE ABOVE) CAN ALSO BE USED AS DIRECTED BY THE ENGINEER AND APPROVED BY THE LOCATION AND DESIGN DIVISION D.O.H.
- 7. GRASS CARING : THE CONTRACTOR MUST TAKE GOOD CARE OF THE GRASS SO THAT THEY GROW WELL WITHIN 3 MONTHS, THERE SHOULD BE COVERED NOT LESS THAN 75 %, OF THE GRID AREA ANYWHERE, THE GROWTH PERCENTAGE LESS THAN THIS THE CONTRACTOR MUST MANAGE TO REACH THE PERCENT OTHERWISE. THE AGRICULTURAL OPERATION TO INCREASE GROWTH OF GRASS COULD BE DONE.
- 8. THIS DRAWING CAN BE USED FOR GRASSING IN CONCRETE GRID BEAM (ACCORDING TO DWG. NO. SP-301).
- 9. ALL DIMENSION ARE IN METERS UNLESS OTHERWISE INDICATED.

### REMARK :

THIS METHOD OF SLOPE PROTECTION IS SUITABLE ONLY IN SPECIFIC LOCATIONS AND CERTAIN CONDITIONS, IT SHOULD BE APPLIED WITH THE FIRM INFORMATIONS CONCERNED AND RECOMMENDED BY THE ENGINEER.

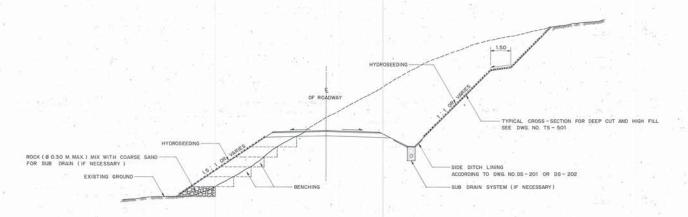
## KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS STANDARD DRAWING GRASSING IN CONCRETE SQUARE GRID AND GRID BEAM DESIGNED: D.O.H. & CONSULTANTS CHECKED: DATE JULY 1994 SUBMITTED: Bugging SCALE AS SHOWN APPROVED: DOWN ON SP-203 SHEET NO. 63



- 1. THE VETIVER GRASS SHOOTS : VETIVER (VETIVERIA ZIZANIOIDES NASH.) SHALL BE LOCAL KINDS OR ANY OTHER APPROPRIATE ONES AS APPROVED BY THE LOCATION AND DESIGN DIVISION DO.H. VETIVER SHOOTS SHOULD BE RESTED IN A POLY BAG WHICH HAS DIAMETER OF 3 INCHES AND THE LENGTH OF 7 INCHES (SIDE - FOLDED) BEFORE PLANTING, EACH BAG WHICH CONTAINS SOIL AND FERTILIZER IS TO SERVE 1-2 VETIVER SHOOTS. THE SHOOTS WILL BE RESTED IN THE BAGS FOR  $1\frac{1}{2} - 2\frac{1}{2}$  MONTHS
- HOLE DIGGING AND GROUND PREPARATION : AFTER THE WORKS FOR BACK SLOPE, SIDE SLOPE, BRIDGE APPROACH SLOPE, AND BANK SLOPE ARE FINISHED ACCORDING THE CONSTRUCTION PLAN, GROUND PREPARATION FOR VETIVER PLANTING SHOULD BE STARTED BY DIGGING A HOLE WHICH IS 15 CM. WIDE AND 15-20 CM. DEEP A GROOVE ALONG THE ROW OF VETIVER SHOULD BE MADE AND THE BOTTOM OF SOME FERTILIZER. THE FERTILIZER MUST HAVE THE SAME QUALITY AS BKK. 2 (BANGKOK 2) FERTILIZER OR DECAYED MANURE. THE PROPORTION OF SOIL AND FERTILIZER IS 1:1 BY VOLUME AND THE SOIL AND THE FERTILIZER SHOULD BE
- 3. PLANTING : THE SPACE BETWEEN ROWS IS AS SHOWN IN THE PLAN, THE DISTANCE TEA - SPOON OF AMMONIAM SULPHATE FERTILIZER(21:0:0) OR ONE-FOURTH OF TEA-SPOO OF UREA FERTILIZER ( 46 : 0 : 0 ) OVER EACH HOLE AND OVER EACH SPACE BETWEEN HOLES FILL THE HOLE WITH SOME EXISTING SOIL AND COMPACT TO MEET EXISTING SLOPE
- PERIOD FOR PLANTING : THE SUITABLE PERIOD SHALL BE 1-3 WEEKS BEFORE
- PLANT CARING : THE RATE OF SURVIVAL OF VETIVER SHOULD BE NOT LESS THAN AFTER 2 MONTHS OF PLANTING, IF LESS, THE CONTRACTOR HAS
- SHOOTS REPRODUCED THROUGH VARIOUS WAYS, SUCH AS BY TISSUE CULTURE BUT THE QUALITY OF THE SHOOTS NEED TO BE APPROVED BY
- LEAF TRIMMING AND FERTILIZING ; THE VETIVER WILL BE TRIMMED AND FERTILIZED EVERY 4 MONTHS IN THE FIRST YEAR OF VETIVER GROWTH AND HEIGHT OF THE PLANT FOR 30 - 50 CM. AFTER THE SECOND LEAF TRIMMIN
- VETIVER GRASSING FOR BANK PROTECTION : VETIVER GRASSING CAN ALSO BE APPLIED TO PROTECT EROSION OF ROADWAY SLOPE ALONG THE RIVER. THE LENGTH O PLANTING SHOULD BE DEPENDED ON THE NATURE OF THE RIVER CONDITION AND
- 9. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.

LOCATIONS AND CERTAIN CONDITIONS, IT SHOULD BE APPLIED WITH THE FIRM INFORMATIONS CONCERNED AND RECOMMENDED BY

### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS STANDARD DRAWING VETIVER GRASSING FOR SLOPE PROTECTION DESIGNED : DO.H. & CONSULTANTS CHECKED : DATE JULY 1994 7. Barry SCALE AS SHOW! DWG. NO. SP-204 SHEET NO. 64



### HYDROSEEDING FOR SLOPE PROTECTION

### CONSTRUCTION DETAILS

- 1. AREA PREPARATION
- 1.1 AFTER THE EARTH WORKS OF CUT AND FILL ARE FINISHED AS PLANNED, HYDROSEEDING SHOULD BE DONE RIGHT AWAY BEFORE THE OCCURRENCE OF SLOPE EROSION.
- 1.2 IF THE SURFACE OF BACK SLOPE IS TOO HARD, SHAPE IT BY STRIPPING SOME EXISTING SOIL THAT IS COVERING THE SURFACE. THE SHAPING WILL MAKE THE SURFACE SOFT AND SHOULD LEAVE THE 2.5 CM. THICKNESS OF THE SURFACE. THERE SHOULD BE GROOVED WHICH ARE 5 7 CM. APART ALONG THE LENGTH OF AREA.
- 1.3 SHAPING THE SURFACE OF THE SIDE SLOPE IS DONE THE SAME WAY AS IN ITEM 1.2 AND YET AS DIRECTED BY THE ENGINEER.
- 14 IN SOME AREAS THAT ARE COVERED WITH BOULDERS, SOFT ROCK OR ROCK WITH SEEPAGE FLOW OR HAVE RELL'S OR GULL'ES, THE SOIL SHAPING SHOULD BE DIRECTED BY THE ENGINEER.
- 2. SUGGESTED GRASS AND QUALITIES OF ITS SEEDS
  - 2.1 NI GENERAL, THE SUGGESTED GRASS IS THAT OF LOCAL KINDS OR ANY KINDS THAT HAVE BEEN WELL GROWN IN THAILAND. THE GRASS USED AS PERMANENT GRASS FOR PREVENTION SOIL EROSION SHOULD BE PERENNIAL GRASS AND HAS STRONG ROOTS SYSTEM THAT CAN FIRMLY HOLD TO THE GROUND AND CAN BEAR ANY BAD STATE OF SOIL. WHILE IN USE, THE GRASS SHOULD BE MIXED WITH BEAR PLANTS WHICH WILL BE A NITROGEN CATCHER OR MIXED WITH SHORT LIFE GRASS WHICH WILL BE AS STRONG ON CATCHER OR MIXED WITH SHORT LIFE GRASS WHICH WILL BE SACRIFY GRASS, AS DIRECTED BY THE ENGINEER.
  - 2.2 SUGGESTED GRASS AND QUALITY OF ITS SEEDS

				SEEDS ' QUALITY		3 10	
NO.	O COMMON NAMES	OMMON NAMES SCIENTIFIC NAMES	MOISTURE NOT MORE THAN (%)	PURITY NOT LESS THAN (%)	RATE OF GROWTH NOT LESS THAN (%)	NOTE	
1	หญ้ารูชี่	BRACHIARIA RUZIZIENSIS	10	70	50		
2	หญ้าหลิแคททูลั่ม	PASPALUM PLICATULUM	10	60	40		
3	หญ้าแพรก	CYNODON DACTYLON	.10	50	30		
4	หญ้าชิกแนลเลื้อย	BRACHIARIA HUMIDICOLA	10	. /2	-		
5	หญ้าลตาร์	CYNODON PLECTOSTACHYUS SPP.	10	-	-	6 0	
6	หญ้าบาเอีย	PASPALUM NOTATUM	10	50	30		
7	หญ้าหวาย	ERAGROSTIS DIPLACHNOIDES	.10	50	30		
8	หญ้า WEEPING	ERAGROSTIS PARVIFLORA	10	50	30		
	LOVE GRASS	ERAGROSTIS CURVULA			5 57	100	
9	หญ้าดีนกา -	ELEUCINE INDICA	10	40	2.5	70	
10	หญ้าสระ	LOLIUM PERENNE	10	50	30		
n;	หญ้าFESCUE	FESTUCA ELATOR	10	50	30	GOOD FOR COLD	WEATHE
12	หญ้าACACIA	ACACIA SPP.	10	50	30		
	พีชตระกูลถัง	10.0	10.0				
13	ถึงลาย	CENTROSEMA PUBESCENS	10	90	50		
14	ลัวลามาต้า	STYLOSANTHES HAMATA	10	70 -	30		
15	ถั่วชิงาโดเร	MACROPTILIUM ATROPURPUREUM	12	90	50		

ANY OTHER KINDS THAT HAVE THE SAME QUALITIES AS MENTIONED IN ITEM 2.1 CAN ALSO BE USED, YET AS DIRECTED BY THE ENGINEER AND APPROVAL BY THE LOCATION AND DESIGN DIVISION D.O.H.

2.3 TWO KINDS OF GRASS OR MORE AS MENTIONED IN ITEM 2.2 SHOULD BE USED TOGETHER ACCORDING TO THE SITE, THE PROPORTION OF THE GRASS MIXED SHOULD BE 15 GM./M<sup>2</sup> AND BEANS WITH RATE OF GROWTH 10 - 15 STEMS/M<sup>2</sup>

### 3. OTHER MATERIALS

- 3.3 ACID DILLITE: IN GENERAL, THE pH OF SOIL SUITABLE FOR GRASSING IS ABOUT 5-8. IF THE SOIL HAS CONCENTRATED ACID, USE THESE CHEMICALS TO DILITE THE ACID CONDITION OF THE SOIL.
- 3.1.1 CRUSHED AND BURNED OYSTER SHELL OR
- 3.1.2 DOLOMITE

THE SUITABLE PROPORTION OF THESE CHEMICALS IS USED TO DILUTE THE ACID CONDITION OF SOIL, TO BE NEUTRAL CONDITION.

- 3.2 NPK FERTILIZER (15:15:15): NOT LESS THAN 30 GM./M.2
- 3.3 TACKIFIER OR BINDER MATERIALS (TAKEN FROM PLANTS); WHITE GLUE OR EMULSION —
  PRODUCED FROM PETROLEUM SHOULD NOT BE USED BECAUSE IT HINDERS THE GROWTH OF
  THE SEEDS. THE TACKIFIER USED SHOULD BE 30 GM./M.<sup>2</sup>.
- 3.4 WETTING AGENT : USED TO PREVENT THE CLINGING AMONG THE SEEDS, WITH SUITABLE PROPORTION.
- 3.5 INDUSTRIAL DETERGENT : USED TO INCREASE  $\kappa$  VALUE ( PERMEABILITY ) OF SQLL . THE APPROPRIATE AMOUNT IS 0.1 0.2 GM./  $M^2_{\odot}$
- 3.6 MULCH MATERIALS : NOT LESS THAN 200 GM./M.2
- 4. PUMPING MACHINE

THE PREFERABLE KIND IS THAT OF HIGH PRESSURE MACHINE THAT CAN FORCE THE MIXTURES AS HIGH AS 30 M AND THAT HAS ITS HEAD THAT CAN SPRAY THE MIXTURES ALL OVER THE PLACE.

5. PERIOD OF TIME

HYDROSEEDING SHOULD BE DONE 2-4 WEEKS BEFORE RAINY SEASON.

6. FERTILIZING

10 - 15 GM/M2 OF UREA FERTILIZER SHOULD BE SPRAYED ON THE AREA ONE MONTH AFTER HYDROSEEDING.

- 75 % OF GRASS SHOULD GROW AND COVER THE GROUND 2-3 MONTHS AFTER HYDROSEEDING, EXCEPT HARD SOIL OR ROCK OR BOULDERS.
- 8. INVESTIGATION : IF THERE ARE STILL SOME AREAS THAT ARE NOT SUCCESSFULLY DONE ACCORDING TO ITEM 7, THE CONTRACTOR NEEDS TO REDO THE HYDROSEEDING.
- 9. ANY OTHER OPERATION CONFORMING TO AGRICULTURAL, TO INCREASE EFFICIENCY OF GROWTH,
  CAN BE DONE.

### REMARK :

THIS METHOD OF SLOPE PROTECTION IS SUITABLE ONLY IN SPECIFIC LOCATIONS AND CERTAIN CONDITIONS, IT SHOULD BE APPLIED WITH THE FIRM INFORMATIONS CONCERNED AND RECOMMENDED BY THE ENGINEER.

### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

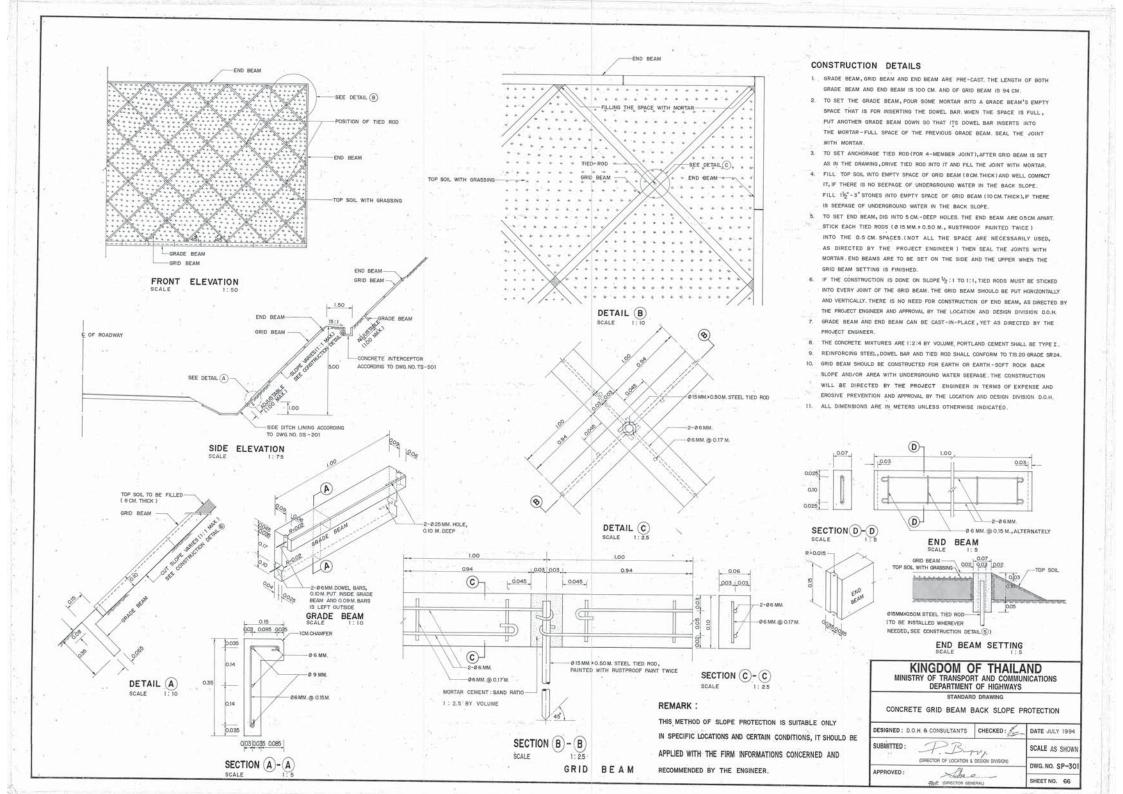
HYDROSEEDING FOR SLOPE PROTECTION

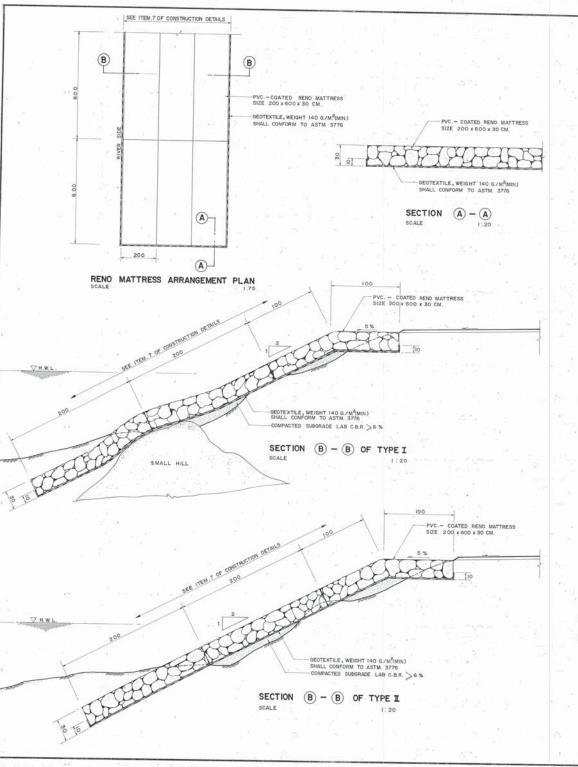
DESIGNED : DO.H. B CONSULTANTS CHECKED: DATE JULY 1994

SUBMITTED: SCALE NONE

(INDECTOR OF LOCATION & DESIGN DIVISION)

APPROVED: Share C





### RENO OR WIRE MATTRESS

PVC. - COATED RENO MATTRESS ; OR GALVANIZED WIRES THAT IS COATED WITH PVC. DETAILS ARE AS FOLLOWS :

### I. NETTING

THE GALVANIZED WIRES COATED WITH PVC. ARE MADE INTO 6x8 CM.( ± 10%) HEXAGONAL, NETTING JOINED TOGETHER BY TWISTING THE WIRES ROUND EACH OTHER TWICE AS SHOWN IN THE PICTURE



### 2. WIRES

THE WIRE MUST BE ABLE TO RESIST THE TENSION OF 28.5 -51 KG. / MM. (ACCORDING TO BS 1052 / 1980 STANDARD) THE DIAMETER OF WIRE FOR FRAME IS AT LEAST 2.7 MM. THE DIAMETER OF WIRE FOR NETTING IS AT LEAST 2:0 MM. THE DIAMETER OF WIRE FOR BOX WRAPPING IS AT LEAST 2.0 MM. AND THE WIRE IS LONG ENOUGH TO BE USED IN INSTALLATION

### 3. GALVANIZING

THE WIRES MUST BE GALVANIZED ACCORDING TO BS 443/1982 STANDARD. THE MINIMUM OF ZINC'S WEIGHT USED IN ACCORDANCE WITH THE DIAMETER

WIRE'S DIAMETER (MM.) WEIGHT OF ZINC COATING (GM./M.) 2.0 240

2.7

( ABOUT 5 % OF RENO MATRESS' WEIGHT ) .

### 4. PVC .- COATED

AFTER HAVING GALVANIZED, THE WIRE MUST ALSO BE COATED WITH BLACK OR GREY PAINT PVC. (POLY VINYL CHLORIDE), THE AVERAGE THICKNESS OF PVC. COATING IS 0.55 MM. (AND NOT LESS THAN 0.40 MM.) THE QUALITIES OF PVC.

260

THE BONDING QUALITY CAN BE TESTED BY DIPPING PVC .- COATED WIRE INTO DISSOLVED POTASSIUM PERMANGANATE (1%) FOR 50 HOURS CONTINUOUSLY AT THE ROOM TEMPERATURE. THE DISSOLVED POTASSIUM PERMANGANATE WILL ERODE , INTO THE WIRE'S SURFACE AND PVC., NOT MORE THAN 15 MM. WHICH EXAMINATION BY CUTTING THE END OF WIRE.

### B. EROSION DURABILITY OF PVC.

AFTER THE PVC -- COATED WIRE (NOT THE TWO END PARTS) HAS BEEN PUT INTO THE CONCENTRATED SOLUTION OF SODIUM CHLORIDE FOR 60 HOURS CONTINUOUSLY AT THE ROOM TEMPERATURE, THE PVC'S TOTAL WEIGHT MUST STILL REMAIN

### C. EROSION DURABILITY OF END PART'S WIRE

AFTER THE PVC -- COATED WIRE HAS BEEN PUT INTO THE 50 % SOLUTION OF HYDROCHLORIC ACID FOR 50 HOURS CONTINUOUSLY AT THE ROOM TEMPERATURE, ITS END PART SHOULD NOT BE ERCOE MORE THAN 20 MM.

### D. TEMPERATURE DURABILITY

AFTER THE PVC - COATED WIRE HAS BEEN LEFT IN 100°C FOR 100 HOURS CONTINUOUSLY, IT QUALITIES SHOULD NOT CHANGE WHEN THE WIRE BE BROUGHT BACK TO TEST IN ACCORDING TO ITEM A-C.

### 5. DEVIATION

THE DEVIATION OF THE WRE'S DIAMETER IS LESS THAN ± 2.5 %

### 6. ROCKS

THE ROCKS USED FOR THIS WORK MUST BE HARD ENOUGH AND NOT CRUMBLE WHEN WET AND ENDURE REGARDLESS OF THE CLIMATE. THEY ARE GRANITE, LIME STONE , SAND STONE , GRAVEL AND CRACKED CONCRETE. THE AVERAGE SIZE OF THE ROCKS SHOULD BE 6-10 CM. OR  $\pm (5\% - 7\%)$ ,

### CONSTRUCTION DETAILS OF RENO MATTRESS

- I. CLEAR THE AREA OF RENO MATTRESS CONSTRUCTION AND MARK THE SHAPE OF SIDE SLOPE AS SHOWN IN THE DRAWING AS WELL AS WELL COMPACT THE GROUND . IF THERE IS A SMALL HILL IN THE AREA WHICH MAKES IT IMPOSSIBLE TO REGULARLY MAKE THE SIDE SLOPE , THE RENO MATTRESS ON THE SMALL HILL SHALL BE CONSTRUCTED AS SHOWN IN SECTION (B)-(B)
- 2. LAY THE GEOTEXTILE ON THE SIDE SLOPE BEFORE CONSTRUCTION THE RENO MATTRESS
- SHAPE THE GROUND TO EXISTING CONDITION AFTER THE RENO MATTRESS CONSTRUCTION IS FINISHED.
- GENERAL DETAILS OF PVC .- COATED RENO MATTRESS ARE AS STATED IN THE TOPIC PVC - COATED RENO MATTRESS .
- 5. THE PROJECT ENGINEER WILL ADJUST THE SUITABLE AREA IN SITE FOR RENO MATTRESS CONSTRUCTION .
- 6. THE GEOTEXTILE USED IN THIS CONSTRUCTION IS MADE FROM NONWOVENS TYPE. THE INSTRUCTION ON HOW TO USE ARE GIVEN BY THE PRODUCERS OF THE PRODUCTS AND ARE APPROVED BY THE ENGINEER
- 7. THE PART OF RENO MATTRESS STICKING INTO STREAM CAN BE NECESSARILY ADJUSTED SO AS TO SUIT THE AREA
- B. ALL DIMENSIONS ARE IN CENTIMETERS UNLESS OTHERWISE - INDICATED

### REMARK :

THIS METHOD OF SLOPE PROTECTION IS SUITABLE ONLY IN SPECIFIC LOCATIONS AND CERTAIN CONDITIONS, IT SHOULD BE APPLIED WITH THE FIRM INFORMATIONS CONCERNED AND RECOMMENDED BY THE ENGINEER.

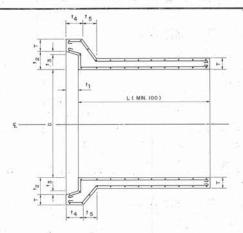
### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS

DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

RENO MATTRESS SLOPE PROTECTION

DESIGNED : D.O.H. & CONSULTANTS CHECKED : DATE JULY 1994 Burn SUBMITTED SCALE AS SHOWN IDIRECTOR OF LOCATION & DESIGN DIVI DWG. NO.SP-401 APPROVED : SHEET NO. 67

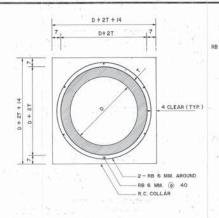


A BELL AND SPIGOT TYPE

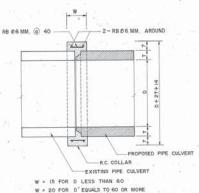
L (MIN. 100 )

B TONGUE AND GROOVE TYPE

DETAIL OF R.C. PIPE CULVERT



A TRANSVERSE SECTION



B LONGITUDINAL SECTION



### TABLE 1

C. PIPE CULVERT	INSIDE DIAMETER ( D ) ( C M.)	WALL THICKNESS (T) (CM.)	MIN, CI REINFOR		CRUSHING LOAD TO PRODUCE 0.03 CM, CRACK WIDTH AND 30 CM, CRACK LENGTH	MAXIMUM CRUSHING LOAD ( KG./M.)	ULTIMATE COMPRESSIVE STRENGTH FOR 0 IS x 30 CM. CONCRETE CYLINDER AT 28 DAYS AGE	OVER FILL ON R.C. PIPE CULVERT NOT MORE THAN (METERS)	
	Com.	10.11.7	INNER CAGE OUTER CAG		( KG./M.)	grand in	( KG. / CM. )		
700	30	5.0	1,5	- 2	3,060	4,590	1	7	
	40	6.0	2.5	-	4,080	6,120	100 (201	6	
	50	7.0	3.8	-	5,100	7,650	P		
	60	7.5	5.7	-	6,120	9,180	280 (350)	10.0	
2	80	9.5	5.8	4.1	8,160	12,240			
	100	11.0	7.0	5.2	10,200	15,300			
	120	12-5	8.9	6.8	12,240	18,360			
	150	15.0	12.5	9.5	15,300	22,950	350 (440)	11.0	
. ,	30	5.0	1.5	-	1,990	3,060	7	7	
	40	6.0	1.5	1	2,650	4,080		l I	
	50	7.0	1.5	-	3,320	5,100			
3	60	7.5	1.5	5 1	3,980	6,120	280 (350)	8.0	
	80	9.5	4.0		5,300 8,160		200,3007	1	
	100	11.0	4.2	3.2	6,630	10,200		100	
	120	12.5	5.1	3.8	7,960	12,240	7 7	19.1	
	150	15.0	7.2	5.5	9,950	15,300			

FIGURES IN PARENTHESES ARE ULTIMATE COMPRESSIVE STRENGTH FOR 15 x 15 x 15 CM. CONCRETE CUBE AT, 28 DAYS AGE

### TABLE 2

	INSIDE				PI	PE EN	DETAIL	s (CM	(.)		
R.C. PIPE CULVERT CLASS	DIAMETER (D)	THICKNESS (T)		BELL	a spigo	TYP!	E	TONG	UE & G	ROOVE 1	YPE
-0-0-0-0-0	(CM.)	(CM.)	1	†2	†3	14	(MIN)	q	b.	c	đ
	30	5.0	6.0	6.6	0.4	8,5	15.0	1.9	8.0	2.3	3.0
	40	6.0	6.7	7.6	0.4	9.7	18.0	2.3	1.0	2.7	3.0
	50	7.0	7.0	8.6	0.4	10.5	21.0	2.8	1,0	3.2	4.0
	60	7.5	7.6	9.1	0.4	11.4	22.5	2.8	1.5	3.2	4.0
283	80	9.5	8.9	0.51	0.4	13.7	28.5	3.8	1.5	4.2	4.5
	100	11.0	9.5	12.6	0.4	15.0	33.0	4.3	2.0	4.7	4.5
	120	12.5	10.0	14.1	0.4	16.5	37.5	4.8	2.5	5.2	5.0
1	150	15.0	10.0	16.6	0.4	17.7	45.0	5.7	3.0	6.3	6.0

A TONGUE AND GROOVE TYPE

B BELL AND SPIGOT TYPE

### PIPE CONNECTION DETAILS

INTERIOR JOINT SPACE -SHALL BE MORTARED FLUSH ON LOWER HALF OF PIPES ( CEMENT AND SAND RATIO 1:2 BY VOLUME )

MORTAR 1:2 BY VOLUME-

TO

SCALE

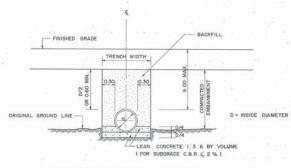
- I. ALL DIMENSIONS ARE IN CENTIMETERS UNLESS OTHERWISE INDICATED
- 2. REINFORCED CONCRETE PIPE CULVERT CLASSES 2 AND 3 SHALL CONFORM TO TIS 128.
- 3. CEMENT, STEEL REINFORCEMENT, AGGREGATES AND TEST METHODS USED FOR R.C. PIPE CULVERT SHALL CONFORM TO THE REQUIREMENT OF TIS. 128 OR TO THE DEPARTMENT OF HIGHWAYS STANDARDS
  - 3.1 CEMENT CONTENT USED FOR CONCRETE MIX SHALL NOT BE LESS THAN 335 KILOGRAM PER CUBIC METER OF CONCRETE .
  - 3.2 CONCRETE COVER FOR SINGLE LAYER CIRCULAR REINFORCEMENT SHALL BE 0.35 TO 0.5 TIME OF WALL THICKNESS (MEASURED FROM INNER WALL)
  - 3.3 CONCRETE COVER FOR DOUBLE LAYERS CIRCULAR REINFORCEMENT SHALL BE 2.5 CM. IN AVERAGE BUT NOT LESS THAN 1.5 CM.
  - 3.4 LONGITUDINAL REINFORCEMENT SPACING FOR PIPE SIZE Ø 50 CM. OR SMALLER SHALL BE A MINIMUM OF 4-04 MM. BARS OR "8-Ø 4 MM. BARS FOR PIPE Ø 60 CM. OR LARGER
  - .3.5 CIRCULAR REINFORCEMENT SPACING FOR PIPE SIZE @ 30 CM. TO Ø 80 CM. SHALL BE 10 CM. OR LESS AND FOR PIPE SIZE Ø 100 CM, TO Ø 150 CM, SHALL BE 15 CM, OR LESS BUT NOT MORE THAN THEIR WALL THICKNESS.
- 4. THE CULVERT WHICH HAVING TRANSVERSE REINFORCEMENT IN ELLIPTICAL CAGE AS SPECIFIED IN THE TIS. 128 SHALL NOT BE USED.
- PIPE MAY BE EITHER BELL AND SPIGOT TYPE OR TONGUE AND GROOVE TYPE AS DIRECTED BY THE ENGINEER
- 6. CULVERT JOINTS SHALL BE MORTARED AS SHOWN ON THE DRAWING WITH CEMENT MORTAR (1:2 BY VOLUME).
- 7. CULVERT LENGTH (L) SHALL BE 100 CM. UNLESS OTHERWISE SPECIFIED.
- 8. REINFORCED CONCRETE PIPE CULVERT CLASS 2 SHALL BE USED UNDER PAVEMENT
- 9. REINFORCED CONCRETE PIPE CULVERT CLASS 3 SHALL BE USED UNDER SIDEWALK

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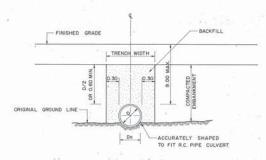
STANDARD DRAWING

R.C. PIPE CULVERT

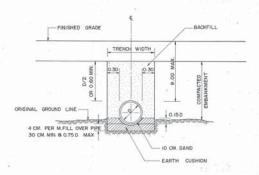
DESIGNED: D.O.H. & CONSULT	ANTS CHECKED:	DATE JULY 1994		
SUBMITTED:	oft.			
(DIRECTOR OF L	OCATION & DESIGN DIVISIONS	DWG. NO. DS-101		
APPROVED:	COTOR GENERAL)	SHEET NO. 68		



### (d) CONCRETE CRADLE BEDDING

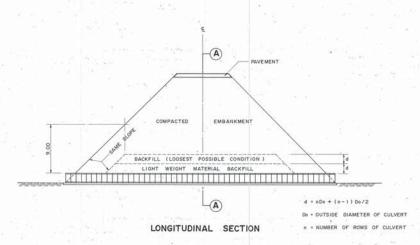


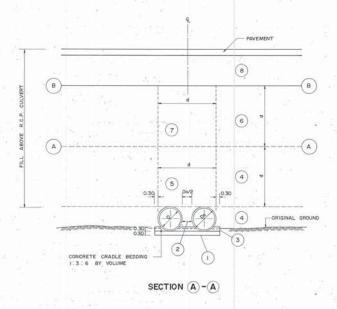
### (b) ORDINARY BEDDING



### (c) BEDDING FOR ROCK OR UNYIELDING FOUNDATION

R.C. PIPE CULVERTS INSTALLATION BY TRENCH METHOD TO SCALE





R.C. PIPE CULVERTS INSTALLATION FILL ABOVE PIPE OVER 9.00 M.

### NOTES:

REINFORCED CONCRETE CULVERT PIPE INSTALLATION (MAX. FILL HEIGHT 9.00 M.)

- A.) CONSTRUCTION METHODS WHEN FILL HEIGHT IS LESS THAN 1.20 M.
- I. THE PROJECTION METHODS SHALL BE USED FOR PIPE INSTALLATION
- THE PIPE LINE LAYOUT SHALL BE SUITABLE FOR THE TERRAIN, THE EXISTING GROUND ALONG THE LINE OF CULVERT SHALL BE PREPARED TO THE SPECIFIED SLOPE.
- PIPE BEDDING SHALL BE TYPE (a), (b) OR (c) AS SHOWN ON THE DRAWING AND SHALL DEPEND UPON PREVAILING SOIL CONDITION AND THE JUDGEMENT OF THE ENGINEER.
- AFTER THE PIPE HAS BEEN PLACED, THE SUBGRADE AND/OR PORTION OF PAVEMENT SECTION WHICH ARE TO BE LAID ALONG BOTH SIDES OF THE LINE WITH IN A DISTANCE OF 4 PIPE DIAMETERS BUT NOT LESS THAN 5.00 M. FROM CENTERLINE OF THE PIPE SHALL BE CONSTRUCTED LIGHT WEIGHT CONSTRUCTION EQUIPMENT USED FOR COMPACTION SHALL OPERATE IN A DIRECTION PERPENDICULAR TO CENTERLINE OF ROADWAY (OR PARALLEL TO PIPE LINE UNTIL THE BACKFILL HAS REACHED AN ELEVATION OF AT LEAST 30 CM. ABOVE THE TOP OF PIPE.
- 8.) CONSTRUCTION METHODS WHEN FILL HEIGHT EXCEEDS 1.20 M.
- I. PIPE INSTALLATION SHALL BE BY TRENCH METHOD. SUBGRADE SHALL BE FIRST CONSTRUCTED TO AN ELEVATION D/Z OR AT LEAST 60 CM OVER TOP OF PROPOSED PIPE A TRENCH SHALL THEN BE EXCAVATED ALONG THE PROPOSED LINE AS SHOWN ON THE DRAWING. TRENCH WALLS SHALL HAVE A SMOOTH SURFACE AND SHALL BE CONSTRUCTED VERTICALLY.
- 2. THE TRENCH BED SHALL BE PREPARED TO THE SPECIFIED SLOPE, BEDDING TYPE SHALL BE (a),(b) OR (c) DEPENDING ON FOUNDATION SOIL AND AS DIRECTED BY THE ENGINEER.
- PIPE SHALL BE INSTALLED ACCORDING TO SIZES SHOWN ON THE DRAWINGS. BACKFILLING
  OF PIPE CULVERTS SHALL NOT BE PERMITTED UNTIL AT LEAST 48 HOURS HAVE
  ELAPSED AFTER JOINT HAVE BEEN COMPLETED.
- 4. BACKFILL SHALL BE PLACED TO THE SUBGRADE ELEVATION AS DESCRIBED IN NOTE I. BACKFILL SHALL BE A SELECT MATERIAL AND SHALL REQUIRE THE APPROVAL OF THE ENGINEER METHOD OF COMPACTION OF BACKFILL SHALL BE THE SAME AS REQUIRED FOR SUBGRADE, COMPACTION EQUIPMENT SHALL BE APPROVED BY THE ENGINEER.

### REINFORCED CONCRETE CULVERT PIPE INSTALLATION (FILL HEIGHT OVER 9.00 M.)

- 1. AFTER EXISTING GROUND HAS BEEN PREPARED TO SPECIFIED SLOPE AND ELEVATION, CONCRETE BEDDING FOR PIPE SUPPORT SHALL BE PLACED AS SHOWN IN SECTION (A) (A)
- 2. THE NUMBER OF PIPE BARRELS AND SIZE OF PIPE CULVERT SHALL BE INSTALLED AS SHOWN
- 3. A CONCRETE CRADLE SHALL THEN BE PLACED AS SHOWN IN SECTION (A) (A)
- 4. EMBANKMENT SHALL BE CONSTRUCTED WITH SELECTED MATERIAL AND COMPACTED IN ACCORDANCE WITH THE TYPICAL CROSS-SECTION FOR THE ROUTE, EMBANKMENT SHALL BE COMPACTED TO THE (A-(A) LEVEL," d" METERS ABOVE THE TOP OF PIPE WITH LIGHT WEIGHT COMPACTION EQUIPMENT, CONSTRUCTION PROCEDURES SHALL REQUIRE SUPERVISION BY THE ENGINEER.
- A TRENCH SHALL BE EXCAVATED TO A WIDTH "d" METERS WITH VERTICAL, SMOOTH WALL AND BACKFILL, WITH LIGHT WEIGHT MATERIAL OF A TYPE SPECIFIED BY THE ENGINEER.
- 6. A) WHEN FILL HEIGHT IS LESS THAN IB.OO M. THE CONSTRUCTION HAS REACHED THE (A)-(A) LEVEL AS DESCRIBED IN NOTE 4 8 5 ABOVE, FULL EMBANKMENT SHALL THEN BE PLACED TO MEET THE FINISHED SUBGRADE LEVEL.
- B) WHEN FILL HEIGHT EXCEEDS IB.OO M, EMBANKMENT CONSTRUCTION SHALL CONTINUE AS DESCRIBED IN NOTE 4 ABOVE UNTIL THE (B)-(B) LEVEL IS REACHED.
- A TRENCH SHALL THEN BE EXCAVATED AS DESCRIBED IN NOTE 5 ABOVE EXCEPT BACKFILL
  SHALL BE REGULAR BACKFILL MAYERIAL AND SHALL BE PLACED AS LOOSELY AS POSSIBLE
  WITHOUT COMPACTION.
- THE REMAINING EMBANKMENT SHALL THEN BE CONSTRUCTED TO SUBGRADE LEVEL AND COMPACTED BY METHODS NORMALLY USED.
- 9. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED

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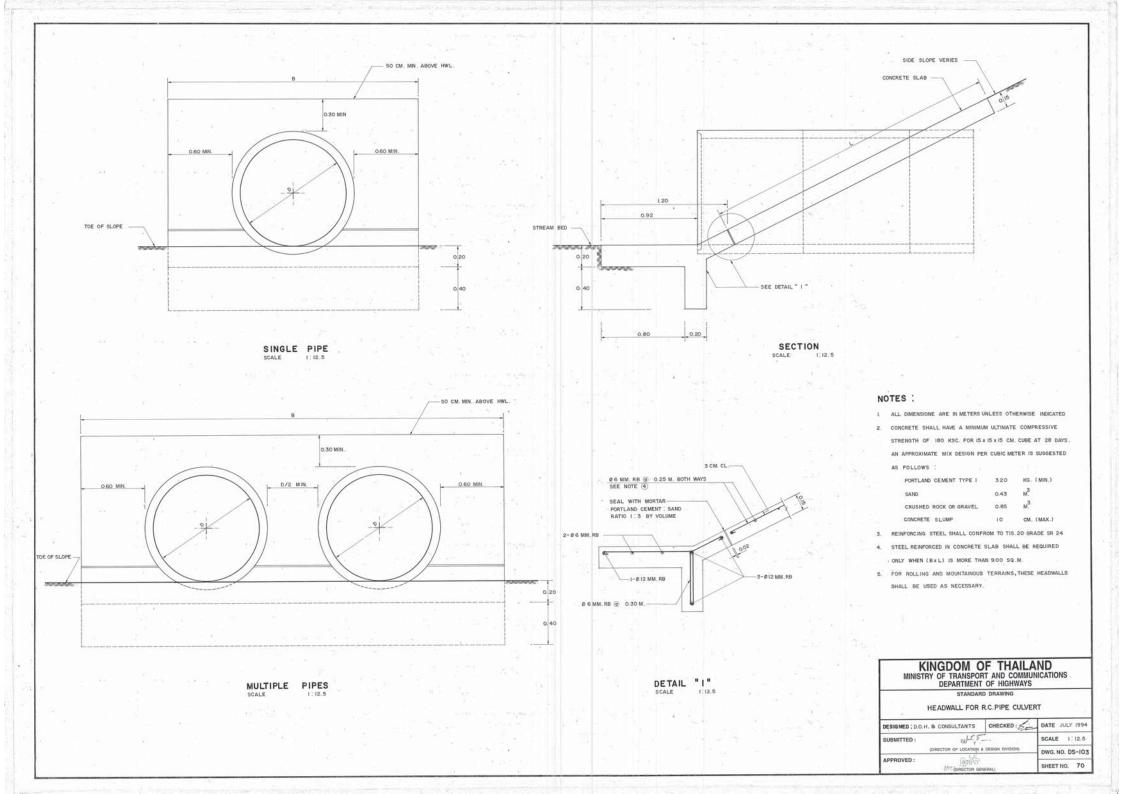
R.C. PIPE CULVERT INSTALLATION

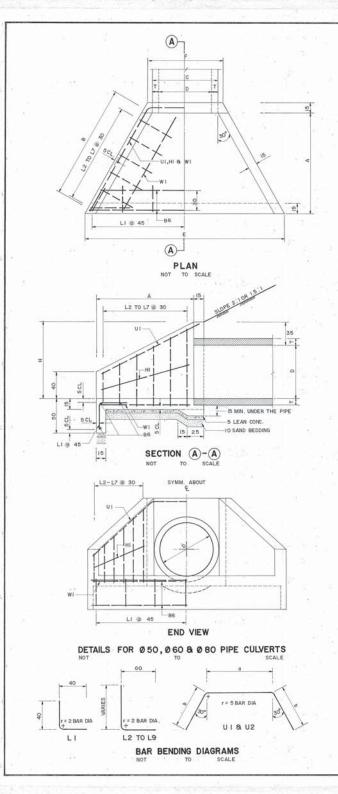
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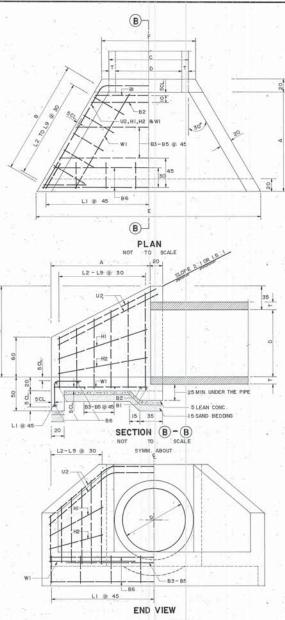
SUBMITTED: P. Burney P. SCALE NONE

DWG. NO. DS-102

APPROVED: SHEET NO. 69







# DETAILS FOR Ø 100 & Ø 120 PIPE CULVERTS

			TABLE	OF I	DIMENS	NOIS							
DIA. WALL THICKNES					DIMENSIONS								
PIPE	T		Α	В		-			1.5	0.60			
D .		S = 2:1	\$#1,511	В	C	E	F	н	a	ь			
50	. 7	90	75	104	64	203	81	85	77	118			
60	7.5	110	90	127	75	237	92	95	- 88	143			
80	9.5	150	120	173	99	307	116	115	112	193			
100	-11-	150	, 120	173	122	341	145	135	143	196			
120	12.5	190	150	219	145	411	168	155	166	246			

BAR	SIZE		D= 50		D= 60		D= 80		001 =	D# 120		
MARK	Ø (MM.)	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	
UI	12	1	313	1	374	1	498	-	-	-	170	
U2	16	-	-	-	-	-	-	2	535	2	658	
LI	12	5	80	6	80	8	80	8	80	10	80	
L2	16 (12)	2	107	2	107	2	107	2	132	2	132	
L3	16 (12)	2	120	2	120	2	120	2	145	2	145	
1.4	16(12)	2	133	2	133	2	133	2	158	2	158	
L5	16(12)	2	146	2	.146	2	146	2	171	2	171	
L6	16(12)	-	1	2	159	2	159	2	184	2	184	
L7	16(12)	-	23	-	-	2	172	2	197	2	197	
LS	16	-		-	Silver.	-		-	-	2	120	
L9	16	-		-	-	-	-	-	-	2	225	
WI	12	4	96-	4	119	4	165	4	165	4	211	
B 1	16	-	-	-	-	-	-	1	162	1	185	
B2	16	-	-	-	- 1	-		.1	174	.1	197	
B3	12		-	-	-	+		1	220	1	242	
B4 1	12	-	-	-	-	-	-	1	272	-1	294	
B5	12	-	- 2	-	11/12	-		-	21	1	346	
B6	12	3	-	3	-	3	-	3		3	-	
HI	12	2	99	2	122	2	169	2	181	2	225	
H2	12	-	-	-	-	7.	-	2	177	2	223	
REIN	F (KG.)		25		32		41		84		111	
CONC	(M <sup>3</sup> )		0.6		0.8		1.2		2.0		2.8	

## NOTES :

- I. ALL DIMENSION ARE IN CENTIMETERS UNLESS OTHERWISE INDICATED.
- 2. CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 210 KSC. FOR 15 x 15 x 15 CM. CUBE AT 28 DAYS. AN APPROXIMATE MIX DESIGN PER CUBIC METER IS SUGGESTED AS FOLLOWS

PORTLAND CEMENT TYPE I 350 SAND

0.43

CRUSHED ROCK OR GRAVEL 0.86

CONCRETE SLUMP

REINFORCING STEEL SHALL BE DEFROMED BARS CONFORMING TO TIS, 24 GRADE SD 30.

DESIGN WORKING STRESSES :-

(a) CONCRETE, fc = 70 KSC.

(b) STEEL , fs = 1400 KSC, n = 10

CLEAR CONCRETE COVER SHALL BE 4 CM. UNLESS OTHERWISE INDICATED

LOCATION OF LAP SPLICE OF REBARS SHALL BE APPROVED BY THE ENGINEER

LAP LENGTH SHALL NOT BE LESS THAN 24 BAR DIAMETER

ALL CONCRETE EXPOSED CORNERS SHALL HAVE 2 CM. CHAMFER UNLESS OTHERWISE INDICATED.

DIMENSION OF HEADWALL AND QUANTITIES OF MATERIAL SHOWN IN THE TABLE ARE FOR EMBANKMENT SLOPE OF 2:1 ONLY. FOR EMBANKMENT SLOPE 1.5:1, ALL DIMENSION SHALL BE WORKED OUT FROM DIMENSION OF "A" AS SHOWN TOGETHER WITH THE FLARE ANGLE OF 30 AND THEN QUANTITIES OF MATERIALS SHALL BE CHANGED ACCORDINGLY.

10. UNLESS OTHERWISE SPECIFIED, THESE HEADWALLS SHALL BE USED FOR HIGHWAYS CLASS D AND CLASS I.

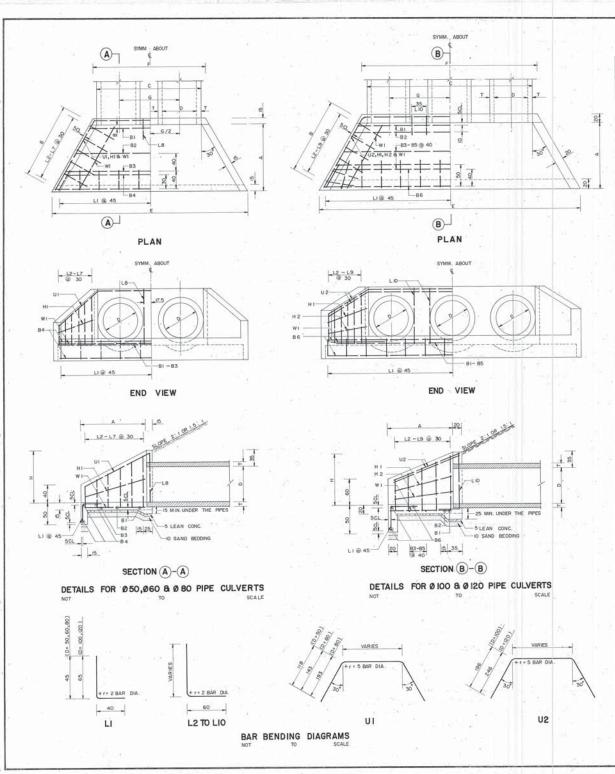
FOR ROLLING AND MOUNTAINOUS TERRAINS , THESE HEADWALLS SHALL BE USED AS NECESSARY .

#### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

R.C. HEADWALL FOR SINGLE R.C.P. CULVERT

DESIGNED : D.O.	H. B. CONSULTANTS	CHECKED:	DATE JULY 1994
SUBMITTED:	at ;	7	SCALE NONE
(DIRECTOR OF LOCATION & DESIGN DIVISION)			DWG. NO. DS-104
APPROVED:	(//// (DIRECTOR GE	NERAL)	SHEET NO. 71



					TABL	.E	OF I	DIME	NSI	ONS						
DIA.		1		ENSIG			DIMENSIONS									
PIPE	Τ.		ALL		TIPLE	s	FOR	FOR 2 PIPES FOR 3 PIPES		PES	FOR 4 PIPES					
-D.			S=2:1		В	н	G	с	Ε	F	С	E	F	с	E	F
50	7.	90	75	104	85	110	174	313	191	284	423	301	-	-	-	
60	7.5	110	90	127	95	120	195	357	212	315	477	332	-	-	-	
80	9.5	150	120	173	115	140	239	447	256	379	587	396	-	-	-	
100	11	150	120	173	135	170	292	511	315	462	681	485	632	851	655	
120	12.5	190	150	219	155	190	335	601	358	525	781	548	715	981	738	

			TA	77.7	T (5)	Met.	INFOR		107 LT 200 LT	ė.			100
			FO	R 2	PIPES		FOR 3 PIPES						
BAR	SIZE	0	D = 50		D= 60		D = 80		D = 50	D = 60		D = 80	
mount		NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO	LENGTH	NO.	LENGTH
Bi	12	1	202	1	223	ı	267	1	312	ï	343	1	- 407
82	12	-	-	1	246	1	336	-	-	E	366	1	476
83	12	1	248	1	292	1	382	1	358	1	412	1	522
84	12	3	-	3	-	3	-	3	-	3	-	3	-
LI	12	8	85	9	85	11	85	10	85	11	85	14	85
L2-L7	12	8	-	10	-	12	-	8	-	10	-	12	-
L8	12	2	130	2	160	2	180	4	130	4	160	4	180
UI	12	1	423	1	494	i	638	1	533	1	614	1	778
н	12	2	99	2	122	2	169	2	99	2	122	2	169
WI	12	4	96	4	119	4	165-	4	96	4	119	4	165

			TABLE (	OF QUAN	TITIES					
DIA. OF		- (1	CONCRETE -HDWL.) N		REINF. BARS , TOTAL WT. (I - HOWL.) KG.					
PIPE "D"	т	FOR 2 PIPES	FOR 3 PIPES	FOR 4 PIPES	FOR 2 PIPES	FOR 3 PIPES	FOR 4 PIPES			
50	7	0.9	1.3	-	41	51				
60	.7.5	1.2	1.6	141	50	63				
80	9.5	1.8	2.3	-	64	79	-			
100	-11	3.0	4.0	5.0	121	151	181			
120	12.5	4.0	5.3	6.6	141	175	208			

							D1 N 1801			_		_	
			FOR 2	PIF	ES		FOR 3	PIP	ES		FOR 4	PIPE	s
BAR MARK	SIZE	0	* 100	D	= 120	D	= 100	D	= 120	D	* 100	D	e 120
	(MM.)	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH
ВІ	16	1	332	1	375	1	502	1	565	1	672	1	755
82	16	1	344	1	387	T	514	1	577	1	384	1	767
83	12	1	351	13	440	1	521	1	630	1	691	1	820
84	12	1	398	1	487	1	568	1	677	1	738	1	867
85	12	t	444	1	533	1	614	1	723	1	784	1	913
B6	12	3	-	3	-	3	-	3	-	3	-	3	-
н	12	2	177	2	223	2	177	2	223	2	177	2	223
HZ	12	2	181	2	235	2	181	2	235	2	181	2	235
LI ·	12	12	105	14	105	16	105	18	105	20	105	23	105
L2	16	2	132	2	132	2	132	2	132	2	132	2	132
L3:	16	2	145	2	145	2	145	2	145	2	145	2	145
L4	16	2	158	2	158	2	158	2	158	2	158	2	158
L5	16	2	171	2	171	2	171	2	171	2	171	2	171
L6	16	2	186	2	186	2	186	2	186	2	186	2	186
L7	16	2	197	2	197	2	197	2	197	2	197	2	197
L8	16	-	-	2	210	-	-	2	210	7	-	2	210
L9	16	-	-	2	225	-	-	2	225	-	-	2	225
LIO	16	2	205	2	225	4	205	4	225	6	205	6	225
U2	16	2	705	2	444	2	475	2	1035	2	162	2	1228
WI	12	4	165	4	211	4	165	4	211	4	162	4	211

TABLE OF REINFORCMENT FOR Ø IOO & Ø I2O CM.

#### NOTES

ALL DIMENSIONS ARE IN CENTIMETERS UNLESS OTHERWISE INDICATED.
 CONCRETE SHALL HAVE A MINMAUM ULTIMATE COMPRESSIVE STRENGTH

OF 210 KSC, FOR 15 x15 x15 CM. CUBE AT 28 DAY, AN APPROXIMATE MIX DESIGN PER CUBIC METER S SUGGESTED AS FOLLOWS:

PORTLAND CEMENT TYPE I	350	KG. (MIN.)
SAND	0.43	M.3
CRUSHED ROCK OR GRAVEL	0.86	м.
CONCRETE SLIMP	10	CM.(MAX.)

- 3. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO TIS. 24
- 4. DESIGN STRESSES :

a ) CONCRET	ΓE,	fc		70	KSC.
b ) STEEL	,	fs		1,400	KSC.
			-	10	

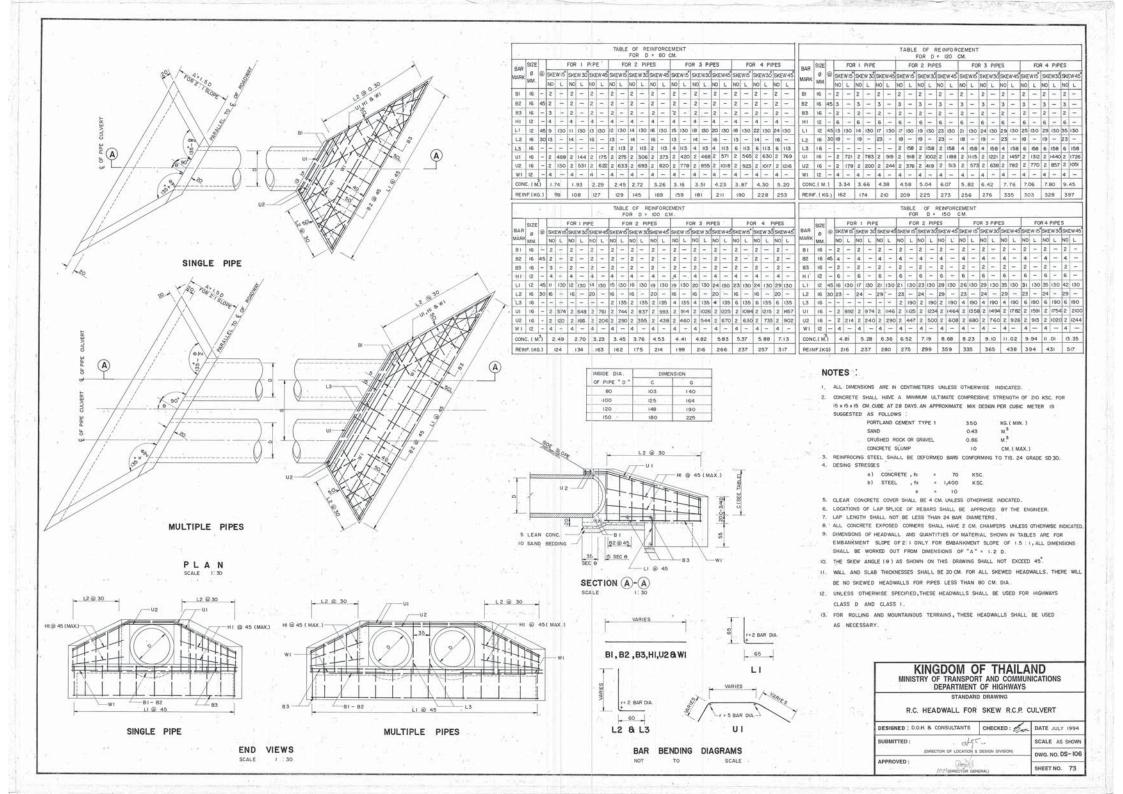
- 5. CLEAR CONCRETE COVER SHALL BE 4 CM. UNLESS OTHERWISE INDICATED.
- 6. LOCATIONS OF LAP SPLICE OF REBARS SHALL BE APPROVED BY THE ENGINEER
- 7. LAP LENGTH SHALL NOT BE LESS THAN 24 BAR DIAMETERS.
- ALL CONCRETE EXPOSED CORNERS SHALL, HAVE 2 CM. CHAMFER UNLESS OTHERWISE INDICATED.
- 9. DMENSION OF HEADWILL AND QUANTITIES OF MATERIALS SHOWN IN THE TABLES ARE FOR EMBANAMENT SLOPE OF 2:1 ONLY FOR EMBANKMENT SLOPE 1.5:1, IALL DIMENSIONS SHALL BE WORKED OUT FROM DIMENSIONS OF "A" AS SHOWN TOGETHER WITH THE FLARE ANGLE OF 30" THEN QUANTIES OF MATERIALS SHALL BE CHANGED ACCORDINGLY.
- UNLESS OTHERWISE SPECIFIED, THESE HEADWALLS SHALL BE USED FOR HIGHWAYS CLASS D AND CLASS I.
- FOR ROLLING AND MOUNTAINOUS TERRAINS, THESE HEADWALLS SHALL BE USED AS NECESSARY.

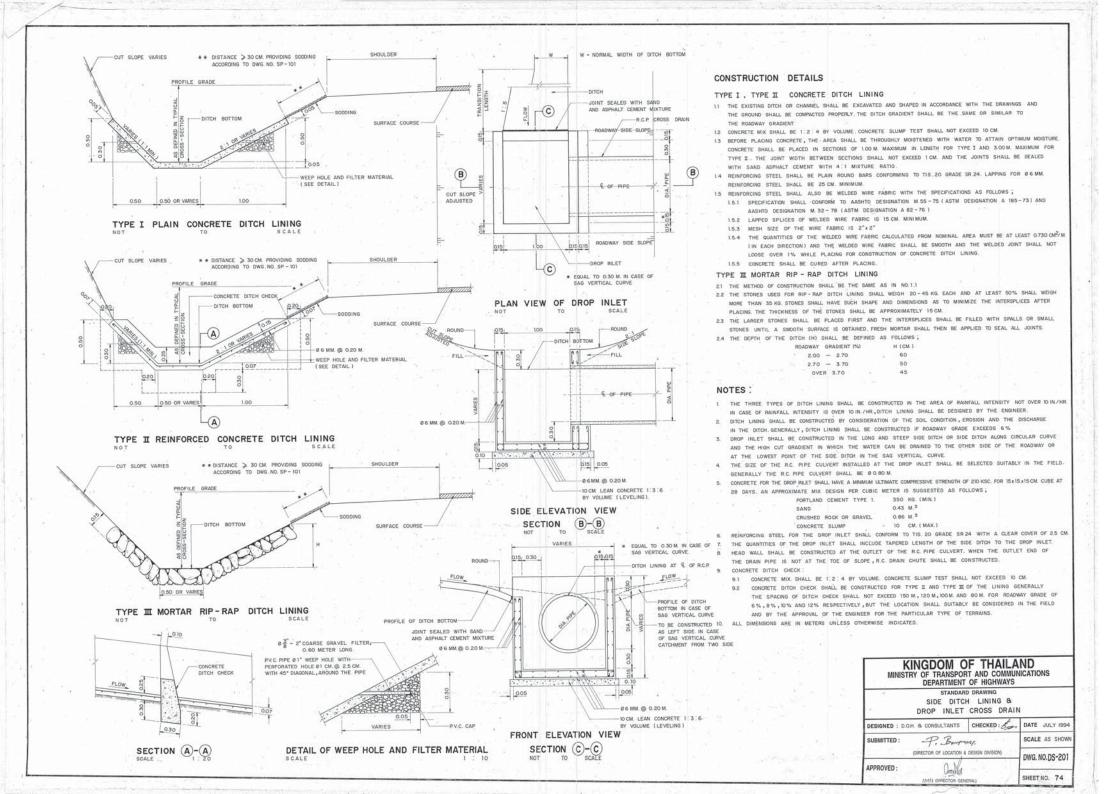
# KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

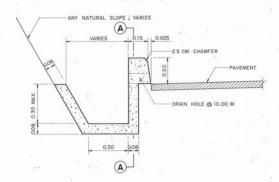
STANDARD DRAWING

R.C. HEADWALL FOR MULTIPLE R.C.P. CULVERT

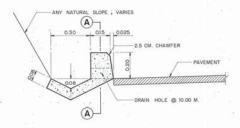
DESIGNED: D.O.H. & CONSULT	NTS CHECKED	DATE JULY 1994
SUBMITTED:	JF5.	SCALE AS SHOWN
Quintariori 61 6	CATION & DESIGN DIVISION)	DWG. NO. DS-105
APPROVED:	TOR GENERALS	SHEET NO. 72







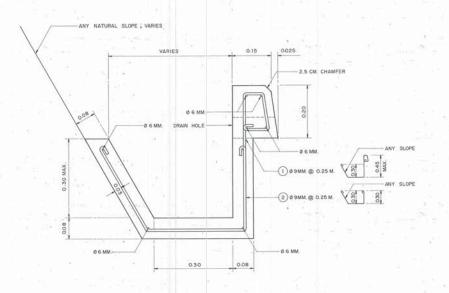
CONCRETE DITCH TYPE "A"



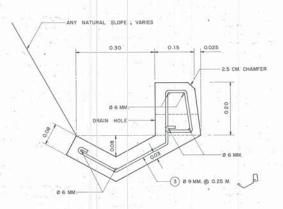
CONCRETE DITCH TYPE "B"



SECTION A-A



CONCRETE DITCH TYPE " A "



CONCRETE DITCH TYPE " B "

#### NOTES:

- 1. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH
   OF 180 KSC. FOR 15 x 15 x 15 cm. CUBE AT 28 DAYS. AN APPROXIMATE MIX DESIGN PER CUBIC METER IS SUGGESTED AS FOLLOWS:

PORTLAND CEMENT 320 KG. (MIN.)

SAND

0.43 M<sup>3</sup>

CRUSHED ROCK OR GRAVEL

0.86 M.3

CONCRETE SLUMP

10 CM. ( MAX.)

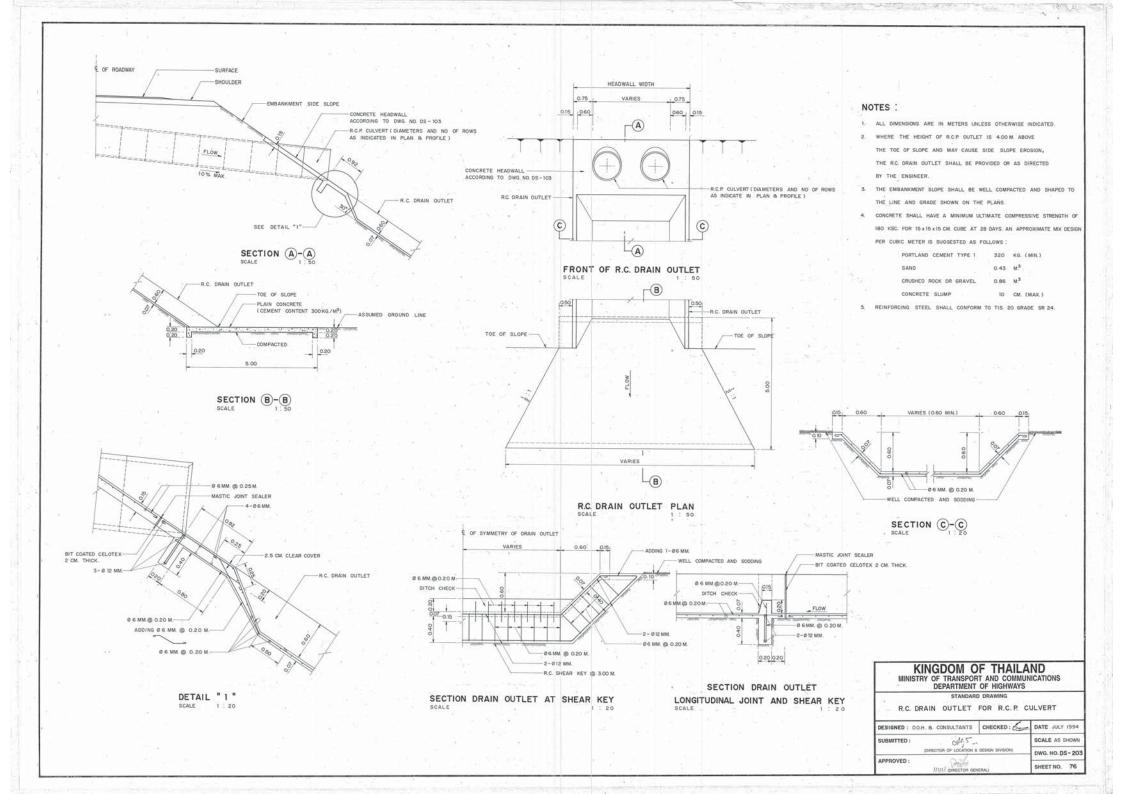
- 3 REINFORCING STEEL SHALL CONFORMED TO TIS 20 GRADE SR 24.
- 4. CLEAR CONCRETE COVER SHALL BE 3 CM.
- IN CASE OF LAPPED SPLICES OF THE REINFORCEMENT THE LAPPED DISTANCE MUST NOT BE LESS THAN 40 TIMES OF THE DIAMETER OF THE REINFORCING STEEL OR AS DIRECTED BY THE ENGINEER.
- ALL OF THE CONSTRUCTION MATERIALS MUST CONFORMED TO D.O.H. STANDARDS.
- 7 EXPANSION JOINT OF 1 CM. WIDTH MUST BE PROVIDED AT THE INTERVAL OF 10 METERS AND FILLED WITH MORTAR (1:3 BY VOLUME )

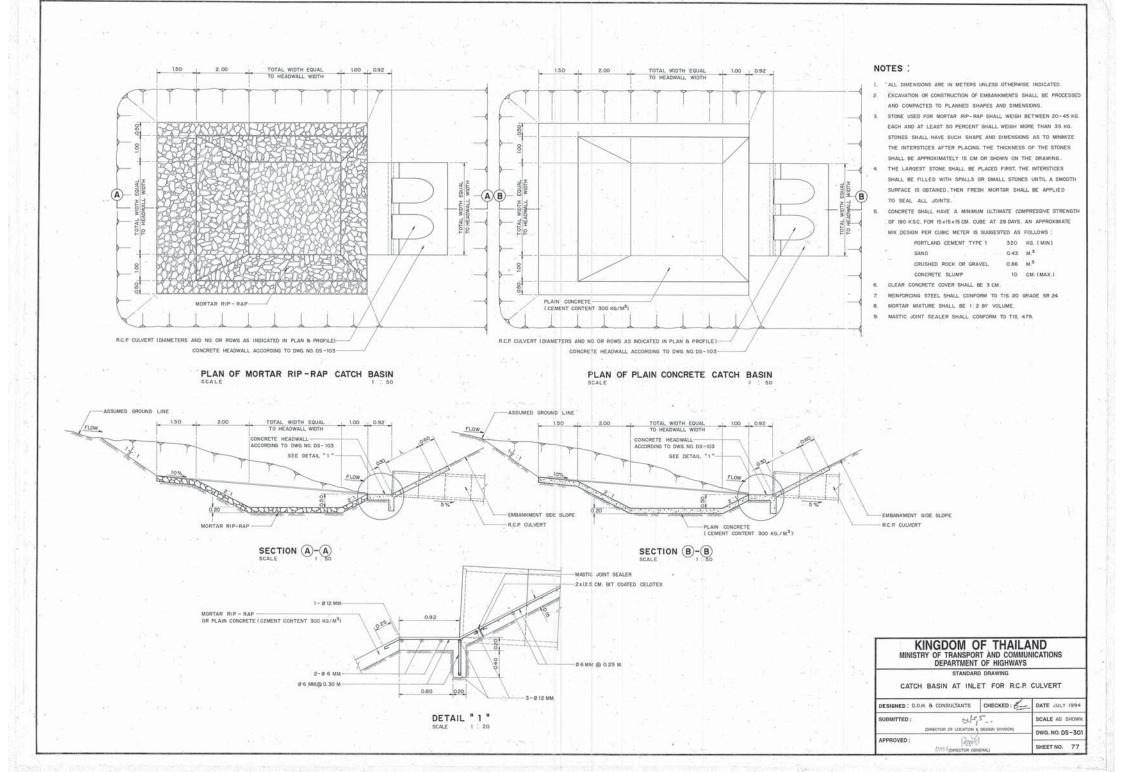
KINGDOM OF THAILAND
MINISTRY OF TRANSPORT AND COMMUNICATIONS
DEPARTMENT OF HIGHWAYS

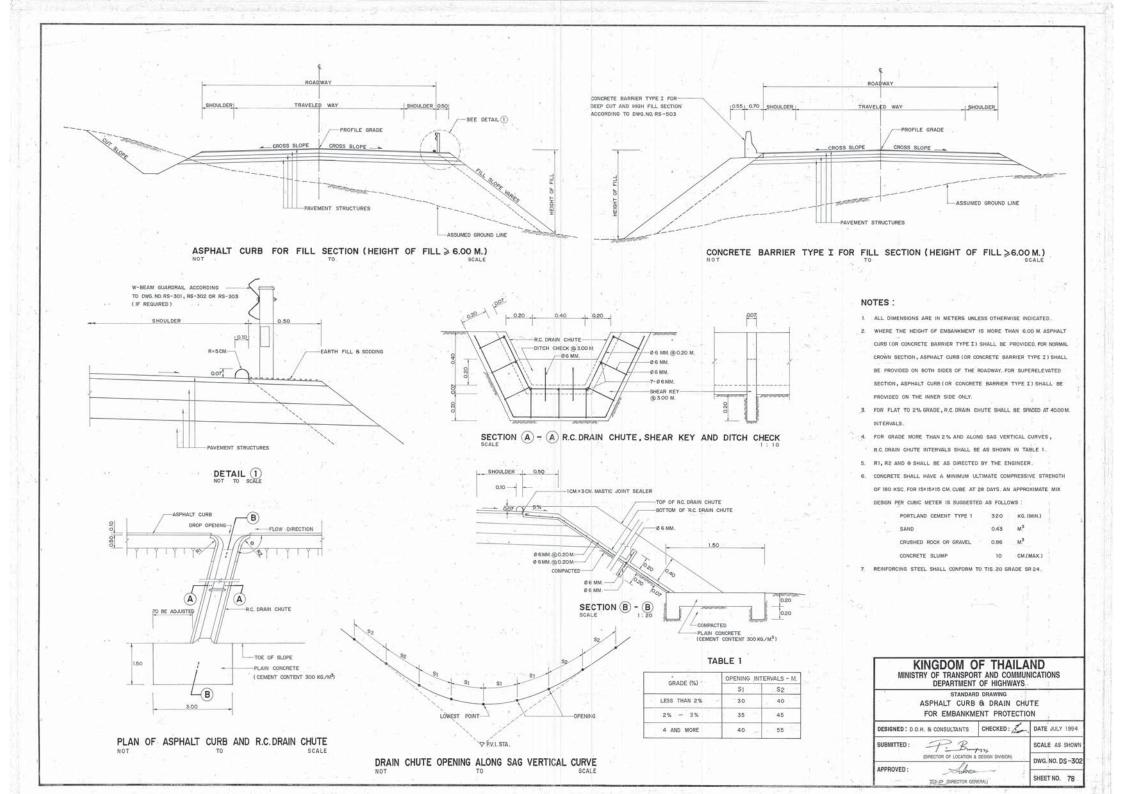
STANDARD DRAWING

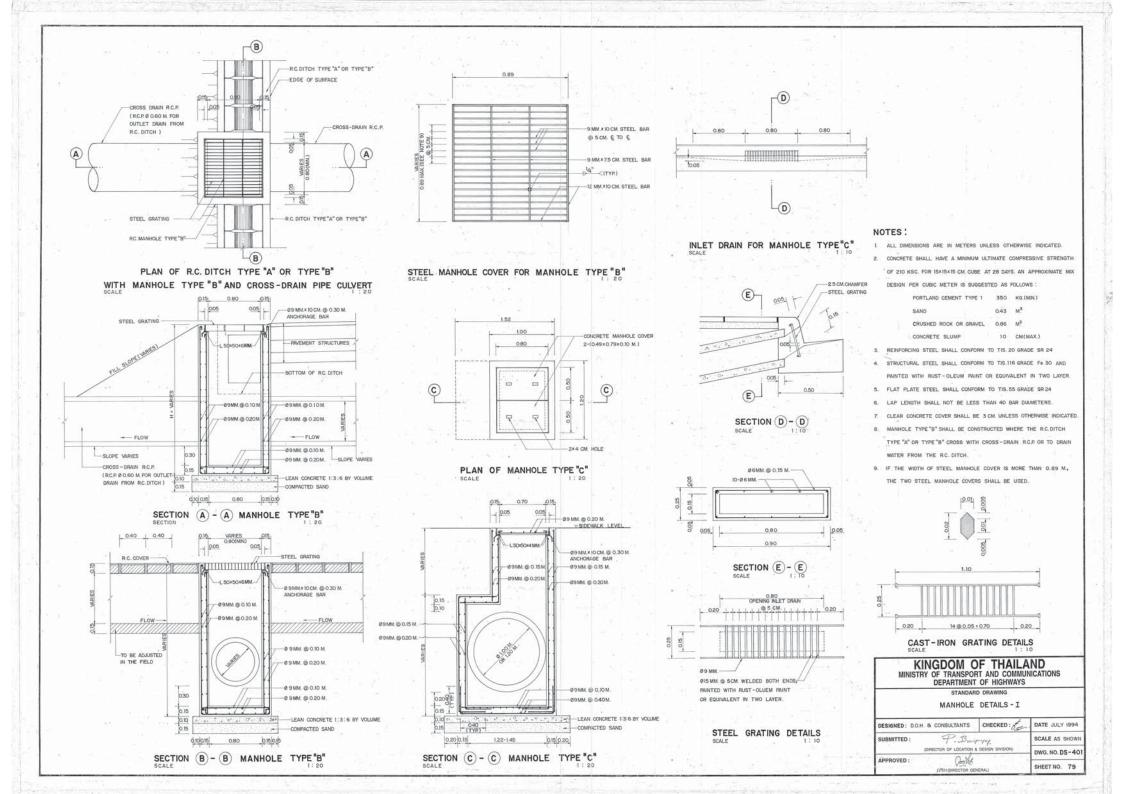
CONCRETE DITCH AT HILLSIDE

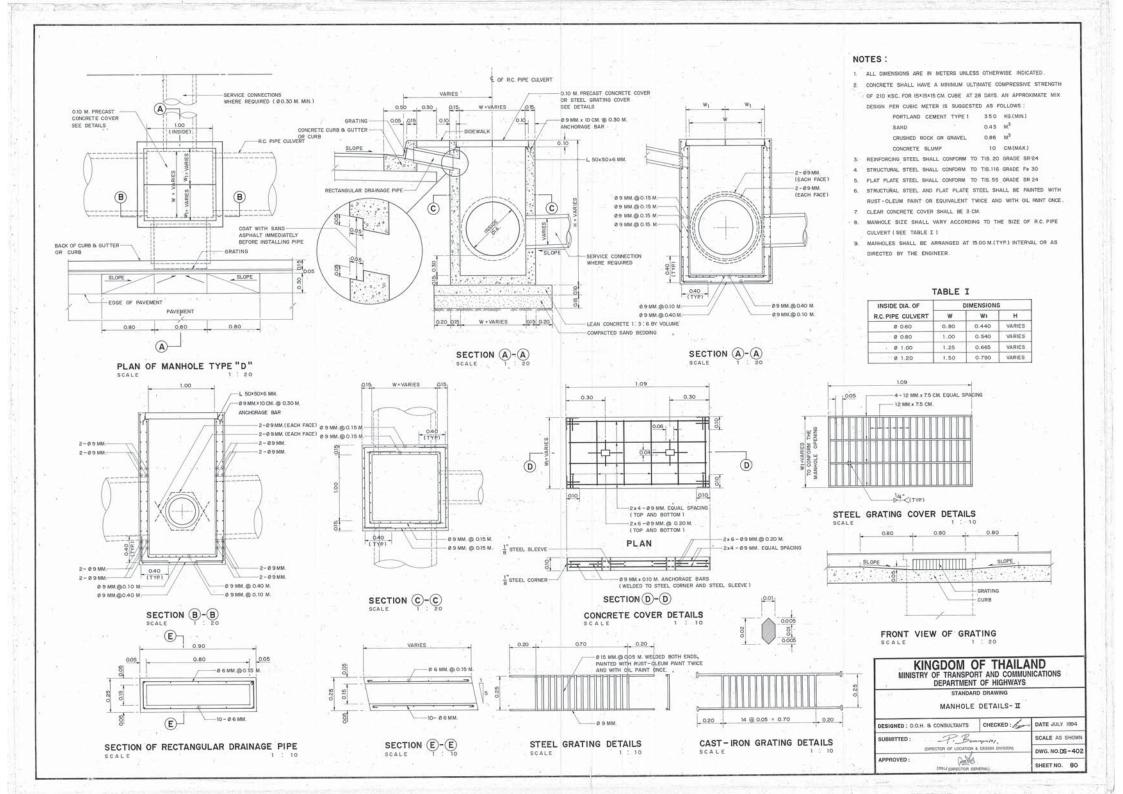
DESIGNED: D.O.H. & CONSULTANTS	CHECKED:	DATE JULY 1994
SUBMITTED:	<	SCALE AS SHOWN
(DIRECTOR OF LOCATIO	& DESIGN DIVISION)	DWG. NO. DS-202
APPROVED:	SENERAL)	SHEET NO. 75

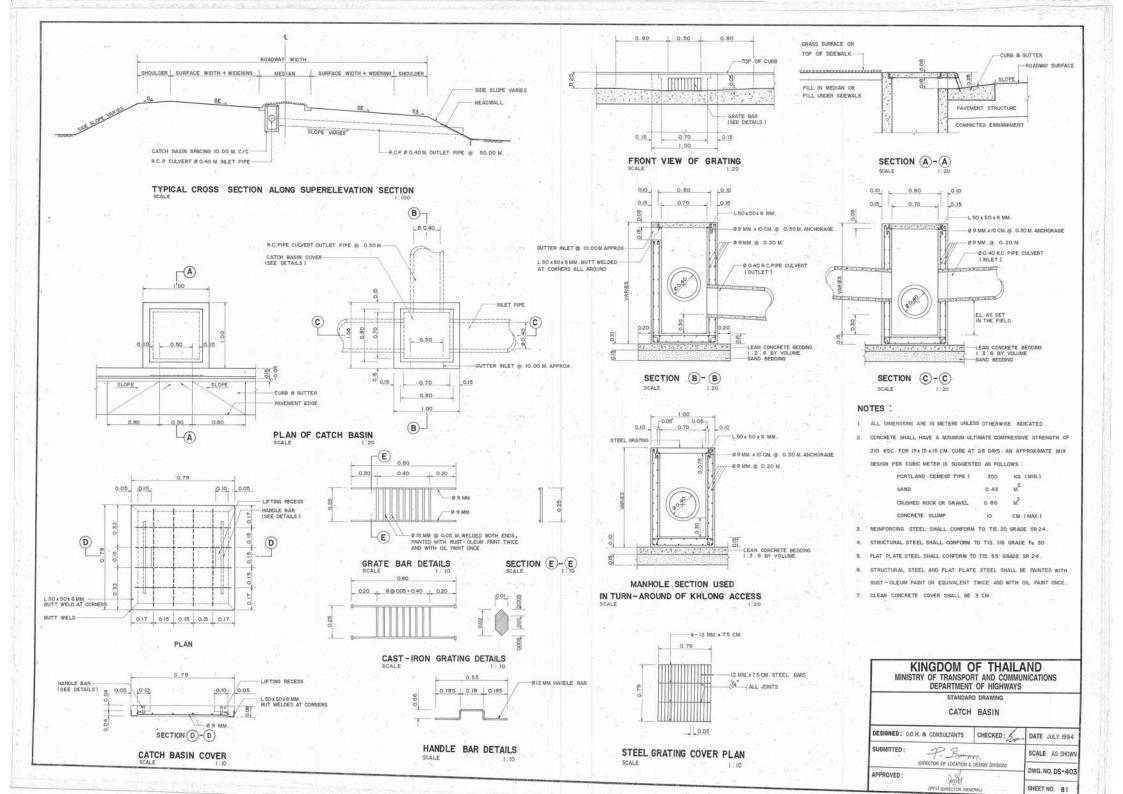


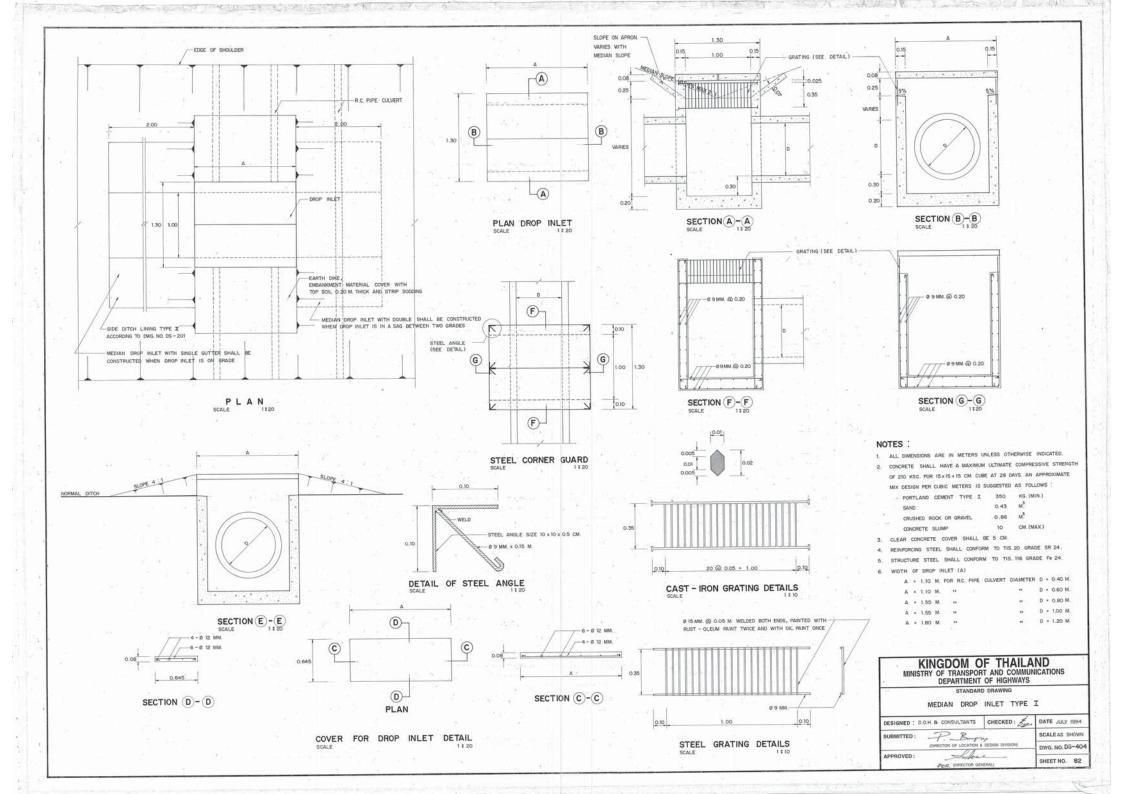


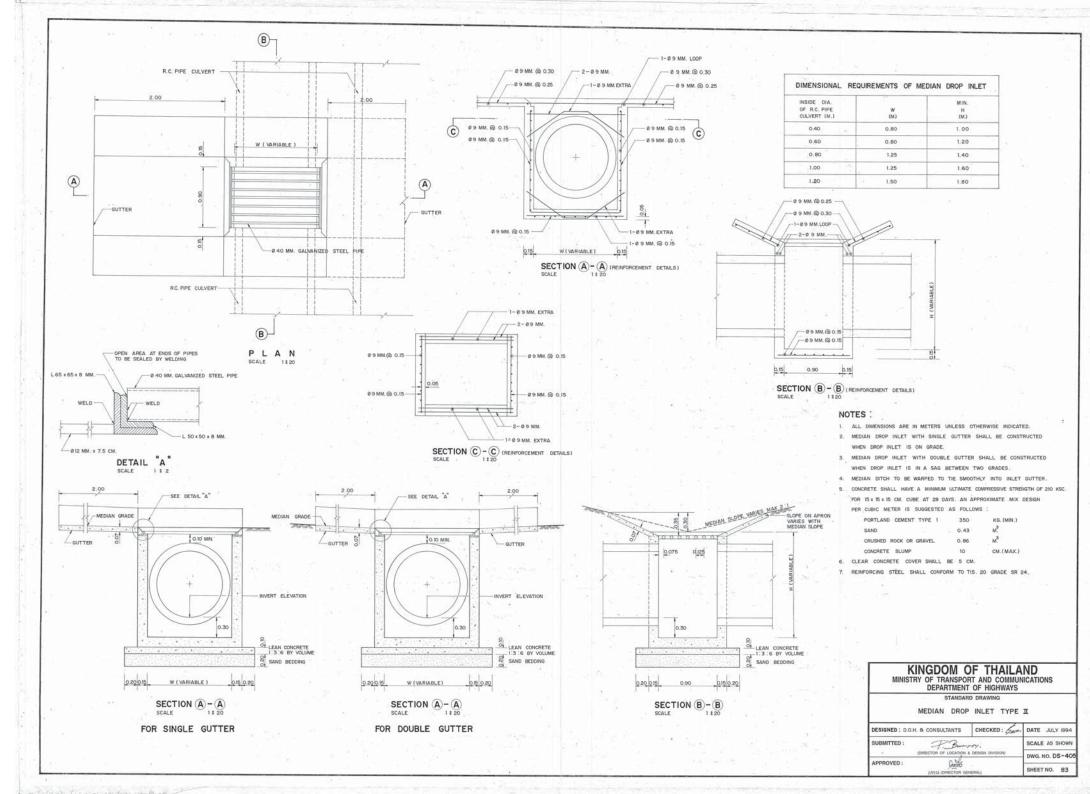


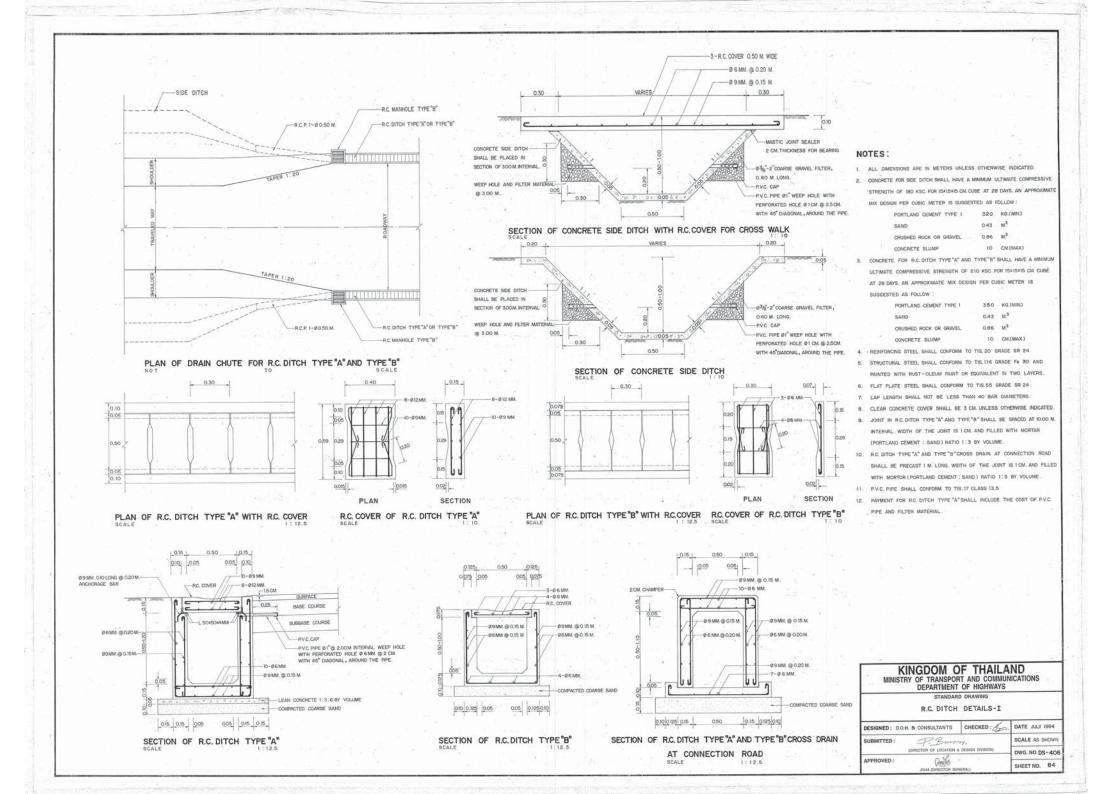


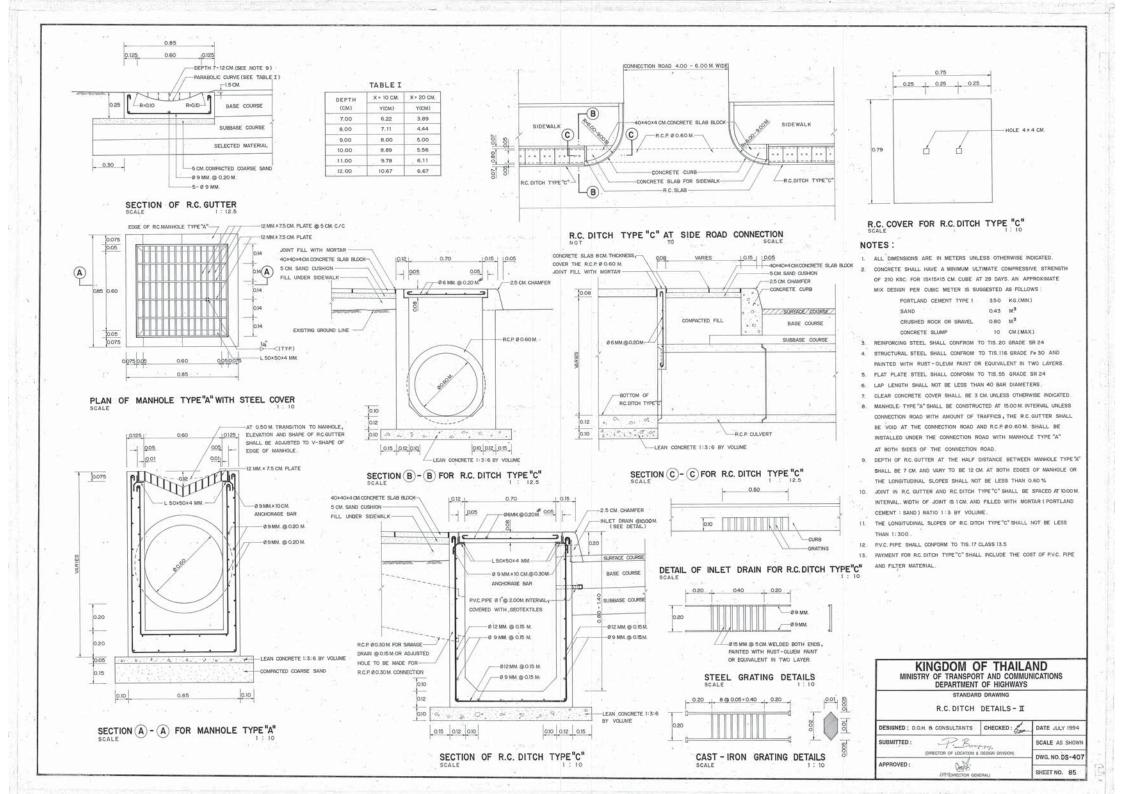


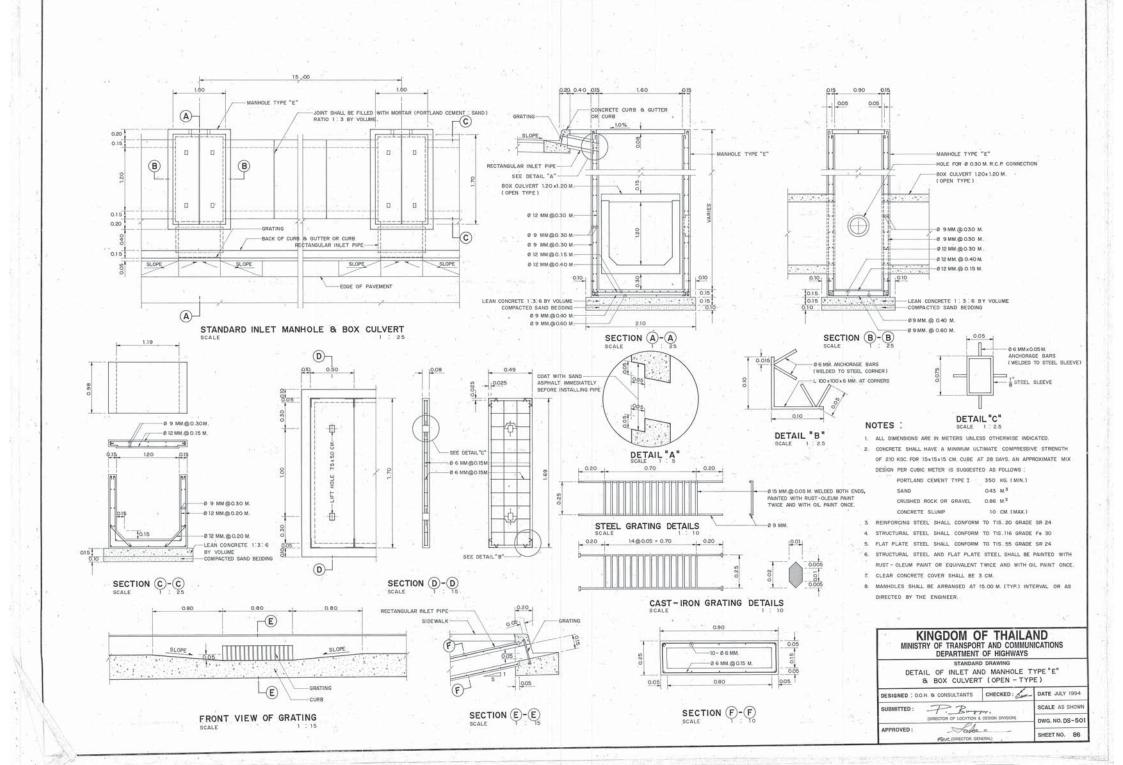


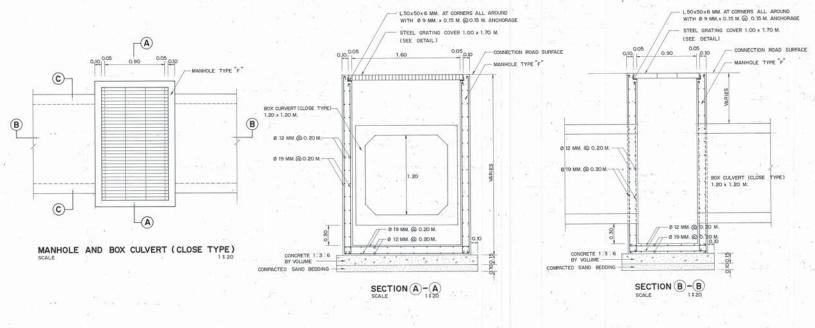


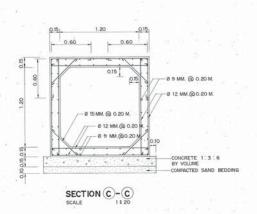


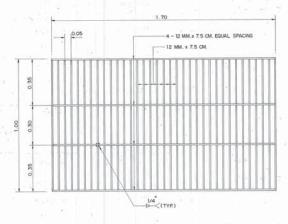












STEEL GRATING COVER DETAILS

#### NOTES:

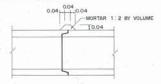
- 1. ALL-DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- 2. BOX CULVERT (CLOSE TYPE) SHALL BE USED FOR CONNECTION ROAD ONLY
- 3. IF NECESSARY TO CONSTRUCT MANHOLE AT ROADWAY, MANHOLE COVER SHALL BE STEEL GRATING.
- 4. CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTY OF 210 KSC. FOR 15 x 15 x 15 CM. CUBE AT 28 DAYS. AN APPROXIMATE MIX DESIGN PER CUBIC METER IS SUGGESTED AS FOLLOWS

KG. (MIN.) PORTLAND CEMENT TYPE 1 0.43 M<sup>3</sup> CRUSHED ROCK OR GRAVEL 10 CONCRETE SLUMF

- 5. REINFORCING STEEL SHALL CONFORM TO TIS. 20 GRADE SR 24.
- 6. STRUCTURAL STEEL SHALL CONFORM TO TIS. 116 GRADE Fe 30.
- FLAT PLATE STEEL SHALL CONFORM TO TIS 55 GRADE SR 24.
- STRUCTURAL STEEL AND FLAT PLATE STEEL SHALL BE PAINTED WITH RUST - OLEUM PAINT OR EQUIVALENT TWICE AND WITH OIL PAINT ONCE.
- 9. CLEAR CONCRETE COVER SHALL BE 3 CM
- 10. THE LENGTH OF BOX CULVERT (CLOSE TYPE) FOR PAYMENT SHALL BE MEASURED AT OUTER EDGE OF MANHOLE:

#### PRE - CAST BOX CULVERT

- 1. THE CONTRACTOR CAN USE PRE-CAST REINFORCED CONCRETE BOX SECTION WHICH CONFORM TO ASTM DESIGNATION C 789M-83 OR
- 2. DESIGN REQUIREMENTS
- 2.1 THE DESIGN EARTH COVER SHALL BE THE MAXIMUM VALUE AT FIELD CONDITION BUT NOT LESS THAN 0.60 M.
- 2.2 BOX CULVERT SIZE SHALL BE 1.20 x 1.20 M. WITH THICKNESS NOT LESS THAN 0.125 M.
- 2.3 THE CIRCUMFERENTIAL REINFORCEMENT OF BOX CULVERT AT ROADWAY SHALL BE CALCULATED FROM WEIGHT OF BACKFILL AND HS 20 - 44 TRUCK LOADING (FROM THE STANDARD SPECIFICATION FOR HIGHWAY BRIDGES - AASHTO ), BUT UNDER SIDEWALK , THE CIRCUMFERENTIAL REINFORCEMENT SHALL BE CALCULATED FROM WEIGHT OF BACKFILL ONLY
- 2.4 REINFORCEMENT STEEL SHALL CONFORM TO ASTM A-82 OR A-496
- 2.5 CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 280 KSC. FOR 15 x 15 x 15 CM. CUBE AT 28 DAYS.
- 4. JOINT OF PRE CAST BOX CULVERT SHALL BE TONGUE AND GROOVE
- 5. JOINT SHALL BE FILLED WITH MORTAR (PORTLAND CEMENT : SAND ) RATIO 1:3 BY VOLUME.



- 6. METHOD OF PLACING AND BEDDING OF PRE CAST BOX CULVERT SHALL BE THE SAME AS THE CAST IN - PLACE BOX CULVERT.
- 7. THE LONGITUDINAL REINFORCEMENT SHALL HAVE DIAMETER NOT LESS THAN 5 MM. WITH SPACING NOT MORE THAN 15 CM.

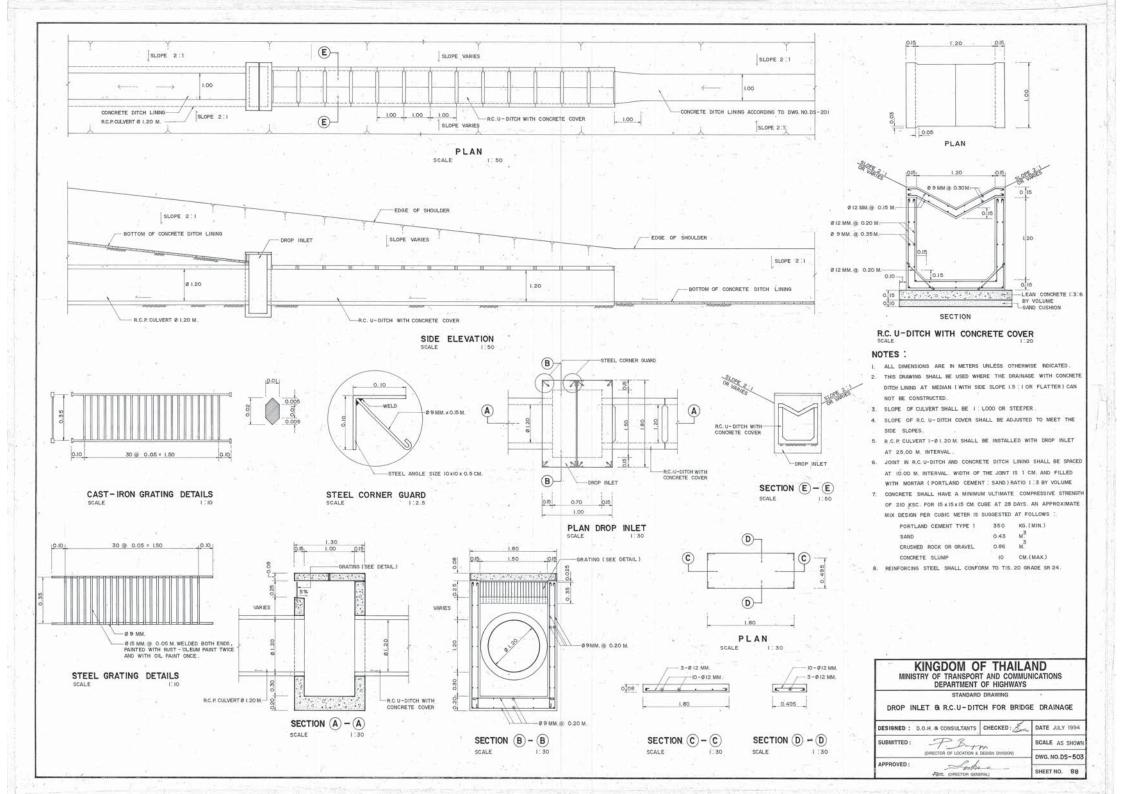


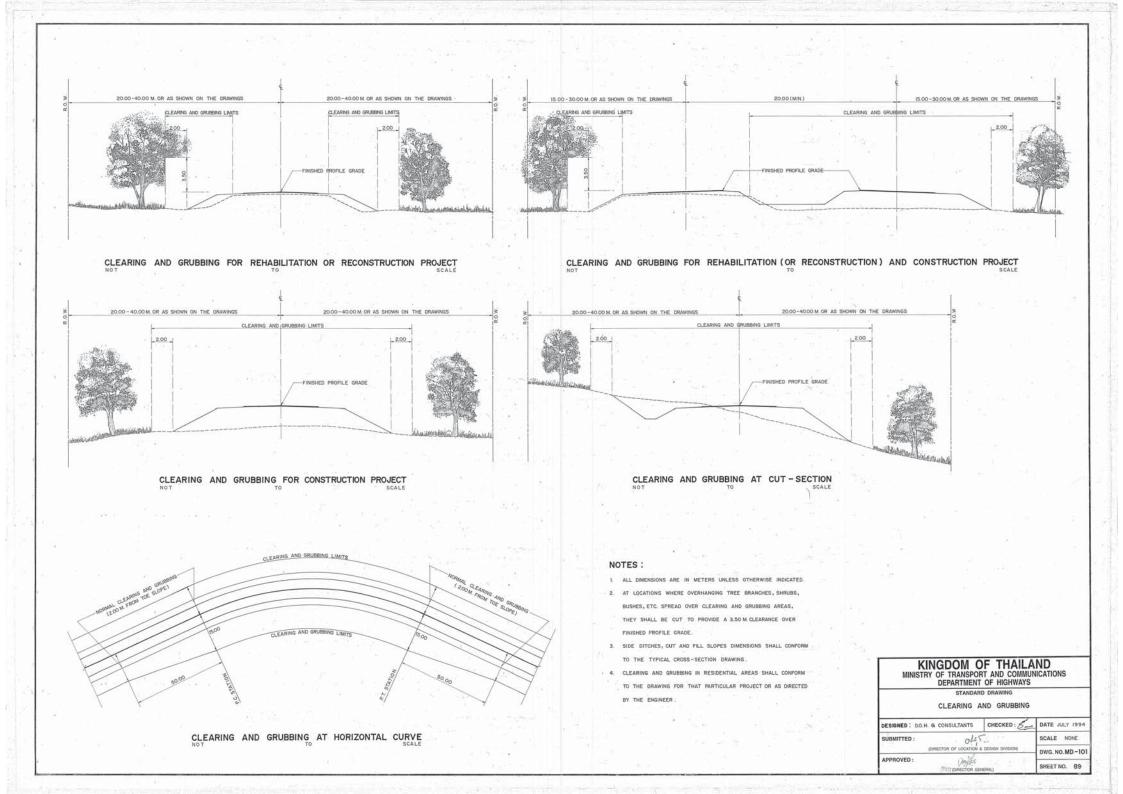
DETAIL OF INLET AND MANHOLE TYPE "F" & BOX CULVERT (CLOSE - TYPE )

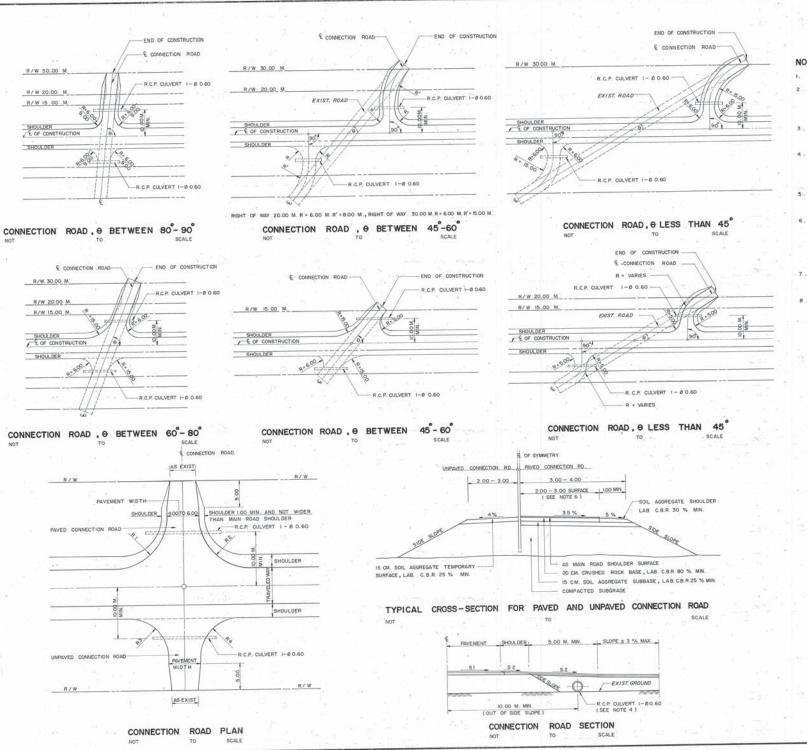
DESIGNED : D.O.H & CONSULTANTS CHECKED : 6-DATE JULY 1994 F. Bayry SCALE AS SHOWN

IDIRECTOR OF LOCATION & DESIGN DIVISIONS DWG. NO. DS-502

SHEET NO. 37 FOR IDIRECTOR GENERALL







#### NOTES:

- 1. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- IN GENERAL, PUBLIC SIDE ROAD CONNECTS THE HIGHWAY TO A VILLAGE, POPULATED AREA, MONASTERY, GOVERNMENT OFFICE, PUBLIC UTILITY OR SAME PLACES OF IMPORTANCE.
- THE PAYEMENT STRUCTURE OF EACH CONNECTION ROAD SHALL BE AS RECOMMENDED IN THE DRAWING.
- 4. CHANGE IN SIZE AND NUMBER OF PIPE CULVERT SHALL REQUIRE APPROVAL OF THE ENGINEER, LOCATION AND LENGTH OF PIPE CULVERT DURING CONSTRUCTION SHALL BE AS DIRECTED BY THE ENGINEER IN THE FIELD.
- 5. THE CONSTRUCTION OF EACH CONNECTION ROAD SHALL END AT R/W
  LIMIT OR AS SHOWN ON THE DRAWING.
- 6. THE WIDTH OF PAVED SURFACE OF THE CONNECTION ROAD SHALL BE

  THE SAME AS OR AS THE EXISTING PAVED CONNECTION ROAD OR AS SHOWN

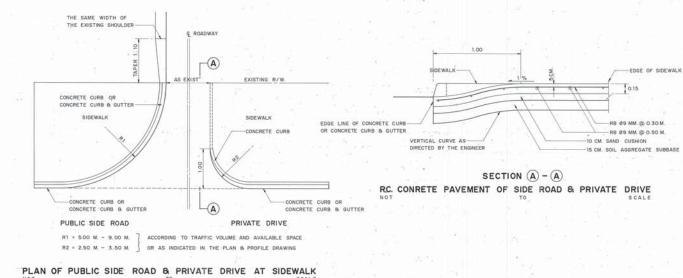
  ON THE DRAWING OR AS DIRECTED BY THE ENGINEER BUT SHALL NOT

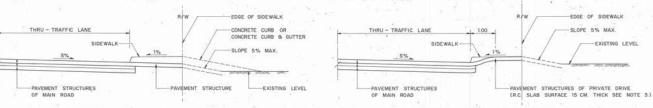
  BE WIDER THAN 6.00 M.
- 7. THE INTERSECTIONS NOT SHOWN ON THE DRAWINGS SHALL REQUIRE PLANS SUBMITTED TO THE ENGINEER FOR APPROVAL SEFORE CONSTRUCTION.
- 8 FOR SIDE ROAD CONNECTED TO A SMALL VILLAGE WITH LOW TRAFFIC,
  THE FOLLOWING SHALL APPLY:
  - THE CONNECTION ROAD SHALL BE UNPAYED ROAD TO THE R/W LIMIT WITH A RECOMMENDED PAVEMENT STRUCTURE AS SHOWN ON THE DRAWING.
  - b. THE ROADWAY WIDTH SHALL BE AS SHOWN ON THE DRAWING OR AS DIRECTED BY THE ENGINEER.

KINGDOM OF THAILAND
MINISTRY OF TRANSPORT AND COMMUNICATIONS
DEPARTMENT OF HIGHWAYS
STANDARD DRAWING

CONNECTION ROAD DETAILS

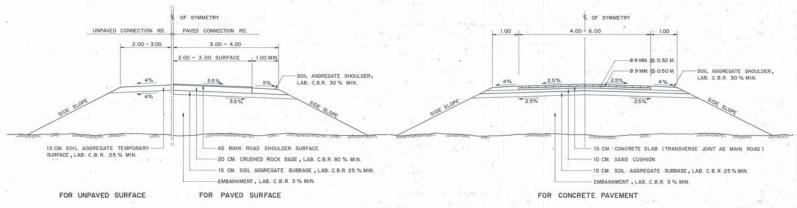
DESIGNED : D.O.H. & CONSULTANTS	CHECKED:	DATE JULY 1994
SUBMITTED: OF		SCALE NONE
(DIRECTOR OF LOCATION	& DESIGN DIVISION)	DWG. NO.MD-20
APPROVED:	ENERAL)	SHEET NO. 90





TYPICAL SIDE ROAD AT SIDEWALK (PROFILE)

TYPICAL PRIVATE DRIVE AT SIDEWALK (PROFILE)



TYPICAL CROSS - SECTION FOR SIDE ROAD & PRIVATE DRIVE CONNECTION

#### CRITERIA AND LIMITS OF CONSTRUCTION

- THE PROJECT ENGINEER SHALL LIST THE CONNECTION ROADS THAT NEED TO BE RECONSTRUCTED AT THE BEGINNING OF THE PROJECT.
- THE PAYMENT FOR CONSTRUCTION OF THE PUBLIC SIDE ROADS (FROM THE LIST OF ITEM.1) ARE
   THE DRAINAGE STRUCTURE (EXTENDED OR NEW STRUCTURE)
  - 2.2 THE PAVEMENT STRUCTURE, THAT SHOULD BE AS GOOD AS, OR BETTER THAN THE EXISTING PAVEMENT STRUCTURE.
- 3. THE PAYMENT FOR PRIVATE DRIVE ( FROM THE LIST OF ITEM. 1 ) ARE
  - 3.1 IN CASE OF GETTING PERMISSION FROM THE DOM.
    - 3.11 THE DRAINAGE STRUCTURE (EXTENDED OR NEW STRUCTURE)
    - 3.12 FOR THE PAVEMENT STRUCTURE, DOH. SHALL CONSTRUCT THE TEMPORARY PAVEMENT DURING THE CONSTRUCTION OF THE HIGHWAY, WHEN THE PROJECT IS COMPLETED, THE OWNER OF EACH PRIVATE DRIVE SHALL PAY FOR THE PAVEMENT STRUCTURE FOR HIMSELF.
  - 32 IN CASE OF NOT GETTING PERMISSION FROM THE DOH.
  - 3.21 DDH. SHALL PAY FOR THE DRAINAGE STRUCTURE ONLY WHEN THE PRIVATE DRIVE INTERFERES WITH THE DRAINAGE SYSTEM OF THE HIGHWAY.
  - 322 ALL OF THE OWNERS OF THE PRIVATE DRIVED ARE REQUIRED TO SEEK THE PERMISSION FROM THE DOH OTHERWISE, ILLEGAL PRIVATE DRIVES SHALL BE REMOVED ACCORDING TO THE ITEM 33 OF THE REVOLUTION ANNOUNCEMENT NO. 295
- EVERY NEW PRIVATE DRIVES AFTER THE START OF THE PROJECT IS NEEDED TO GET THE PERMISSION FROM THE DOM. BEFORE THE CONNECTION CAN BE STARTED
- 5. THE DOH, BY THE LOCATION AND DESIGN DIVISION SHALL DIRECT
  - 5.1 THE ARRANGMENT OF THE DRAINAGE STRUCTURE.
- 5.2 THE LIMIT OF CONSTRUCTION CONNECTION ROADS
- 53 THE TYPE AND QUANTITIES OF PAVEMENT STRUCTURE

#### NOTES :

- 1. ALL DIMENSION ARE IN METERS UNLESS OTHERWISE INDICATED
- IN GENERAL, PUBLIC SIDE ROAD CONNECTS THE HIGHWAY TO THE VILLAGE, POPULATED AREA, MONASTERY, GOVERNMENT OFFICE, PUBLIC UTILITY OR SOME PLACES OF IMPORTANCE WHILE "PRIVATE DRIVE" IS THE PRIVATE CONNECTION ROAD FOR PRIVATE HOUSE.
- CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 325 KSC.
   FOR 15x15x15 CM. CUBE AT 28 DAYS. AN APPROXIMATE MIX DESIGN PER CUBIC METER IS SUGGESTED AS FOLLOWS:

PORTLAND CEMENT TYPE 1 350 KG.(MIN.)

SAND 0.43 M.3

CRUSHED ROCK OR GRAVEL 0.86 M.3

REINFORCING STEEL SHALL BE ROUND BARS CONFORMING TO TIS 20 GRADE SR 24.

5. CLEAR CONCRETE COVER SHALL BE 5 CM.

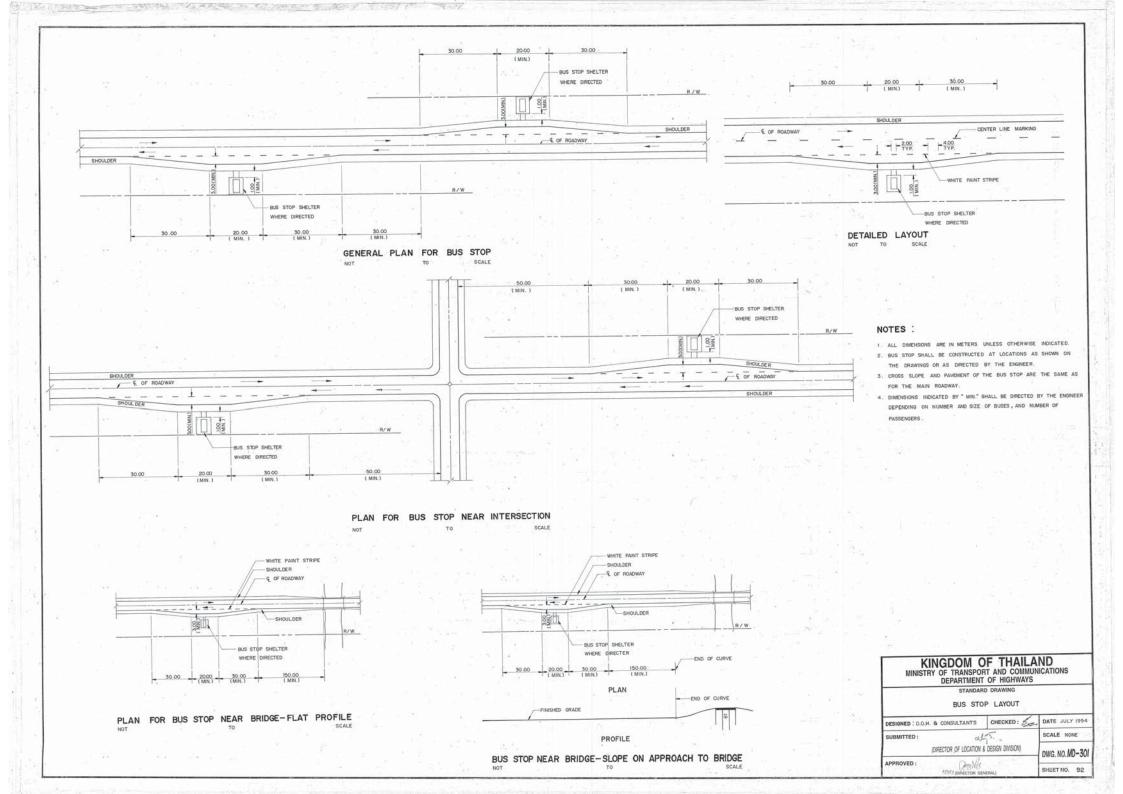
FOR THE PRIVATE DRIVE THAT NEEDS TO BE CONCRETE PAVEMENT, REFER TO SECTION  $\widehat{\bf A} - \widehat{\bf A}$ 

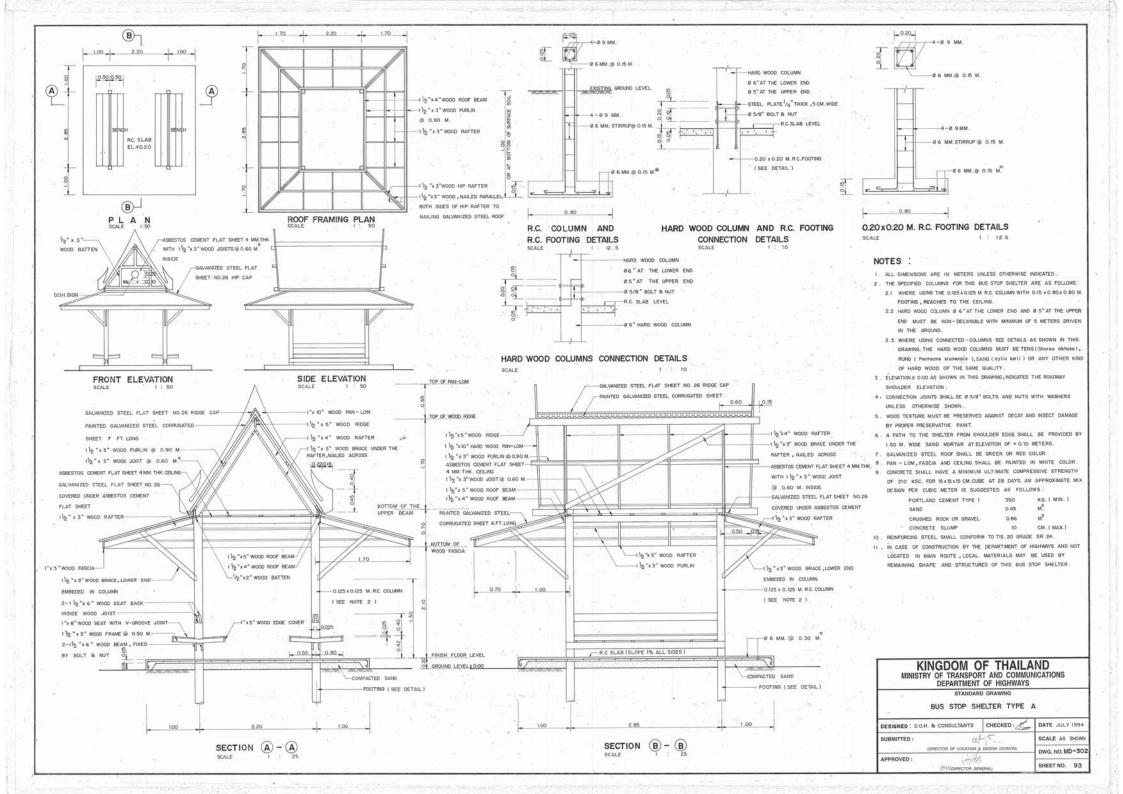
# KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

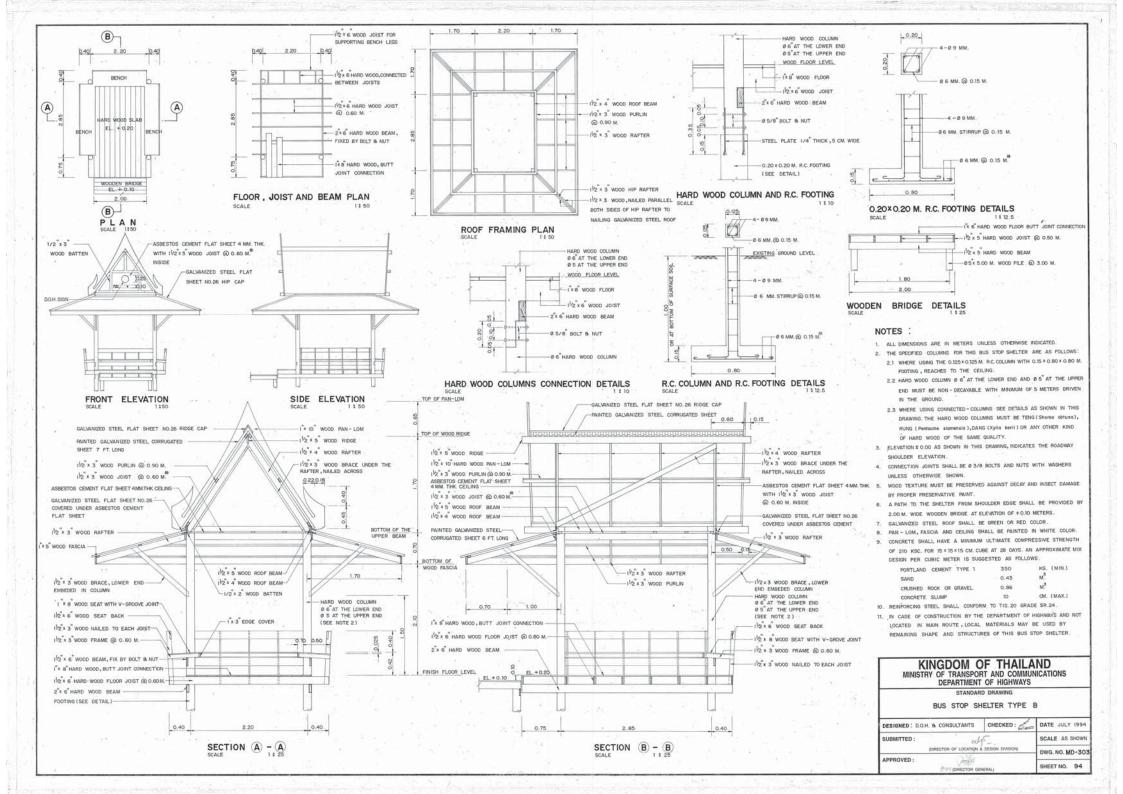
STANDARD DRAWING

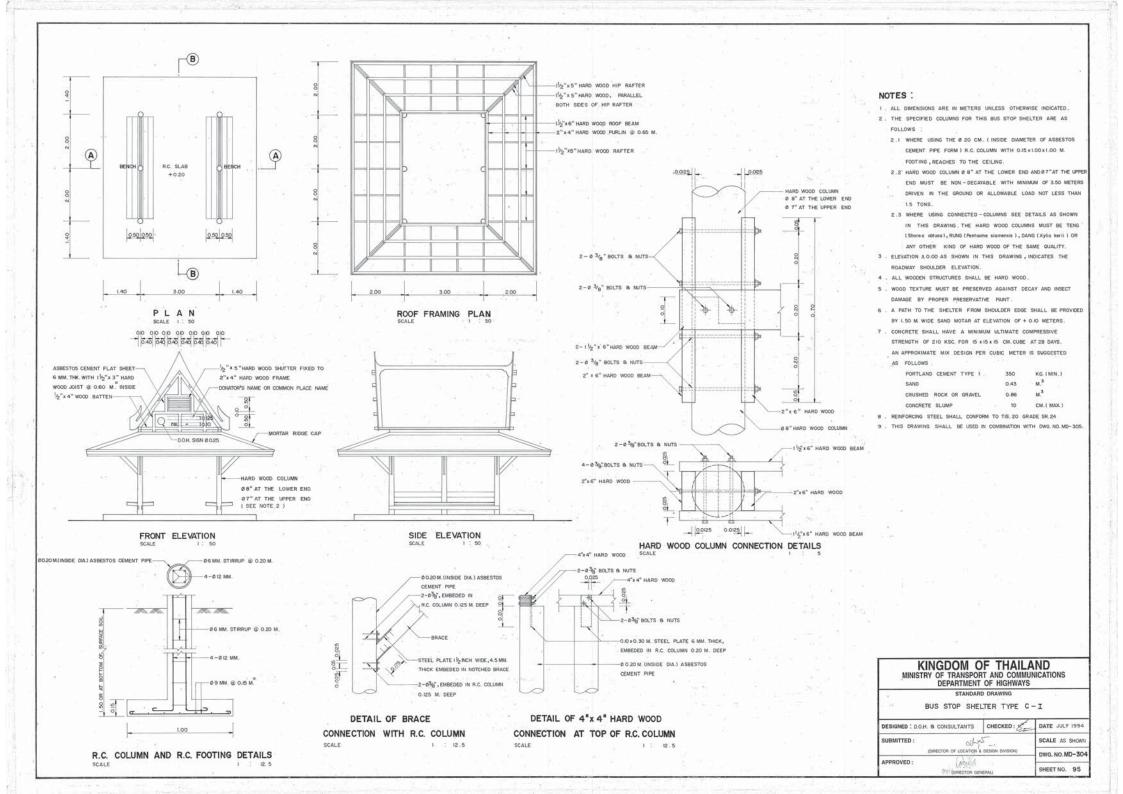
SIDE ROAD & PRIVATE DRIVE DETAILS

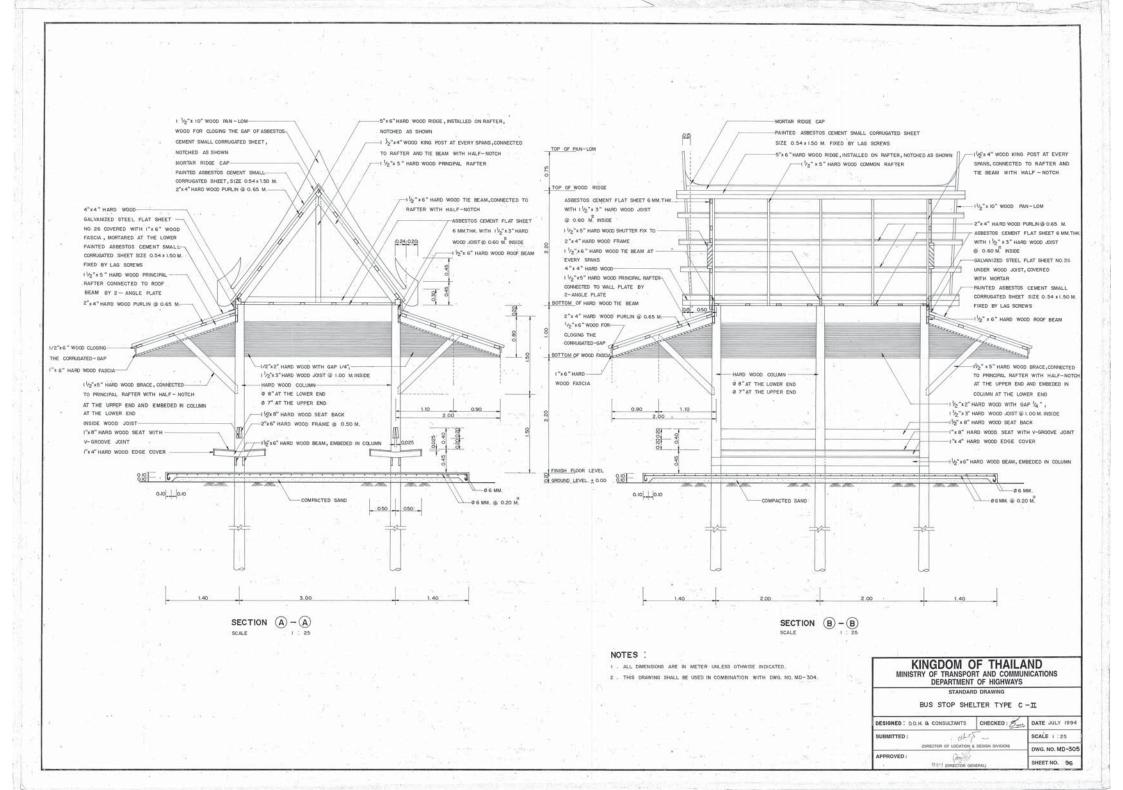
DESIGNED: DO H & CONSULTANTS	CHECKED:	DATE JULY 1994
SUBMITTED:	5_,	SCALE NONE
(DIRECTOR OF LOCATION	& DESIGN DIVISION)	DWG. NO. MD-202
APPROVED:	ENERAL)	SHEET NO. 91

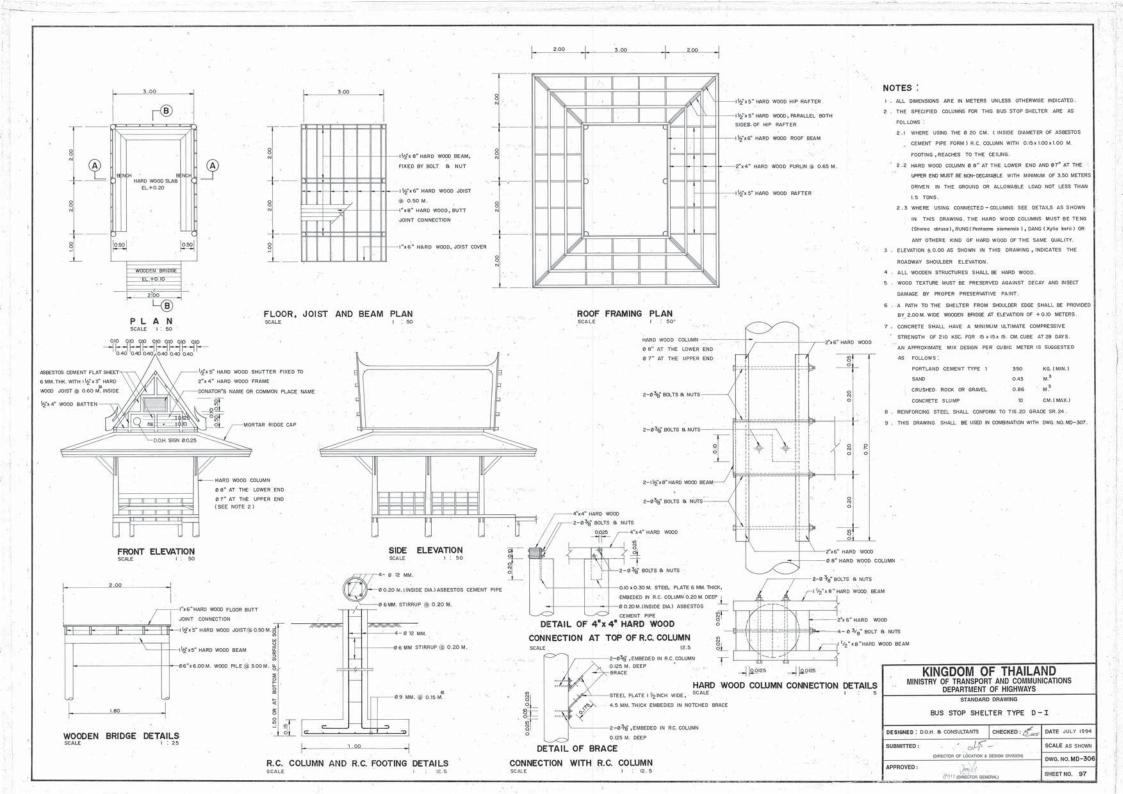


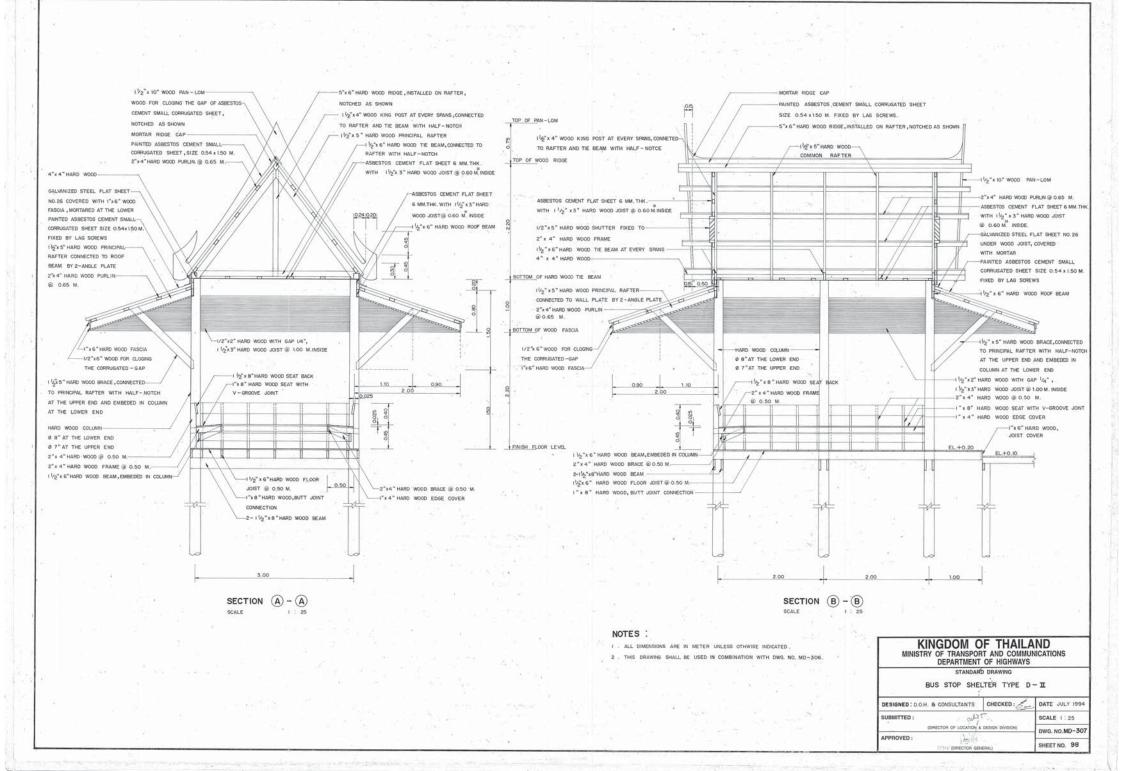


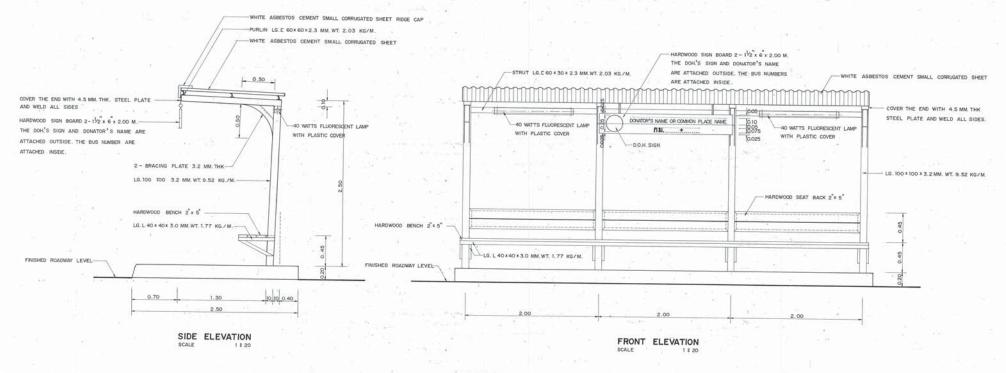












#### NOTES :

- 1. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 210 KSC.
   FOR 15×15×15 CM. CUBE AT 28 DAYS. AN APPROXIMATE MIX DESIGN PER CUBIC
   METER IS SUGGESTED AS FOLLOWS:

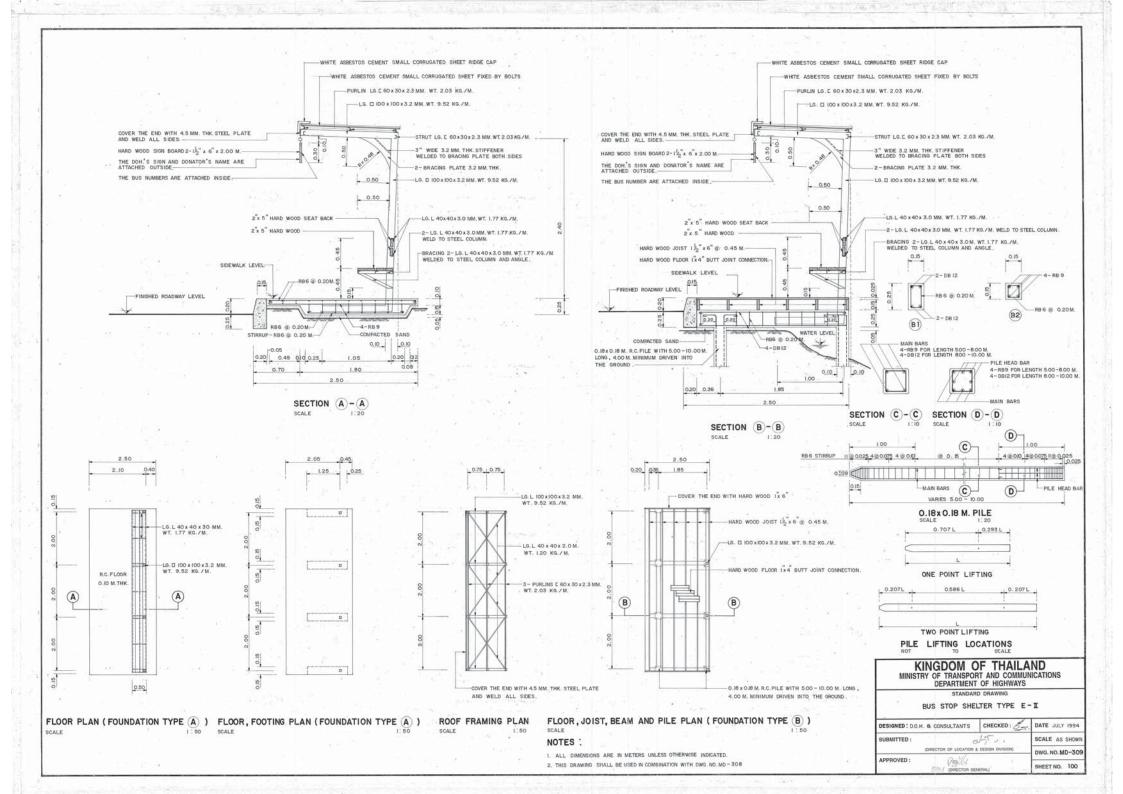
PORTLAND CEMENT TYPE 1 350 KG. (MIN.)
SAND 0.43 M3
CRUSHED ROCK OR GRAVEL 0.86 M3
CONCRETE SLUMP 10 CM. (MAX.)

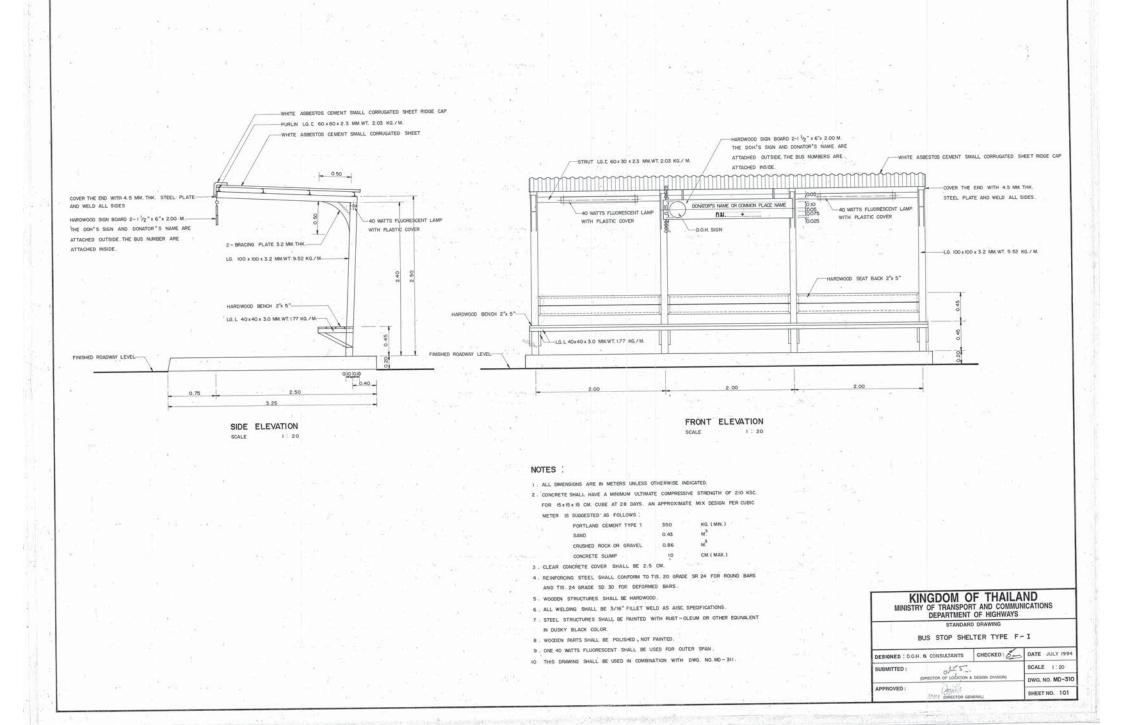
- 3. CLEAR CONCRETE COVER SHALL BE 2.5 CM.
- REINFORCING STEEL SHALL CONFORM TO TIS. 20 GRADE SR 24 FOR ROUND BARS-AND TIS. 24 GRADE SD 30 FOR DEFORMED BARS.
- 5. WOODEN STRUCTURES SHALL BE HARDWOOD.
- 6. ALL WELDING SHALL BE 3/16 FILLET WELD AS AISC. SPECIFICATIONS.
- 7. STEEL STRUCTURES SHALL BE PAINTED WITH RUST-OLEUM OR OTHER EQUIVALENT IN DUSKY BLACK COLOR.
- 8. WOODEN PARTS SHALL BE POLISHED, NOT PAINTED.
- 9. ONE 40 WATTS FLUORESCENT SHALL BE USED FOR OUTER SPAN.
- 10. THIS DRAWING SHALL BE USED IN COMBINATION WITH DWG. NO. MD = 309.

# KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS STANDARD DRAWING

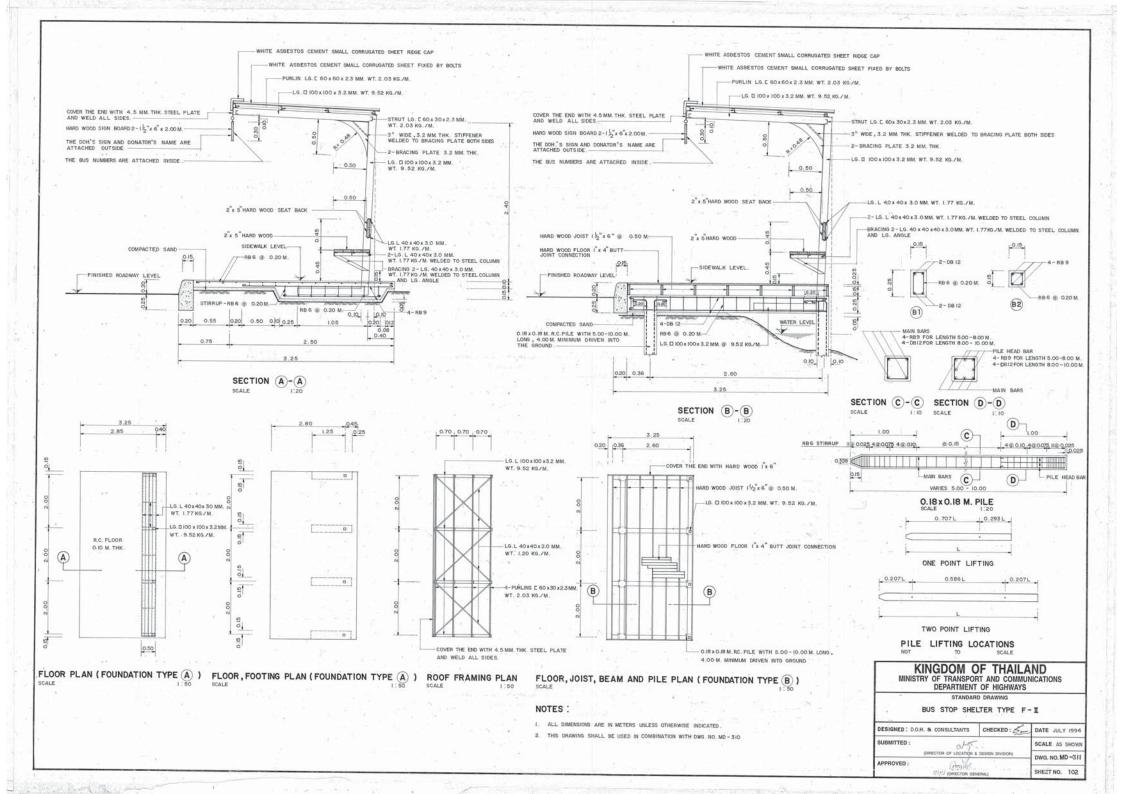
BUS STOP SHELTER TYPE E-I

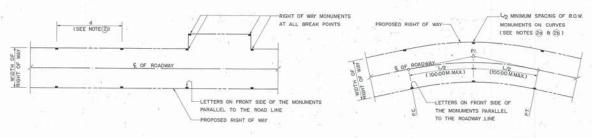
DESIGNED: D.O.H.	& CONSULTANTS	CHECKED :	DATE JULY 1994
SUBMITTED:	o.F.	off.	
(0	RECTOR OF LOCATION	S DESIGN DIVISION)	DWG. NO. MD-30
APPROVED:	1/915 masscrop on	NEDAL V	SHEET NO. 99





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SECTION A-A

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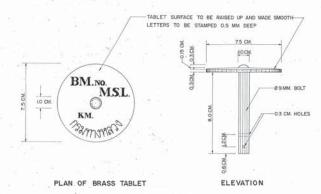
0.02250.0225

R.O.W. MONUMENT IN STRAIGHT LINE

R.O.W. MONUMENT IN HORIZONTAL CURVES

-2-Ø6 MM

-Ø 6 MM. @ 0.15 M.



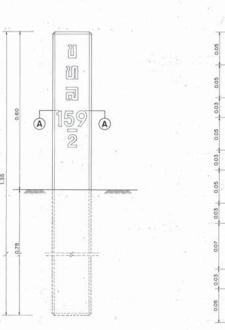
BENCH MARK DETAILS



BENCH MARK LOCATION DETAIL ON BRIDGE

# BACK SIDE OF THE MONUMENT -RIGHT OF WAY FRONT SIDE WITH LETTER AND FIGURES SHOWING KM. CHAINAGE AND DECIMAL NUMBER

PLAN



FRONT ELEVATION RIGHT OF WAY MONUMENT

STANDARD LETTERS & FIGURES

#### NOTES :

- 1. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- 2. LOCATION OF ROW MONUMENTS SHALL BE AS SHOWN ON THE PLAN AND SHALL BE SPECIFIED AS FOLLOWS :
  - a) RURAL (RICE FIELD, WOOD AND ETC.) d = 100 M.
  - b) URBAN (TOWNS AND VILLAGES)
  - c) AT ALL BREAK POINTS OF ROW.
  - d) AT P.C., P.T. AND MIDDLE POINT OF HORIZONTAL CURVE
- 3. PAINTING OF R.O.W. POSTS.
  - a) POSTS, WHITE
  - b) LETTERS & FIGURES , BLACK
- 4. BLACK AND WHITE PAINTS MUST BE GLOSS ENAMAL PAINTS IN ACCORDANCE WITH TIS, 327 AND SHALL BE APPLIED IN 2-LAYERS.
- 5. LETTERS ARE DEPRESSED 1 CM. INTO THE CONCRETE.
- 6. POSTS SHALL BE ERECTED TRULY VERTICAL.
- 7. SIZES OF LETTERS AND FIGURES SHALL BE IN ACCORDANCE WITH DOH'S TRAFFIC CONTROL MANUAL AND THE DRAWING.
- 8. CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 180 KSC. FOR 15×15×15 CM, CUBE AT 28 DAYS, AN APPROXIMATE MIX DESIGN PER CUBIC METER IS SUGGESTED AS FOLLOWS

PORTLAND CEMENT TYPE 1

320 KG.(MIN.)

SAND

0.43 M<sup>3</sup>

CRUSHED ROCK OR GRAVEL

0.86 M<sup>3</sup>

CONCRETE SLUMP

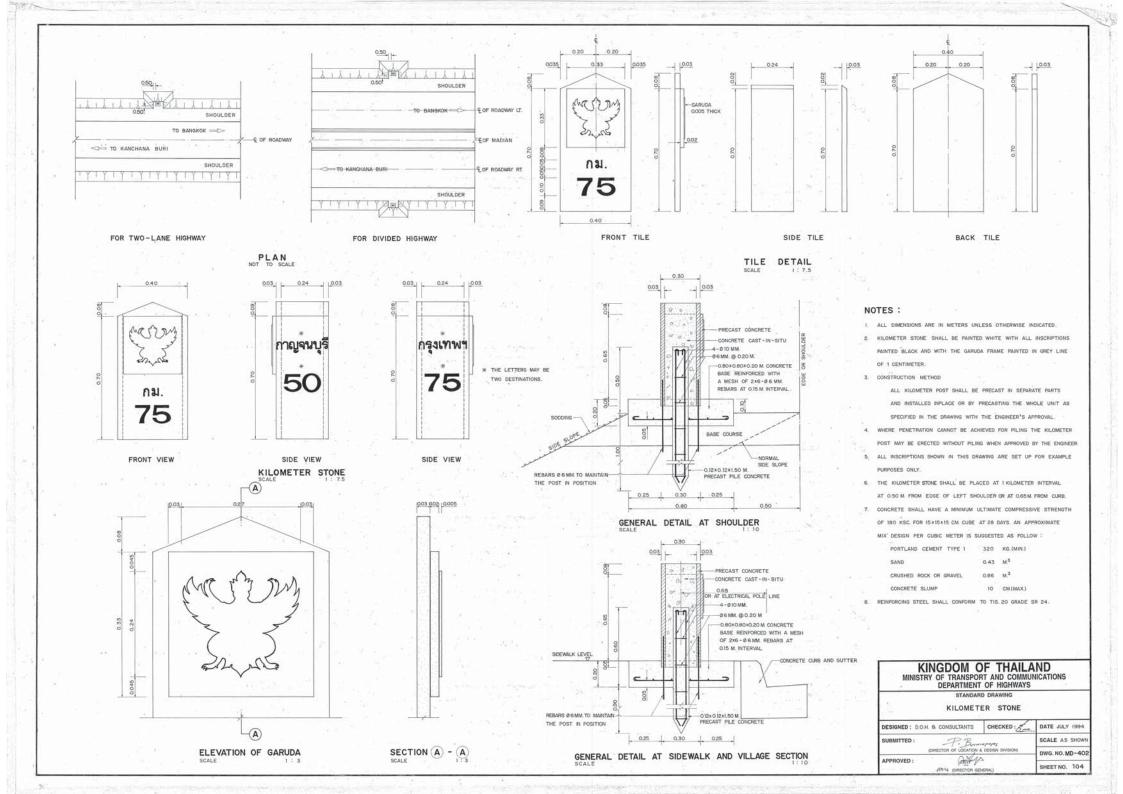
10 - CM.(MAX.)

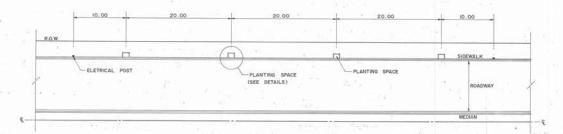
9. RAINFORCING STEEL SHALL CONFORM TO TIS 20 GRADE SR 24

KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

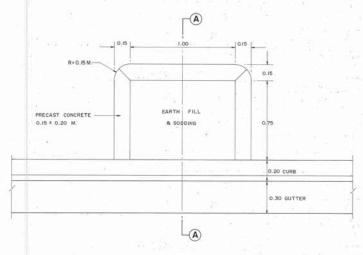
STANDARD DRAWING RIGHT OF WAY MONUMENT & BENCH MARK

DESIGNED : D.O.H. & CONSULTANTS	CHECKED:	DATE JULY 1994
SUBMITTED:	OFF.	
(DIRECTOR OF LOCATION	& DESIGN DIVISION)	DWG. NO. MD-401
APPROVED:	INERAL)	SHEET NO. 103





#### PLANTING SPACE ON SIDEWALK SCALE 12 250



PLANTING SPACE DETAILS

#### NOTES :

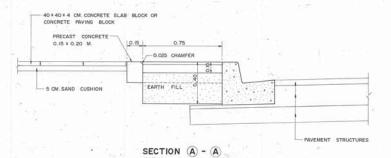
- I. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 180 KSC.
   FOR 15x15x15 CM. CUBE AT 28 DAYS. AN APPROXIMATE MIX DESIGN PER CUBIC METER
   IS SUGGESTED AS FOLLOWS:

 PORTLAND
 CEMENT TYPE I
 320
 KG (MI

 ŞAND
 0.43
 M.<sup>3</sup>

 CRUSHED ROCK OR GRAVEL
 0.86
 M.<sup>3</sup>

- EARTH FILL IN PLANTING SPACE SHALL BE THE PROPORTION OF SOIL: FERTILIZER: SAND EQUAL TO 4:3:2 BY VOLUME AND SHALL INCLUDE IN THE COST OF PLANTING SPACE.
- 4. PLANT TREES SHALL BE PAID FOR IN A SEPARATED ITEM.



KINGDOM OF THAILAND
MINISTRY OF TRANSPORT AND COMMUNICATIONS
DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

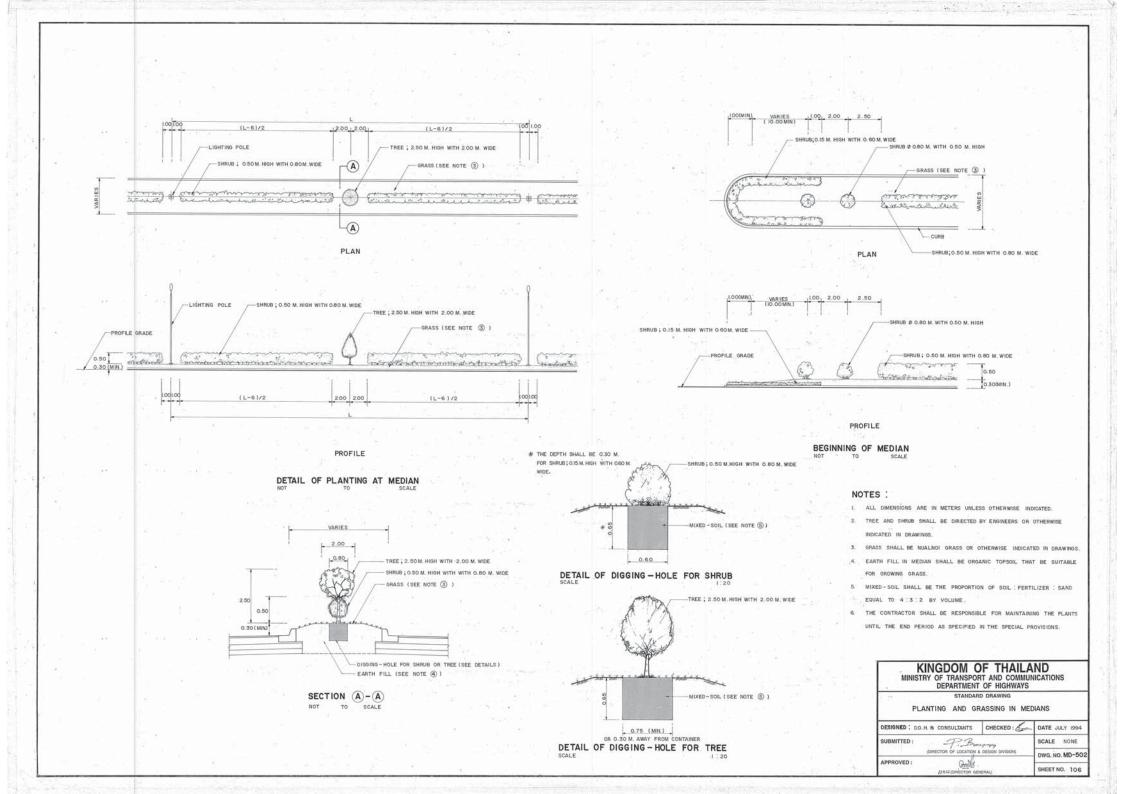
PLANTING SPACE ON SIDEWALK

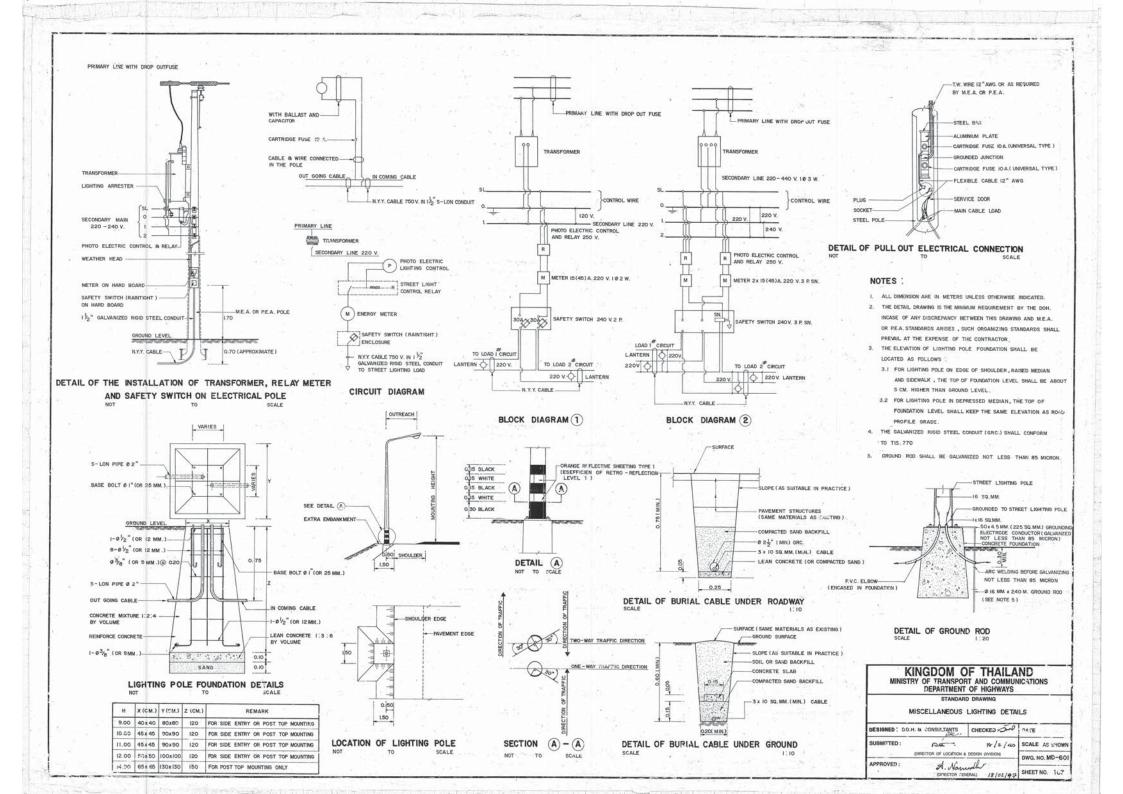
DESIGNED: D.O.H. & CONSULTANTS CHECKED: DATE JULY 1994

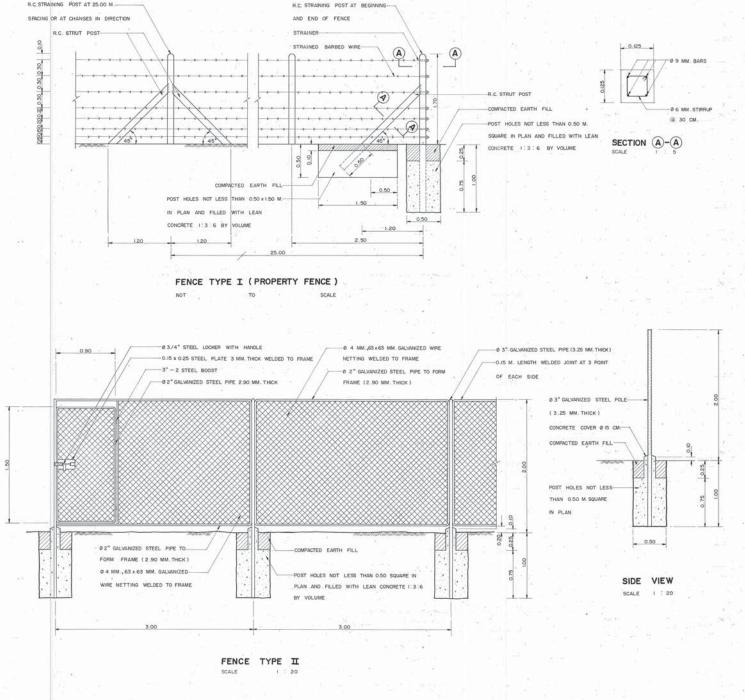
SUBMITTED: SCALE AS SHOWN
DWG. NO.MD-501

APPROVED: SHEET NO. 105

SHEET NO. 105







R.C. STRAINING POST AT 25.00 M .-

#### NOTES :

- I. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- 2. CONSTRUCTION LOCATION OF THE TWO TYPES OF FENCES ARE SHOWN ON LAY OUT PLAN WHERE THE FENCES SHALL NOT BE OBSTRUCTED TRAFFIC, WHEN ACCIDENT ARE OCCURED TO THE FENCES.
- 3 . CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 180 KSC. FOR "5 x 15 x 15 CM. CUBE AT 28 DAYS. AN APPROXIMATE MIX DESIGN PER CUBIC METER IS SUGGESTED AS FOLLOWS

PORTLAND CEMENT TYPE 1 320 KG. ( MIN. ) SAND 0.43 CRUSHED ROCK OR GRAVEL 0.86 10 CM. (MAX.) CONCRETE SLUMP

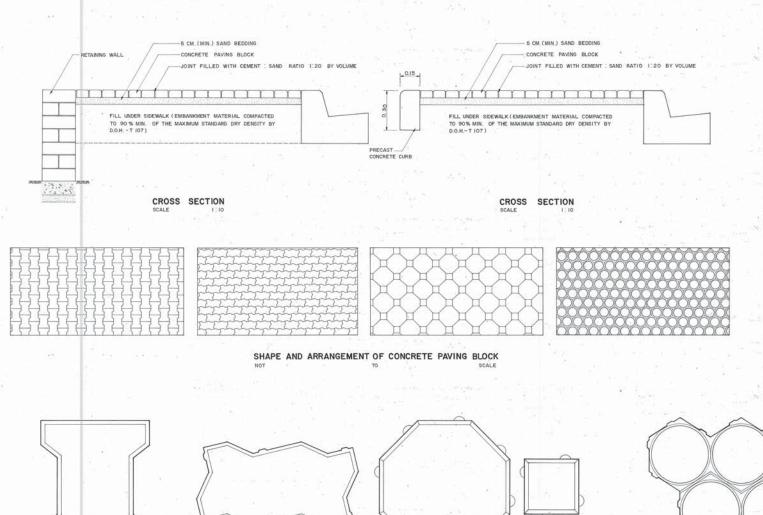
- 4. REINFORCING STEEL SHALL CONFORM TO TIS. 20 GRADE SR. 24
- 5. WIRE NETTING SHALL BE GALVANIZED AS SPECIFIED BY TIS. 208
- 6. STRAINED BARBED WIRE SHALL BE GALVANIZED AS SPECIFIED BY TIS. 76.
- 7. Ø 2" AND Ø 3" STEEL PIPE SHALL BE GALVANIZED.
- B. THE COST OF FENCE TYPE I SHALL INCLUDE THE COST OF R.C. STRUT POST AND ITS FOUNDATION.
- 9. THE COST OF FENCE TYPE II SHALL INCLUDE THE COST OF 0.90 x 1.50 M. EXIT- ENTRANCE OF FENCE GATE, AND ITS FOUNDATION .
- IO. LOCATION OF EXIT-ENTRANCE OF FENCE GATE TYPE II SHALL BE AS DIRECTED BY THE ENGINEER .

KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING FENCING DETAILS

DESIGNED: D.O. H. B. CONSULTANTS CHECKED: DATE JULY 1994 SCALE AS SHOWN DWG. NO. MD-701 APPROVED:

SHEET NO. 108

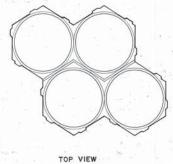


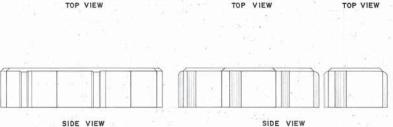
#### NOTES :

- I. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED
- 2. SHAPE AND ARRANGEMENT SHALL BE SPECIFIED IN THE DRAWINGS OR DIRECTED
- 3. COST OF CONCRETE PAVING BLOCK SHALL INCLUDE SAND BEDDING , JOINT FILLING, AND PRECAST CONCRETE CURB.
- 4. CONCRETE PAVING BLOCK SHALL CONFORM TO TIS. 827
- 5. JOINT SHALL BE FILLED WITH CEMENT : SAND RATIO 1:20 BY VOLUME

#### PROPERTIES OF SAND BEDDING AND JOINT FILLING SAND

SIEVE SIZE	% PASS	ING
mm.	SAND BEDDING	JOINTING SAND
9,52	100	-
4.75	95 - 100	8- , <del>-</del> 1
2.36-	80 - 100	100
1.18	50 - 85	90 - 100
0.60	25 — 60	60 - 90
0.30	10 - 30	30 - 60
0.15	5 - 15	15 - 30
0.075	0 - 5	10 - 20



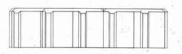


SIDE VIEW

TOP VIEW

SIDE VIEW

CONCRETE PAVING BLOCK BEHATON SHAPE CONCRETE PAVING BLOCK UNI-PAVE SHAPE CONCRETE PAVING BLOCK OCTA-CUBE SHAPE



SIDE VIEW CONCRETE PAVING BLOCK DYWIDAG SHAPE

KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

CONCRETE PAVING BLOCK

DESIGNED: D.O.H. & CONSULTANTS CHECKED:			DATE JULY 1994
SUBMITTED: (DIRECTOR OF LOCATION & DESIGN DIVISION)		SCALE AS SHOWN	
		s DESIGN DIVISION)	DWG. NO. MD-801
APPROVED:	fine (pinecton de	NERALI	SHEET NO. 109

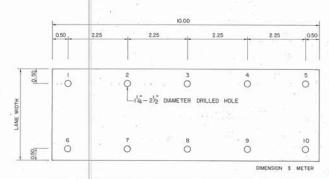


FIGURE 1 PLAN OF DRILLING FOR CEMENT MORTAR COMPRESSION ALONG THE PUMPING JOINT

#### I. PUMPING JOINT REPAIRING

#### 1.1 MATERIALS

MATERIALS TO BE MIXED AND COMPRESSED INTO CONCRETE JOINT ARE AS FOLLOW \$

- 1.1.1 SOIL \$ SOIL SHALL BE SILTY SAND WHICH IS PASSED THROUGH SIEVE NO. IO (IOO%)
  AND SIEVE NO. 200 (0-50%), AND IS NON PLASTIC MATERIAL.
- 1.1.2 CEMENT & CEMENT SHALL BE TYPE III. OR EQUIVALENT.
- 1.1.3 WATER \$ WATER WHICH IS USED FOR MIXING WITH SILTY SAND AND CEMENT MUST BE CLEAN.
- 1.1.4 ADDITIVE \$ ADDITIVE IS USED TO INCREASE CAPACITY OF FLOWING THROUGH SPACE UNDER THE CONCRETE SLAB. THE ADDITIVE SHOULD HAVE SUITABLE QUALITIES AND WILL BE APPROVED BY THE ENGINEER.
- 1.1.5 MIXED RATIO \$ MIXED RATIO OF SLURRY CEMENT MORTAR IS AS FOLLOW \$

- SILTY SAND 20 LITERS

- SILIT SAND 20 LIT

- CEMENT 6 LITER

- WATER IO LITERS
- ADDITIVE AS NECESSARY

THE MIXED RATIO OF SLURRY CEMENT MORTAR CAN BE ADJUSTED BY THE ENGINEER TO OPTIMUM MIXED RATIO FOR SUITABLE CAPACITY OF FILLING INTO THE SPACE UNDER CONCRETE PAYEMENT.

#### 1.2 WORK PREPARING

#### STEPS ARE AS FOLLOW \$

- 1.2.1 THE ENGINEER SPECIFIES THE CONCRETE SLABS TO BE REPAIRED. THE CONCRETE SLABS SHOULD HAVE THE SIGN OF DEFLECTION OR THE TRANSVERSE OR LONGITUDINAL CRACKS THAT CAN BE CLEARLY SEEN.
- 1.2.2 EACH REPAIRED CONCRETE SLAB SHOULD BE MARKED AS THE DRILLING POSITION IN ORDER TO COMPRESS SLURRY CEMENT MORTAR THROUGH, AS SHOWN IN FIGURE I.
- 1.2.3 DRILL THE MARKED POSITION, AS SHOWN IN STEP 1.2.2.
- 1.2.4 PREPARE THE SET OF SLURRY CEMENT MOTAR MIXER, SLURRY CEMENT MORTAR COMPRESSOR, AND OTHER NECESSARY MATERIALS.

#### 1.3 SLURRY CEMENT MORTAR COMPRESSING

- I.3.1 COMPRESS THE SLURRY CEMENT MORTAR INTO THE FIRST HOLE (SHOWN IN FIGURE 1).
  GENTLY DO IT AT FIRST SO THAT THE CEMENT CAN FLOW FREELY AND REGULARLY AND THEN INCREASINGLY COMPRESS THE CEMENT TILL THE MACHINE CAN NO LONGER PRESS.
  - WHILE COMPRESSING THE SLURRY CEMENT MORTAR INTO THE HOLE NO.1, THERE MAY BE SOME SLURRY CEMENT MORTAR SLIPPED INTO AND FILLED THE HOLES NO.2 OR NO.6 AS IN FIGURE I. IF SO, CORK OR PLUG THOSE PARTICULAR HOLES AND DO NOT COMPRESS THE SLURRY CEMENT MOTAR INTO THEM.
- 1.3.2 MOVE THE COMPRESSOR TO THE HOLE NO. 2 AND DO THE SAME WAY AS TO THE HOLE NO. 1. THEN MOVE TO THE REST OF THE HOLES.
- 1.3.3 UNCORK OF UNPLUG THE HOLES. SOME HOLES MAY NEED TO BE REFILLED A LITTLE MORE, THEN SHAPE THE HOLES.
- 1.3.4 LET THE SLURRY CEMENT MORTAR DRY FOR AT LEAST 12 HOURS. THE TRAFFIC SHALL NEED TO BE STOPPED.



FIGURE 2A LONGITUDINAL SINGLE CRACK



FIGURE 2B TRANSVERSE SINGLE CRACK

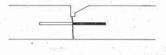


FIGURE 3 SECTION OF SHALLOW JOINT SPALLING



FIGURE 4 SHATTERED SLAB

#### 1.4 JOINT SEALER CHANGING

IN THE AREA WHERE HAVING PUMPING AND SLURRY CEMENT MORTAR COMPRESSING UNDERNEATH THE CONCRETE SLAB, THE OLD JOINT SEALER HAS TO BE REMOVED AND REPLACED BY A NEW JOINT SEALER. THE JOINT SEALER SHOULD BE STANDARDIZED AS SUGGESTED BY THE DEPARTMENT OF HIGHWAYS.

#### 2. TRANSVERSE AND LONGITUDINAL SINGLE CRACK REPAIRING (STRUCTURAL CRACK)

THE TRANSVERSE AND LONGITUDINAL CRACKS ALONG THE THICK OF CONCRETE SLAB CAN BE

THE REPAIR CAN BEGIN WITH COMPRESSING EPOXY INTO SMALL POROUS TUBES LAID ALONG THE CRACKS. THE DISTANCE BETWEEN TUBES IS ABOUT 5-IO CM. THE EPOXY COMPRESSED UNDER HIGH PRESSURE WILL SPREAD THROUGH THE CRACKS ALONG THE THICK OF THE CONCRETE SLAB.

THE ENGINEER MAY USE OTHER APPROPRIATE WAYS OF USING EPOXY AS FOR A BETTER RESULT.

THE AIMED RESULT IS THAT THE CRACKS ARE SO WELL SEALED THAT WATER CAN NOT PASS THROUGH.

THE TRANSVERSE AND LONGITUDINAL CRACKS ARE AS SHOWN IN FIGURE 2.

#### 3. SHALLOW JOINT SPALLING REPAIRING

SHALLOW SPALLING OF JOINT CAN BE REPAIRED BY USING EPOXY MORTAR AND FOLLOWING THESE STEPS \$

- 3.1 CUT THE CONCRETE AROUND THE AREA OF SPALLING AND MAKE A SHAPE OF RECTANGLE (AS DEEP AS 2.5 - 3.0 CM.)
- 3.2 SPREAD THE CUT WITH EPOXY MORTAR WHICH IS A APPROPRIATE COMBINATION OF COARSE SAND AND EPOXY.

THE USE OF EPOXY FOR REPAIRING WORK SHOULD FOLLOW THE INSTRUCTIONS OF THE PRODUCER AND ALSO THE APPROVAL OF THE ENGINEER.

SECTION OF SHALLOW SPALLING IS SHOWN IN FIGURE 3.

#### 4. SHATTERED SLAB REPAIRING

TO REPAIR THE SHATTERED SLAB WITH MORE THAN TWO TRANSVERSE AND LONGITUDINAL CRACKS, THE CONCRETE SLAB NEEDS TO BE DISMANTLED AND THEN THE PIECES OF CONCRETE SLAB ARE TO BE REMOVED. THEN THE AREA WHERE THE SLABS WHERE REMOVED IS POURED NEW CONCRETE OVER. THE METHOD IS AS FOLLOW &

- 4.1 DISMANTLE THE SHATTERED SLABS BY USING A MACHINE AND REMOVE THEM.
- 4.2 EXAMINE AND MAKE SURE IF THE SUBBASE OR SAND CUSHION OR CRUSHED ROCK ARE IN GOOD CONDITION. COMPACTION SHOULD BE THE STANDARD OF THE DEPARTMENT OF HIGHWAYS. SOFT-SPOT FOUND IN ANY AREA HAS TO BE REMOVED AND REPLACED BY SUITABLE MATERIALS SUCH AS CRUSHED ROCK OR SAND.
- 4.3 MAKE A HOLE AND PUT A NEW SET OF TIE BAR INTO IT. (REMOVE THE OLD SET )
- 4.4 THE NEW SET OF TIE BAR SHOULD BE PUT IN THE POSITION NEAR THE POSITION OF THE PREVIOUS SET
- 4.5 POURING CONCRETE OVER THE PREPARED AREA. THE MATERIALS AND CONCRETE MIXING AS WELL AS CONSTRUCTION TECHNIQUES AND BROOMING SHALL BE IN ACCORDANCE, WITH THE STANDARD DH. S. 409 REGARDING \*\* REGULATIONS OF CONSTRUCTION CONTROL OF PORTLAND CEMENT CONCRETE PAWEMENT.\*\* REQUIRE NECESSARILY FOR CONCRETE MIXING AS \$

- PORTLAND CEMENT 350 KG/M<sup>3</sup>
- WATER/CEMET RATIO 0.55 BY WEIGHT
- CONCRETE SLUMP 3-7 CM.

CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 325 KSC. FOR 15 X 15 X 15 CM. CUBE AT 28 DAYS.

- 4.6 JOINTS SHALL BE CUT WITHIN THE SUITABLE TIME ( APPROXIMATE 4-12 HOURS AFTER CONCRETE POURING )
- 4.7 MASTIC JOINT SEALER SHALL BE DROPPED AS APPROVAL BY THE ENGINEER.

#### NOTES

- 1. FOR THE CONCRETE SLAB WITH TRANSVERSE AND LONGITUDINAL SINGLE CRACKS, THE ELEVATION OF THE CONCRETE SLAB IN THE CRACK POSITION NEEDS TO BE CHECKED. IN CASE OF SETTLEMENT OR PAVEMENT STRUCTURES DAMAGE WHICH CAN CAUSE CRACKS AND WHICH WILL NOT BE REPAIRED BY METHOD ②, THE METHOD ③ SHALL BE BROUGHT TO CONSIDERATION.
- TRANSVERSE AND LONGITUDINAL SINGLE CRACKS ON CONCRETE SLABS, AFTER REPAIRING BY USING EPOXY, SHOULD BE CONSIDERED TO COMPRESS SLURRY CEMENT MORTAR INTO THE UNDERNEATH OF THE SLABS.

# KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING

CONCRETE PAVEMENT REPAIRING

DESIGNED: D.O.H. & CONSULTANTS CHECKED: DATE JULY 1994

SUBMITTED: STATE OF LOCATION & DEBINE DUMBON

APPROVED: See: DIRECTION GENERALLY

SHEET NO. 1110

SHEET NO. 1110





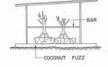




5.) REMOVING



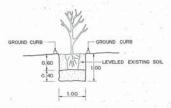
1.00 2.) DIGGING UP



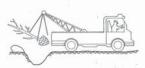
6.) GREEN HOUSE



3.) TAPROOT CUTTING



7.) PLANTING

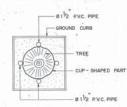


GROUND CURB

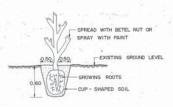
4.) CRANE - TRUCK OR TRIPOD USING 8.) SUPPORTING



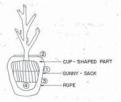
9.) WATERING



10.) P.V.C. PIPE INSTALLATION



CUP - SHAPED SOIL DETAILS



- THE FIRST TIED ROPE
- THE THIRD TIED ROPE
- 4 WRAP THE TREE BY THE KNITTED ROPE (SIMILAR TO WRAP THE WATERMELON)

DETAIL OF WRAPING THE CUP - SHAPED SOIL WITH GUNNY - SACK

#### METHOD OF TRANSPLANTING TREE

- 1. TRIMMING UNNECESSARY BRANCHES TO REDUCE VAPORIZATION OF THE TREE
  - 1.1 CUT MINOR BRANCHES AND LEAVE THEM ABOUT 2".
  - 1.2 SPRAY THE CUTS WITH PAINT OR SPREAD WITH BETEL NUT TO PREVENT FUNGAL INFECTION.
- 2. DIGGING AROUND THE TREE TO REMOVE
  - 2.1 TOOLS USED ARE A HOE, A SPADE AND A CROWBAR.
  - 2.2 BIGGER OR MORE COMPLICATED GARDENING EQUIPMENTS MAY BE USED FOR BIGGER TREES.
  - 2.3 DIG AROUND THE TREE 50 CM. AWAY FROM IT AND GO CM. DEEP. USE THE COWBAR TO CUT OFF ONCE THE MINOR ROOT.
  - 2.4 ADJUST THE SOIL AND MAKE THE SHAPE OF A CUP. (DO NOT CUT THE TAPROOT)
  - 2.5 WRAP AROUND THE TRUNK WHICH IS BEING COVERED WITH CUP-SHAPED SOIL, WITH THE GUNNY-SACK. SEW THEM TOGETHER IF TWO OR THREE GUNNY - SACKS ARE USED. TIE UP THE GUNNY - SACKS TIGHTLY WITH 30 - 50 ROPES. USED TRIPOD TO HOLD UP THE TRUNK AT LEAST 21 DAYS. (SOME KINDS OF TREE DO NOT ) WATERING THE GUNNY - SACKS , NOT LET TO DRY.
  - 2.6 AFTER 21 DAYS EXAMINE THE TREE, IF IN SUITABLE STATE, DIG INTO THE GROUND FOR ANOTHER 10 20 CM. CUT THE TAPROOT WITH A CROWBAR . WHILE IN THIS STEP , DO IT QUICKLY TO PROJECT THE REST OF THE TREE .

#### 3. REMOVING

- 3.1 WRAP AROUND THE TRUCK WITH GUNNY-SACKS.
- 3.2 LIFT THE TREE FROM THE GROUND BY CRANE-TRUCK OR TRIPOD AND HELP WITH MAN.
- 3.3 LAY THE TREE DOWN ON THE BACK OF TRUCK, UNDER THE TREE ARE PIECES OF GUNNY SACKS PUT TO PROTECT THE TREE. IF THE TREE IS LONGER THAN THE BACK OF TRUCK, LAY THE BRANCHES-SIDE ON THE TRUCK'S ROOF AND THE CUP-SHAPED

#### 4. PRE - TRANSPLANTING

- 4.1 BEFORE TRANSPLANTING, THE TREE SHOULD BE PUT TO REST IN THE GREENHOUSE ABOUT 1 MONTH FOR SOFTWOOD OR 3 MONTHS FOR HARDWOOD. COVER THE CUP-SHAPED PART WITH COCONUT FUZZ AND WAIT TILL THE LEAVES GROW.
- 4.2 THE TREE MAY BE PUT ANYWHERE OTHER THAN IN THE GREENHOUSE AS LONG AS THERE IS NO STRONG SUNLIGHT.

## 5. PLANTING

- 5.1 PREPARE A HOLE OF SIZE 1.00 x 1.00 x 1.00 M.
- 5.2 FILL THE HOLE WITH THE EXISTING SOIL WHICH IS USED AS BASE 40 CM. THICK, PUT THE TREE ON THE BASE SURFACE AND THE CUP-SHAPED PART IS AT THE SAME LEVEL AS THE EXISTING GROUND AT TOP OF THE HOLE.
- 5.3 BEFORE FILLING THE HOLE, USE DILUTE LYSOL SOLUTION TO PREVENT FUNGAL INFECTION BY POURING THE DILUTE LYSOL SOLUTION ON THE CUP-SHAPED PART OF THE TREE . THEN FILL THE HOLE WITH THE EXISTING SOIL AND COMPACTED LOOSELY UNTIL THE SOIL IS AT THE SAME LEVEL AS THE CUP - SHAPED PART.
- 5.4 BUILD GROUND CURB AROUND THE TREE. THIS MAKES A CIRCLE WITH A RADIUS OF 1 METER. THE CURB WILL HELP PRESERVE WATER.
- 5.5 USE TRIPOD TO SUPPORT THE TREE.

## 6. MAINTENANCE

- 6.1 WATERING SHOULD BE DONE IN THE MORNING AND EVENING."
- 6.2 WATER SPRAYING (BY SPRINGER) SHOULD BE DONE FROM THE TOP TO THE BOTTOM OF THE TREE (VERTICALLY).
- 6.3 IT USUALLY TAKES ONE MONTH FOR YOUNG LEAVES TO COME OUT DEPENDING UPON THE KINDS OF TREES. IN THE MEAN TIME, THE TREE WILL ALSO NEED VITAMIN B-1 AT THE MORNING AND EVENING TWICE A WEEK.
- 6.4 AFTER ONE MONTH, FERTILIZER (15-30-15) SHALL BE GIVEN
- 6.5 IF IT IS DIFFICULT TO WATER THE TREE SINCE IT IS GROWN ON HARD SOIL SUCH AS LATERITE SOIL , THE RV.C. PIPE @ 2 1/2 WHICH IS 30 CM. LONG WITH DRILLED AT BOTTOM PART Ø 5 MM, WITH 10 CM. APART SHALL BE BURIED, WATER WILL BE SENT THROUGH THE PV.C. PIPE TO THE ROOTS.
- 6.6 THE TREE WILL GROW IN ABOUT 3 MONTHS AND WILL COMPLETELY GROW IN ONE YEAR.

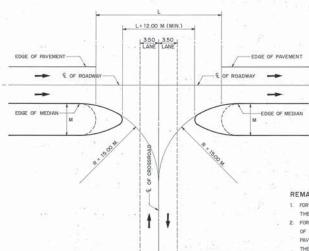
#### NOTES:

- 1. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE INDICATED.
- 2. TREES TO BE TRANSPLANTED FROM THE AREA OF CONSTRUCTION ARE TREES THAT ARE IMPORTANT TO BE RESERVED AS DIRECTED BY THE ENGINEER.
- 3. THE ENGINEER WILL ASSIGN A NEW LOCATION TO PLANT OUT OF ROADBED.
- 4. WHEN THE TRANSPLANTING ARE FINISHED, THE CONTRACTOR SHALL TAKE CARE OF THE TREES UNTIL THEY ARE BIG ENOUGH.
- 5. THE DEPARTMENT OF HIGHWAYS SHALL PAY ACCORDING TO THE SIGNS OF THE GROWTH OF THE TREES. IF THE TREES DIE, THE DEPARTMENT OF HIGHWAYS SHALL PAY ACCORDING TO ROADWAY EXCAVATION (EARTH). THE NUMBERS OF THE HOLES SHALL BE TAKEN INTO ACCOUNT.

#### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

METHOD OF TRANSPLANTING TREE

DESIGNED: D.O.H. & CONSULTANTS		CHECKED :	DATE JULY 1994
SUBMITTED:	W.S.		SCALE NONE
4 2	(DIRECTOR OF LOCATION	s DESIGN DIVISION)	DWG. NO.MD-901
APPROVED:	White preserves of	NEBALL	SHEET NO. 111



MINIMUM DESIGN OF MEDIAN OPENINGS

TABLE : MINIMUM DESIGN OF MEDIAN OPENINGS (SU DESIGN VEHICLE, CONTROL RADIUS OF 15.00 M.)

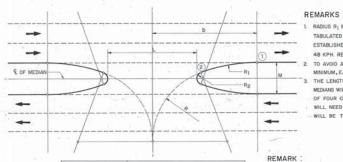
WIDTH OF MEDIAN, M	L = MINIMUM LENGTH OF MEDIAN OPENING ( M.)				
-( M, )	SEMICIRCULAR -	BULLET NOSE			
1.20	29.00	29.00			
1.80	28.00	23.00			
2.40	28.00	20.00			
3.00	27.00	19.00			
3.60	26.00	17.00			
4.20	26.00	16.00			
4.80	25.00	15.00			
6.00	24.00	13.00			
7.20	23.00	12.00 MIN.			
8.40	22.00	12.00 MIN.			
9.60	20.00	. 12.00 MIN.			
10.80	19.00	12.00 MIN.			
12.00	18.00	12.00 MIN.			
15.00	15.00	12.00 MIN.			
18.00	12.00 MIN.	12.00 MIN.			
21.00	12.00 MIN.	12.00 MIN.			

#### REMARKS

- 1. FOR A MEDIAN WIDTH OF 3.00 M. OR MORE THE BULLET NOSE IS SUPERIOR TO
- 2. FOR ANY THREE OR FOUR LEG INTERSECTION ON A DIVIDED HIGHWAY THE LENGTH OF MEDIAN OPENING SHOULD BE AS GREAT AS THE WIDTH OF CROSSROAD ROADWAY PAVEMENT PLUS SHOULDER AND IN NO CASE LESS THAN 12:00 M ( OR LESS THAN THE CROSSROAD PAYEMENT PLUS 2.40 M.) WHERE THE CROSSROAD IS A DIVIDED HIGHWAY, THE LENGTH OF OPENING SHOULD BE AT LEAST EQUAL TO THE WIDTH OF THE CROSSROAD ROADWAYS PLUS THAT OF THE MEDIAN PLUS 2.40 M.
- WIDTH OF MEDIAN OR THE CONTROL RADIUS SHOULD NOT BE CONSIDERED EXCEPT AT VERY MINOR CROSSROADS. THE 12:00 M. MINIMUM LENGTH OF OPENING DOES NOT APPLY TO OPENINGS FOR U-TURNS, AS DISCUSSED ELSEWHERE

15.00

24.00



DESCRIPTION OF THE PROPERTY OF THE PARTY OF

WIDTH OF MEDIAN . M	DIMENSIONS IN METERS, WHEN					
(M.)	R = 27.00 M.		R = 45.00 M.		R = 69.00 M.	
	L	b	L	b	L	b
6.00	17.00	19.00	20.00	23.00	21.00	27.00
9.00	14.00	20.00	17.00	26.00	19.00	30.00
12.00	12.00	21.00	15.00	27.00	17.00	33.00
15.00	-	-	13.00	29.00	15.00	35.00
18.00		-	-	-	14.00	37.00
21.00		-		-	12.00	38.00

#### ABOVE - MINIMUM DESIGN OF MEDIAN OPENINGS FACTOR FOR GRADE. ( TYPICAL BULLET NOSE ENDS )

DECELERATION LENGTH 1.0 M/S FOR ADDED THROUGH LANE

R\* . EDGE OF PAVEMENT ROUNDING I 100 FOR DECEL LANE 250 FOR ADDED THROUGH LANE )

Dd \* 50 MAX - RURAL FOR DECEL LANE

Dd = V Yd 3.6 S S \* RATE OF LATERAL MOVEMENT ( M./SEC )
V \* DESIGN (OR 85 PERCENTILE OPERATING ) SPEED (KM./H.

RADIUS R1 MAY VARY FROM ABOUT 24.00-120.00 M. OR MORE THE TABULATED VALUES SHOWN, 27.00 M, 45.00 M, AND 69.00 M. ARE

ESTABLISHED MINIMUM RADII FOR TURNING SPEEDS OF 32, 40, AND TO AVOID A LARGE OPENING, R SHOULD BE HELD TO A REASONABLE

THE LENGTH OF MEDIAN OPENING IS GOVERNED BY THE RADII. FOR

MEDIANS WIDER THAN ABOUT 9.00 M. COUPLED WITH A CROSSROAD OF FOUR OR MORE LANES, THE CONTROL RADIUS R GENERALLY WILL NEED TO BE GREATER THAN 15.00 M. OR THE MEDIAN OPENING

MINIMUM, E.G., 15.00 M., AS USED IN FIGURE

THE LENGTH OF THESE LANES IN RURAL AREAS IS GENERALLY BASED ON DECELERATION CONSIDERATIONS HOWEVER IN URBAN AREAS WHERE TRAFFIC SPEEDS ARE SIGNIFICANTLY LOWER AND RIGHT-OF-WAY MORE RESTRICTIVE . THE LENGTH MAY B DICTATED MORE BY VEHICLE STORAGE REQUIREMENTS, PARTICULARLY AT TRAFFIC SIGNAL CONTROLLED INTERSECTIONS. AT LOCATIONS WHERE A FULL LENGTH DECELERATION LANE

CANNOT BE PROVIDED, A MINIMUM TREATMENT (E.G. 30.00 M. TAPER PLUS 30.00 M.

REQUIRED FOR DECELERATION LANES IS GIVEN IN TABLE. THIS LENGTH IS PROVIDED BY

TYPICALLY 50:00 M. RURAL, 30:00 M. URBAN, OR LESS. THE LENGTH OF DECELERATION

PARALLEL LANE ) CAN STILL PROVIDE SIGNIFICANT BENEFITS. THE TOTAL LENGTH

MEANS OF A TAPER AND A PARALLEL LANE. THE LENGTH OF TAPER ADOPTED IS

LANE ON GRADE, IS DETERMINED BY MULTIPLYING THE LENGTH WITH CORRECTION

Yd + LATERAL MOVEMENT (M) OF VEHICLE (OR ADDED ROAD WIDTH ).

#### DIVERGE TAPERS



AUXILIARY TURN LANE TAPERS

V = DESIGN (OR 85 PERCENTILE OPERATING) SPEED (KM./+ Ym = LATERAL MOVEMENT (M) OF VEHICLE, OR WIDTH TABLE : LENGTH OF DECELERATION LANES REDUCTION.

ROAD (KM/H)	0,	20	30	40	50	60	70	80	90
50	60	54	46	32		-	-	-	1
60	80	74	64	50	28		777	777	100
70	100	94	82	70	54	34	-		-
80	120	11.2	104	94	82	64	40	-	-
90	140	134	126	116	102	88	70	44	-
100	170	162	154	144	132	118	100	80	50

### TABLE : CORRECTION FOR GRADE

GRADE	RATIO OF LENGTH ON GRADE TO LENGTH OF LEVEL				
	UPGRADE	DOWNGRADE			
0-2%	1.0	1.0			
3-4%	0.9	1.2			
5-6%	0.8	1.35			

#### KINGDOM OF THAILAND MINISTRY OF TRANSPORT AND COMMUNICATIONS DEPARTMENT OF HIGHWAYS

STANDARD DRAWING MEDIAN OPENING

MEDIAN	Of Elitho	
DESIGNED ; D.O.H. & CONSULTANTS	CHECKED:	DATE JULY 1994
SUBMITTED: F. B.	7-70	SCALE NONE
(DIRECTOR OF LOCATION	A DESIGN DIVISION)	DWG. NO. MD-902
APPROVED:		SHEET NO. 112

- THE SEMICIRCULAR END AND PREFERABLY SHOULD BE USED IN DESIGN.
- 3. THE USED OF A 12:00 M. MINIMUM LENGTH OF OPENING WITHOUT REGARD TO THE

# (SU DESIGN VEHICLE, CONTROL RADIUS OF 15.00 M.) SCALE

#### TABLE : EFFECT OF SKEW ON MINIMUM DESIGN FOR MEDIAN OPENINGS (TYPICAL VALUES BASED ON CONTROL RADIUS OF 15.00 M.)

SKEW			F MEDIAN OPENING		
ANGLE	WIDTH OF MEDIAN	SEMICIRCULAR	BULLET	NOSE	R FOR DESIGN C
( DEGREES )	( DEGREES )	A	SYMMETRICAL B	ASYMMETRICAL C	1
0	3.00	27.00	19.00	II	-
	6.00	24.00	1,3.00		
	9.00	21.00	12.00 MIN	-	= =
	12.00	, 18.00	12.00 MIN.		-
	15.00	15.00	12.00 MIN.	-	
	18.00	12.00	12.00 MIN.	-	-
10	3.00	32.00	24.00	23.00	21.00
	6.00	28.00	17.00	16.00	20.00
	9.00	25.00	14.00	12.00 MIN.	20.00
	12.00	21.00	12.00 MIN.	12.00 MIN.	19.00
	15.00	18.00	12.00 MIN.	12.00 MIN.	18.00
	18.00	14.00	12.00 MIN.	12.00 MIN.	18.00
20	3.00	36.00	29.00	27.00	29.00
	6.00	32.00	22.00	20.00	28.00
	9.00	28.00	18.00	14.00	26.00
	12.00	24.00	14.00	12.00 MIN.	25.00
	15.00	20.00	12.00 MIN.	12.00 MIN.	23.00
	18.00	16.00	12.00 MIN.	12.00 MIN.	21.00
30	3.00	41.00	34.00	32.00	. 42.00
	6.00	36.00	27.00	23.00	39.00
10	9.00	31.00	23.00	17.00	36.00
	12.00	27.00	19.00	13.00	33.00
	15:00	• 23.00	15.00	12.00 MIN.	30.00
	18.00	18.00	12.00	12.00 MIN.	27.00
40	3.00	44.00	38.00	35.00	63.00
	6.00	39.00	32.00	27.00	58.00
	9.00	35.00	27.00	20.00	52.00
	12.00	29.00	23.00	15.00	47.00
		10001189		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	100000000000000000000000000000000000000

19.00

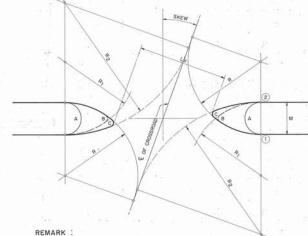
15.00

12.00 MIN

12.00 MIN.

42.00

36.00



IN GENERAL, MEDIAN OPENINGS LONGER THAN 24.00 M. OR 30.00 M. SHOULD BE AVOIDED.

REGARDLESS OF SKEW THIS PLAN MAY CALL FOR SPECIAL CHANNELIZATION . RIGHT-TURN LANES. OR ADJUSTMENT TO REDUCE THE CROSSROAD SKEW, ALL OF WHICH RESULT IN ABOVE-MINIMUM DESIGNS

#### MINIMUM DESIGN OF MEDIAN OPENINGS (EFFECT OF SKEW) SCALE

#### NOTES:

- 1. MEDIAN OPENING SHOULD BE 3 5 KM. INTERVAL FOR RURAL AREAS WITH VILLAGE 1.5 KM. (MINIMUM) LONG IN THE INTERVAL.
- 2. MEDIAN OPENING IN URBAN AREAS SHOULD BE 250 M. (MINIMUM.) INTERVAL (FOR THE LARGE CITY).

